FOR MORE EXCLUSIVE

(Civil, Mechanical, EEE, ECE) ENGINEERING & GENERAL STUDIES

(Competitive Exams)

TEXT BOOKS, IES GATE PSU's TANCET & GOVT EXAMS
NOTES & ANNA UNIVERSITY STUDY MATERIALS

VISIT

www.EasyEngineering.net

AN EXCLUSIVE WEBSITE FOR ENGINEERING STUDENTS & GRADUATES



**Note: Other Websites/Blogs Owners Please do not Copy (or) Republish this Materials without Legal Permission of the Publishers.

**Disclimers: EasyEngineering not the original publisher of this Book/Material on net. This e-book/Material has been collected from other sources of net.

Useful for

- GPSC Assistant Engineers
- GPSC Lecturers Civil
- Engineering Services
- Civil Services
- GATE, M.E. Entrance Tests
- Municipal Corporation Tests
- Interviews and Recruitment Tests

OBJECTIVE CIVIL ENGINEERING

Dr. R. P. Rethaliya

(Ph. D. Structural engineering)
Senior Lecturer, Applied Mechanics Dept.
B.& B. Institute of Technology
and consulting structural engineering
Vallabh Vidyanagar - 388120.

Second Edition: 2016-2017

Price : ₹ 450=00



PUBLISHER:

ATUL PRAKASHAN

Under Farnandis Bridge,

Gandhi Road, Ahmedabad-380 001.

Office: (079) 22160475, 26424342

Stall: 22141244, 25356178

Second Edition: 2016-2017

Price : ₹ 450=00

COPYRIGHT:

All rights reserved by the author. No.parts of this publication may be reproduced, stored in a retrieval system, or transmitted in any form or by any means, electronic, mechanical photocopying, recording or otherwise without the prior consent of the author and Publisher.

Disclaimer: Every care has been taken by the author and publisher to give correct, complete and updated information. In case there is any omission, printing mistake or any other error might have crept in inadvertently, either the author not the publisher take any legal responsibility.

Type Setting:

SHREENATH COMPUTER

1179, Lalpole in Hanumanwali pole, Dariapur, Ahmedabad-380 001.

PREFACE

I take an opportunity to present this treatise entitled as "Objective Civil Engineering" to the students preparing for GPSC, Engineering services, Civil services, Municipal corporation tests and other competitive examinations, interviews.

A large number of objective type questions with answers and explanations are provided for each topic. Almost all the subjects related to Civil Engineering have been covered in this book. Multiple choice questions from various competitive examinations have been incorporated in the various topics. I hope the book will embrace the requirements of the students of civil engineering.

Any errors, omissions and suggestions for the improvement of this book brought to my notice, will be thankfully acknowledged.

I thank Shri Hemendrabhai Shah and Shri Bipinbhai Shah for their keen interest in publishing this book.

August 2016

Dr. R. P. Rethaliya

INDEX

| | <u></u> |
|------------|--|
| CHAPTER-1: | APPLIED MECHANICS1 to 52 |
| 1: Introdu | etion 1 to 2 |
| 1.1 | Engineering Mechanics |
| 1.2 | Scalar and vector quantities 1 |
| 1.3 | Fundamental units |
| 1.4 | Conversion of units |
| | Important Short Questions |
| | Answers |
| 2 : Coplan | ar Concurrent Forces |
| 2.1 | Force |
| 2.2 | Weight and mass |
| 2.3 | System of forces |
| 2.4 | Resultant force6 |
| 2.5 | Parallelogram law of forces |
| 2.6 | Lami's Theorem |
| 2.7 | Triangle law of forces |
| 2.8 | Polygon law of forces |
| 2.9 | Resultant of more than two concurrent forces |
| whhi i | Important Short Questions |
| | Answers 8 |
| 3. Coplana | r non-concurrent forces |
| 3.1 | Moment9 |
| 3.2 | Varignon's principle of moments |
| 3.3 | Couple |
| 3.4 | Types of beams11 |
| 3.5 | Types of loads |
| | |

| 3.6 | Types of supports | 12 |
|-------------|--|----|
| | Important Short Questions | 14 |
| | Answers | 14 |
| 4. Centroid | and Centre of Gravity15 to | 19 |
| 4.1 | Centroid | 15 |
| 4.2 | Centre of gravity (C.G.) | 15 |
| 4.3 | Centroids of Standard Shape | 16 |
| | Important Short Questions | 19 |
| | Answers | 19 |
| 5. Friction | 20 to | 23 |
| 5.1 | Friction or friction force | 20 |
| 5.2 | Limiting friction | 20 |
| 5.3 | Types of friction | 20 |
| 5.4 | Angle of friction | 21 |
| 5.5 | Coefficient of friction | 21 |
| 5.6 | Angle of repose | 22 |
| 5.7 | Laws of static friction | 22 |
| | Important Short Questions | 23 |
| | Answers | 23 |
| 6. Simple | Lifting Machines | 30 |
| 1. | Mechanical advantage (MA) | 24 |
| 6.2 | Systems of pulley | 28 |
| 6.3 | Different simple machines and their velocity ratio | 28 |
| · | Important Short Questions | 30 |
| | Answers | 30 |
| 7. Work, P | ower, Energy31 to | 33 |
| 7.1 | Work | 31 |
| 7.2 | Work necessary for lifting the body | 31 |
| 7.3 | Power | 32 |
| | | |

| | 7.4 | Energy | 32 |
|----------|---|--|----------|
| | 7.5 | Potential energy | |
| | 7.6 | Kinetic energy | 32 |
| | 7.7 | Law of conservation of energy | 32 |
| | | Important Short Questions | |
| | | Answers | 33 |
| 8. Mo | ment | of Inertia | 34 to 52 |
| | 8.1 | Moment of inertia (I) | 34 |
| | 8.2 | Section modulus (Z) | 34 |
| . | 8.3 | Radius of gyration (k) | 34 |
| HD. | 8.4 | Moment of inertia of some standard sections | 35 |
| | 8.5 | Parallel axis theorem | 37 |
| | 8.6 | Perpendicular axis theorem | 38 |
| | | MCQ'S | 39 |
| | | Answers | 48 |
| | | Explanations | |
| CHAPTER | - 2 : | STRENGTH OF MATERIALS | |
| | | nical Properties of Materials | |
| 2 : Di | rect ! | Stress and Strain | 56 to 70 |
| | 2.1 | Change studies all and autority | 56 |
| | 2.1 | Sress, strain, dl calculations | |
| ۸ | 2.2 | Composite Section | 58 |
| Appris | | · | 58 |
| Appri | 2.2 | Composite Section | 58 60 |
| Appris | 2.2 | Composite Section | |
| Appris | 2.2 2.3 2.4 2.5 | Composite Section Elastic constants Thermal stress | |
| 3 : Sh | 2.2 2.3 2.4 2.5 2.6 | Composite Section Elastic constants Thermal stress Strain energy and impact loading | |
| 3 : Sh | 2.2 2.3 2.4 2.5 2.6 ear fe | Composite Section Elastic constants Thermal stress Strain energy and impact loading Tension test on mild steel | |
| 3 : Sh | 2.2 2.3 2.4 2.5 2.6 ear fe | Composite Section Elastic constants Thermal stress Strain energy and impact loading Tension test on mild steel orce and Bending Moment diagrams | |

| 3.3 | Point of contraflexure | 71 |
|--------------|--|-----------|
| 3.4 | Relation between S.F. and B.M | 71 |
| 4 : Slope a | nd deflection of Beams72 to | 74 |
| 4.1 | Slope and deflection | 72 |
| 4.2 | Slope - deflection equations | 73 |
| 5 : Bending | g stresses in Beams and Shear Stresses75 to | 78 |
| 5.1 | Pure Bending Stress | 75 |
| 5.2 | Assumptions in the theory of pure bending | 75 |
| 5.3 | Theory of pure bending | 75 |
| 5.4 | Bending equation | 76 |
| 5.5 | Maximum Bending Moment | 77 |
| 5.6 | Equation of shear stress | 78 |
| 5.7 | Shear stress distribution diagrams | 78 |
| 6 : Column | s and Struts | 83 |
| 6.1 | Strut | 79 |
| 6.2 | Column | |
| 6.3 | Radius of gyration (k) | 80 |
| 6.4 | Slenderness ratio: (l) | 80 |
| 6.5 | Long column | |
| 6.6 | Short column | |
| 6.7 | Crushing load | 81 |
| 6.8 | Crippling load or Buckling load or critical load | |
| 6.9 | Column end conditions and effective length | 82 |
| 6.10 | Euler's formula for crippling load | 83 |
| 6.11 | Assumptions of Euler's formula | |
| 7 : Principa | al Planes and Principal stresses84 to | |
| . 7.1 | Principal plane | |
| 7.2 | Principal stress | |
| 7.3 | Different cases of stresses in materials | 85 |

| 7 | 7.4 | Equations of s _n , s _t , s _r | Ì |
|----------|-----------|---|---------|
| 7 | 7.5 | Sign conventions | ı |
| 7 | 7.6 | Principal planes and principal stresses | |
| 8 : Con | nbin | ed Direct and Bending Stress 89 to 96 | ı |
| { | 8.1 | Axial load and eccentric load 89 | ١ |
| 8 | 8.2 | Stress due to eccentric load90 | |
| { | 8.3 | Stress distribution in column | |
| | 8.4 | Limit of eccentricity | · |
| | 8.5 | Core of section or kernal92 | : |
| | 8.6 | Maximum and Minimum Pressure at the base of Dam95 | ; |
| 9 : Thi | n Cy | lindrical Shells 97 to 97 | ′ |
| | 9.1 | For thin cylindrical shell | 1 |
| | 9.2 | Thin spherical shell | 7 |
| 10 : To | orsio | n and Springs98 to 149 | ' |
| | 10.1 | Terms related to torsion | } |
| | 10.2 | Equation of torsion |) |
| | 10.3 | Assumptions in the theory of torsion |) |
| | 10.4 | Important Equations |) |
| | 10.5 | Equations of power |) |
| | 10.6 | Shaft subjected to combined Bending and Torsion |) |
| | 10.7 | 101 | |
| Annris | 10.8 | Leaf spring or Carriage spring103 | |
| | | MCQ'S | |
| | | Answers | 4 |
| | | Explanations | 5 |
| | | | |
| CHAPTER | - 3 —– | : STRUCTURAL ANALYSIS150 TO 17. | <i></i> |
| | Ans | wers | 7 |
| <u>'</u> | Exp | lanations | 8 |
| | • | | |

| | |
|-------------|--|
| CHAPTER | 2 - 4 : CONCRETE TECHNOLOGY |
| 1 | Short Answer Questions |
| | MCQ'S |
| CHAPTER | - 5: DESIGN OF RCC STRUCTURES213 to 229 |
| 1 | Short Questions with Answers |
| | MCQ'S |
| | Explanations |
| CHAPTER | - 6 : DESIGN OF STEEL STRUCTURES230 to 238 |
| CHAPTER | - 7 : SOIL MECHANICS AND FOUNDATION |
| | ENGINEERING |
| | MCQ'S |
| | Answers |
| | Explanations |
| CHAPTER | - 8 : HIGHWAY ENGINEERING 279 to 293 |
| | MCQ'S |
| | Answers |
| CHAPTER | - 9 : BRIDGE ENGINEERING 294 to 296 |
| CHAPTER | - 10: RAILWAY ENGINEERING 297 to 307 |
| hhlia | MCQ'S |
| | Answers |
| CHAPTER | - 11 : AIRPORT ENGINEERING308 to 312 |
| | MCQ'S |
| | Answers |

| CHAPTER - 19 : BUILDING CONSTRUCTIONS | 418 to 428 |
|---|------------|
| MCO'S | 418 |
| Answers | 428 |
| | <u> </u> |
| CHAPTER - 20 : CONSTRUCTION MANAGEMENT AND | 420 4- 426 |
| EQUIPMENTS | 429 to 430 |
| MCQ'S | 429 |
| Answers | 436 |
| | |
| CHAPTER - 21 : EARTHQUAKE ENGINEERING | 437 10 440 |
| Short Answer Questions | 437 |
| True/False | 442 |
| CHAPTER - 22: WATER SUPPLY AND SANITRARY | |
| CHAPTER - 22: WATER SUPPLY AND SANTIALITY ENGINEERING | 447 to 499 |
| | |
| MCQ's | 447 |
| (A) Water Supply Engineering | |
| (B) Sanitary Engineering | |
| CHAPTER - 23 : BUILDING PLANNING | 500 to 503 |
| MCQ's | |
| | |
| GATE SYLLABUS | 504 10 500 |
| GATE EXAMINATION PAPER | 507 to 624 |
| Gate 2014 Paper-1 | 50 |
| Gate 2014 Paper-2 | 535 |
| Gate 2015 Paner-1 | 56 |
| Gate 2015 Paper-2 | 59 |
| \(\lambda \(\lambda \) | |
| ▼ ▼ | |

| CHAPTER - 12 : DOCKS AND HARBOUR | 313 to 316 |
|--|------------|
| MCQ'S | 313 |
| Answers | 316 |
| CHAPTER - 13 : ESTIMATING AND COSTING | 317 to 327 |
| Useful Information | 317 |
| Short Answer Questions | 318 |
| CHAPTER -14: IRRIGATION ENGINEERING | 328 to 342 |
| MCQ'S | 328 |
| Answers | |
| CHAPTER - 15 : HYDROLOGY AND WATER RESOURCES | |
| ENGINEERING | 343 to 359 |
| MCQ'S | 343 |
| Explanations | • |
| | |
| CHAPTER - 16 : SURVEYING | 360 to 382 |
| MCQ'S | |
| Explanations | 381 |
| CHAPTER - 17 : FLUID MECHANICS | 383 to 408 |
| MCQ'S | 383 |
| Answers | |
| Explanations | |
| CHAPTER - 18 : BUILDING MATERIALS | 409 to 417 |
| MCQ'S | 409 |

1: Introduction

1.1 ENGINEERING MECHANICS:

Engineering mechanics is the branch of applied science, which deals with the laws and principles of mechanics along with their applications to engineering problems.

Engineering Mechanics



Statics

- It deals with the forces and their effects, while acting upon the bodies at rest.

Dynamics

It deals with the forces and their effects, while acting upon the bodies in motion.

Kinetics

It deals with the bodies in motion - It deals with the bodies in motion due to the application of forces.

Kinematics

without any reference to the forces which are responsible for the motion.

1.2 SCALAR AND VECTOR QUANTITIES:

Scalar quantity: A scalar quantity is one which can be completely specified by it's magnitude only. For example

length,

Area

mass,

volume

time,

density

distance,

speed

temprature,

work

energy

moment of Inertia, etc.

Vector quantity: A vector quantity is one which requires magnitude and direction both to completely specify it. For example,

displacement,

Force

Objective Civil Eng. \ 2016 \ 1

Objective Civil Engineering

2

velocity,

Weight

acceleration,

Angular velocity

momentum,

Angular acceleration

moment,

Impulse, etc.

1.3 FUNDAMENTAL UNITS:

All the physical quantities used in engineering mechanics are expressed in terms of three fundamental quantities, i.e.

1. Length

2.

Mass

3. Time

The units of these fundamental quantities are called fundamental units or base units. Derived units: The units of some physical quantities are derived from fundamental units. Such units are called derived units. e.g.

Units of area, velocity, acceleration etc.

1.4 CONVERSION OF UNITS:

$$1m = 100 \text{ cm}$$

$$1m = 1000 \text{ mm}$$

$$1 \text{ cm} = 10 \text{ mm}$$

$$1 \text{ km} = 1000 \text{ m}$$

$$1 \text{ cm}^2 = 100 \text{ mm}^2$$

$$1 \text{ m}^2 = 10^6 \text{ mm}^2$$

$$1 \text{ MPa} = 1 \text{ N/mm}^2$$

$$1 \text{ GPa} = 10^3 \text{ N/mm}^2$$

$$1 \text{ N.m} = 1 \text{ Joule}$$

$$1 \text{ Watt} = 1 \text{ N.m/s} = 1 \text{ J/S}$$

$$1 \text{ kg f} = 9.81 \text{ N} \simeq 10 \text{ N}$$

$$1 \text{ kN} = 10^3 \text{ N}$$

$$1 \text{ MN} = 10^6 \text{ N} = 10^3 \text{ kN}$$

$$1 \text{ GN} = 10^9 \text{ N}$$

$$1 \text{ kW} = 10^3 \text{ watt}$$

$$1 \text{ hp} = 746 \text{ watt}$$

1 degree =
$$\frac{\pi}{180}$$
 radian

IMPORTANT SHORT QUESTIONS

- 1. Speed is a quantity.
- 3. What is S.I. unit of power?
- 5. $1 \text{ kg} = \dots N$
- 7. The system of units used internationally is
- 9. 1 Pascal =

- 2. Which are the fundamental quantities?
 - 4. What is unit of density?
- 6. $1 \text{ MN} = \dots N$
- The unit of force in S.I. units is
- 10. The branch of engineering mechanics dealing with study of motion of a body, causes of motion are not studied is called

ANSWERS

| [| 1. | Scalar | 2. | Length, mass, time | 3. | Watt | 4. | kg/m ³ | 5. 9.81 |
|---|----|-----------------|----|--------------------|----|--------|----|--------------------|-----------------|
| | 6. | 10 ⁶ | 7. | S.I. system | 8. | Newton | 9. | 1 N/m ² | 10. Kinematics. |



2: Coplanar Concurrent Forces

2.1 FORCE:

An agent which produces or tends to produce, destroys or tends to destroy motion of a body is called force.

Unit of force is Newton (N).

Force is a vector quantity.

1 N force: A force which can produce an acceleration of 1 m/s^2 in a mass of 1 kg is called 1 N Force.

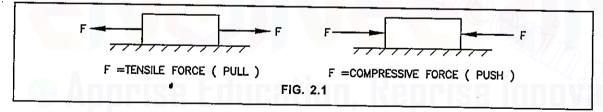
1 kgf force: A force which can produce an acceleration of g m/s² (gravitational acceleration) in a mass of 1 kg is called 1 kgf force.

• Characteristics of a force:

Followings are the characteristics of a force:

- (1) Magnitude: Magnitude of a force may be 10 kN, 50 kN, 200 N etc.
- (2) Direction: i.e. along OX,
 - towards east
 - at 30° West of North
- (3) Nature: The nature of force may be tensile or compressive.
- (4) Point of application:

The point at which the force acts on the body is called the point of application.



2.2 WEIGHT AND MASS:

Weight (W): The force by which the body is attracted towards the centre of the earth is called Weight (W) of the body.

Weight is a vector quantity.

Unit of weight is N, kN, kgf.

Mass (m): The matter contained in the body is called mass.

Mass is a scalar quantity.

Unit of mass is kg, tonne.

2.3 SYSTEM OF FORCES:

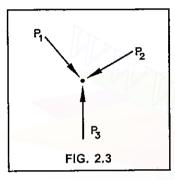
When two or more forces act on a body, they are called to form a System of forces.

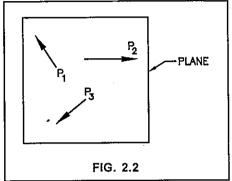
Following systems of forces are important.

(1) Coplanar forces:

The forces whose line of action lie on the same plane, are known as **coplanar forces**.

Here, forces P₁, P₂, P₃ are coplanar forces.





(2) Concurrent forces:

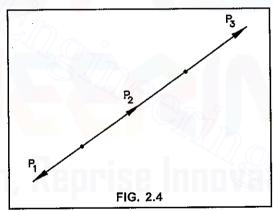
The forces which meet at one point, are known as concurrent forces.

Forces P₁, P₂, P₃ are concurrent forces.

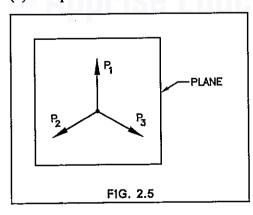
(3) Collinear forces:

The forces whose lines of action lie on the same line, are known as collinear forces.

Forces P₁, P₂, P₃ are collinear forces.



(4) Coplanar concurrent forces:



The forces which meet at one point and their lines of action also lie on the same plane are known as coplanar concurrent forces.

Forces P₁, P₂, P₃ are coplanar concurrent forces.

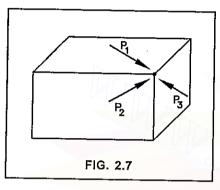
5

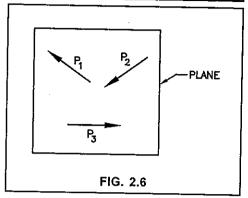
(5) Coplanar non-concurrent forces:

The forces whose lines of action lie on the same plane but they do not meet at one point are known as coplanar non-concurrent forces.

Forces P_1 , P_2 , P_3 are coplanar non-concurrent forces.

(6) Non-coplanar concurrent forces:





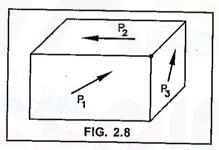
The forces whose lines of action do not lie on the same plane, but they meet at one point are known as Non-coplanar concurrent forces.

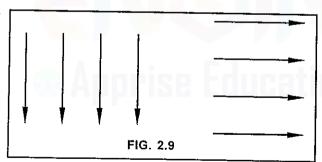
Forces P₁, P₂, P₃ are non-coplanar concurrent forces.

(7) Non-coplanar non-concurrent forces:

The forces whose lines of action do not lie on the same plane and they do not meet at one point are known as non-coplanar non-concurrent forces.

Forces P₁, P₂, P₃ are non-coplanar non-concurrent forces.



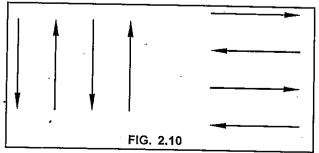


(8) Like parallel forces:

The forces, whose lines of action are parallel to each other and all of them act in the same direction are known as like parallel forces.

(9) Unlike parallel forces:

The forces, whose lines of action are parallel to each other but all of them do not act in the same direction are known as unlike parallel forces.



2.4 RESULTANT FORCE:

If a number of forces P_1 , P_2 , P_3 , P_4 ... etc. are acting simultaneously on a particle, it is possible to find out a single force which could repiace them i.e. which would produce the same effect as produced by all the given forces. This single force is called **resultant force**, and the given forces P_1 , P_2 , P_3 , P_4 ... etc. are called component foces.

Equilibrant force:

To balance the resultant force (to bring the body in equilibrium), a force of same magnitude but of opposite direction is required. This opposite balancing force is called equilibrant-force.

rigid body: A rigid body may be defined as a body which can retain its shape and size, even if subjected to some external forces. In actual practice, no body is perfectly rigid. But for the sake of simplicity, we consider the body as a rigid body.

2.5 PARALLELOGRAM LAW OF FORCES:

It states,

"If two forces, acting simultaneously on a particle, be represented in magnitude and direction by the two adjacent sides of a parallelogram; their resultant may be represented in magnitude and direction by the diagonal of the parallelogram which passes through their point of intersection."

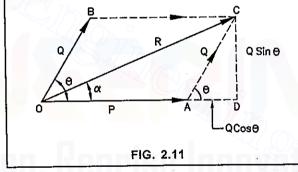
$$R = \sqrt{P^2 + Q^2 + 2PQ\cos\theta}$$

$$\tan \alpha = \frac{Q \sin \theta}{P + Q \cos \theta}$$

where, R = Resultant force

 θ = angle between P and Q

 α = angle between P and R.

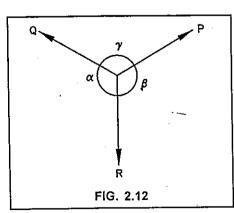


2.6 LAMI'S THEOREM:

"If three coplanar forces acting at a point be in equilibrium, then each force is proportional to the sine of the angle between other two forces."

Mathematically,

$$\frac{P}{\sin\alpha} = \frac{Q}{\sin\beta} = \frac{R}{\sin\gamma}$$



7

2.7 TRIANGLE LAW OF FORCES:

"If two forces acting at a point be represented in magnitude and direction by two sides of a triangle taken in order, their resultant may be represented in magnitude and direction by the third side of the triangle, taken in opposite order."

For example,

Consider two forces P = 40 N, Q = 30 N acting at right angle to eachother.

R = Resultant force = ac
=
$$5 \text{ cm} \times 10$$

= 50 N

Q = Angle of R with P

2.8 POLYGON LAW OF FORCES:

"If a number of forces acting at a point be represented in magnitude and direction, by the sides of a polygon taken in order, then the resultant of all these forces may be represented, in magnitude and direction by the closing side of the polygon taken in opposite order."

Consider a system having four forces acting at a point.

2.9 RESULTANT OF MORE THAN TWO CONCURRENT FORCES:

Consider P_1 , P_2 , P_3 , P_4 ... etc. are acting at a point as shown in figure.

 $\Sigma H = Algebric sum of horizontal forces.$

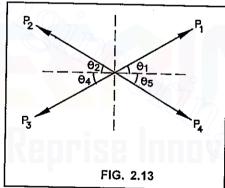
 $\Sigma V = Algebric sum of vertical forces.$

R = Resultant of all forces

$$\therefore R = \sqrt{(\Sigma H)^2 + (\Sigma V)^2}$$

$$\tan\theta = \frac{\Sigma V}{\Sigma H},$$

 θ = angle of resultant with horizontal



IMPORTANT SHORT QUESTIONS

- 1. List the characteristics of a force.
- 2. What is relation between weight and mass?
- 3. The resultant of two concurrent tensile forces is maximum, when angle between them is
- 4. The resultant of two concurrent tensile forces is minimum, when angle between them is

Objective Civil Engineering

8

- 5. The conditions of equilibrium for coplanar concurrent forces are
- 6. A 20 kN force is acting vertically upward. Its horizontal component is equal to
- 7. What is equilibrant force?
- 8. For a coplanar concurrent force system if $\Sigma H = +ve$ and $\Sigma V = -ve$, the resultant force lies in the
- 9. The statement, "the effect of a force upon a body is the same at every point on its line of action" refers to
- 10. A body isolated from all other members which are attached to it is called the

ANSWERS

- 1. Magnitude, direction, nature, point of application.
- 2. $W = m \cdot g$
- 3. 0°
- 4. 180°
- 5. $\Sigma H = 0$, $\Sigma V = 0$, $\Sigma R = 0$.
- 6. Zero
- 7. A force of same magnitude but of opposite to the resultant force is called equilibrant force.
- 8. Fourth quadrant.
- 9. Principle of transmissibility.
- 10. Free body.



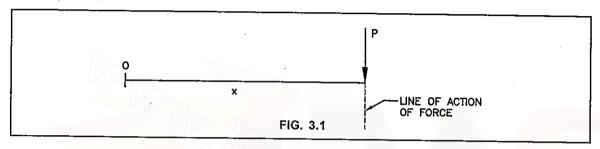
3. Coplanar non-concurrent forces

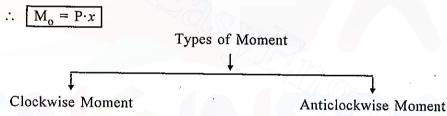
3.1 MOMENT:

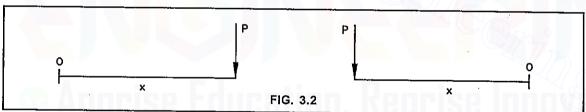
Moment of a force at any point is defined as the product of a force and perpendicular distance of line of action of force from that point.

:. Moment = Force × perpendicular distance

Unit of moment is N.m or kN.m







$$M = P \cdot x$$

- It rotates the body in a clockwise direction.
- Normally, clockwise moment is considered positive (+ve).

$M = P \cdot x$

- It rotates the body in anticlockwise direction.
- Normally, anticlockwise moment is considered negative (-ve).

• Applications of moment:

- 1. To open or close a door.
- 2. To tight nut using spanner.
- 3. To rotate steering wheel of car.

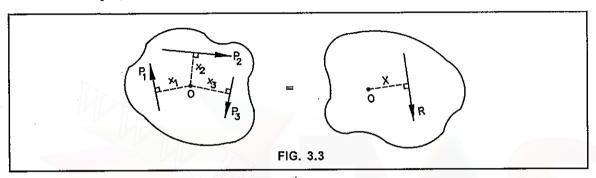
Objective Civil Eng. \ 2016 \ 2

3.2 VARIGNON'S PRINCIPLE OF MOMENTS:

It states,

"If a number of coplanar forces are acting simultaneously on a particle, the algebric sum of the moments of all the forces about any point is equal to the moment of their resultant force about the same point."

For example,



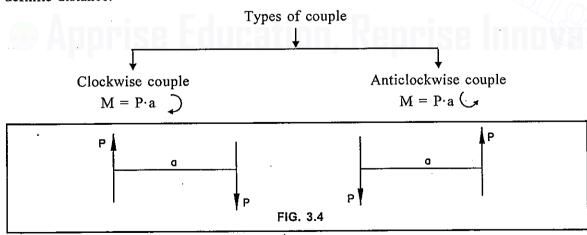
body subjected to three coplanar forces Moment of all the forces about O, $= P_1x_1 + P_2x_2 + P_3x_3$ body subjected to resultant of three forces. Moment of resultant force about O, = $R \cdot x$

.. According to Varignon's principle,

$$P_1 x_1 + P_2 x_2 + P_3 x_3 = R \cdot x$$

3.3 COUPLE:

A couple is defined as a system of two equal and opposite forces separated by a definite distance.



It rotates the body in a clockwise directin. It is considered as positive (+ve).

It rotates the body in anticlockwise direction. It is considered as negative (-ve).

11

Arm of couple: The perpendicular distance between two forces of a couple is called arm of couple.

Moment of couple:

Moment of couple = force \times arm of couple

$$M = P \times a$$

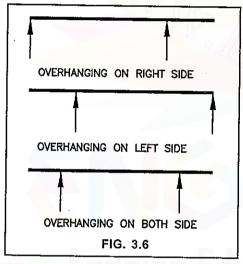
Unit of moment of couple is N.m. or kN.m.

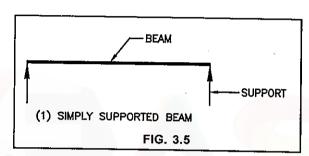
3.4 TYPES OF BEAMS:

Different types of beams are shown below.

(1) Simply supported beam:

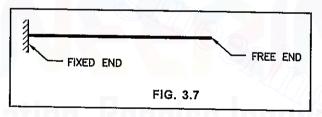
(2) Overhanging beam:





(3) Cantilever beam:

It has one end fixed and other end free.



FIXED END
FIG. 3.8

(4) Fixed beam:

It has both ends fixed.

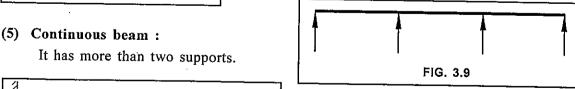


FIG. 3.10

(6) Propped cantilever beam:

12

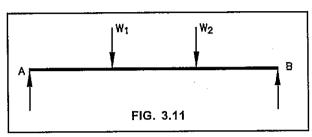
3.5 TYPES OF LOADS:

There are three types of loads.

(1) Point load or concentrated load:

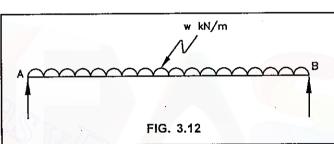
W₁ and W₂ are point loads.

- The load concentrated at one point is called point load.
- Unit of point load is N or kN.
 e.g. 20 kN, 100 N, 60 kN etc.



(2) Uniformly distributed load.

- Load uniformly distributed on certain length of a beam is called uniformly distributed load.
- It is written as u.d.l.
- It is shown by w.
- Unit of u.d.l. is kN/m or N/m



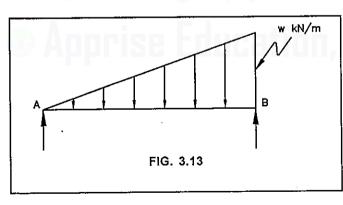
For example,

20 kN/m, means 20 kN load on 1m length.

If span of beam is 4 m,

total load = $20 \times 4 = 80 \text{ kN}$

(3) Uniformly varying load:



This type of load gradually increase or decrease on the length of the beam.

It is also called triangular load.

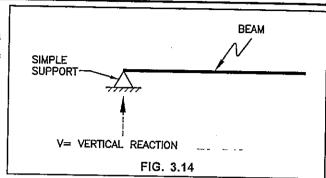
3.6 TYPES OF SUPPORTS:

Different types of supports are:

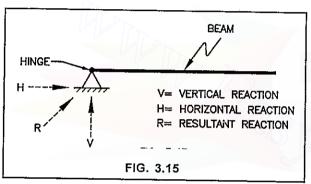
- 1. Simple support
- 3. Roller support
- 2. Hinged support
- 4. Fixed support

(1) Simple support:

- In this type of support, beam is simply supported on the support.
- There is no connection between beam and support.
- At this type of support, only vertical reaction will be produced.



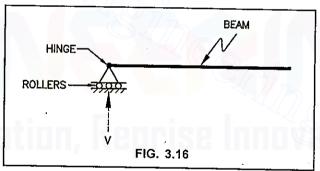
(2) Hinged support:



- Beam and support are connected by a hinge.
- Beam can rotate about the hinge.
- Reactions may be vertical, horizontal or inclined.

(3) Roller support:

- In this type of support, rollers are placed below beam and beam can slide over the rollers.
- Reaction will be perpendicular to the surface on which rollers are supported.
- This type of support is normally provided at the end of a bridge.



Due to breaking forces of vehicles and temperature forces, bridge slab can slide over the roller support and damage to bridge pier can be avoided.

M= MOMENT FIG. 3.17

(4) Fixed support:

- Beam is completely fixed at end in the wall or support.
- Beam can not rotate at end.
- Reactions may be vertical, horizontal, inclined and moment.

13

14

IMPORTANT SHORT QUESTIONS

- 1. Moment is equal to
- 2. The unit of couple is
- 3. Give two examples of moment.
- 4. Give two examples of couple.
- 5. Resultant force of couple is
- 6. Principle of moment is based on theorem.
- 7. A couple is formed by
- 8. If three equal forces 150 kN each are acting along three sides of triangle in clockwise direction, their resultant is
- 9. The support normally provided at the end of a bridge is
- 10. At fixed support, the possible reactions are,
- 11. The reaction at the roller support of a beam is always,

ANSWERS

- 1. Force × Perpendicular distance of line of action of force.
- 2. N.m or kN.m
- 3. (i) To open or close the door.
 - (ii) To tight the nut by spanner.
- 4. (i) To rotate the steering wheel of car by two hands.
 - (ii) To rotate key in the lock.
- 5. Zero
- 6. Varignon's
- 7. Two equal, opposite and non-collinear forces
- 8. Zero
- 9. Roller support
- 1 Vertical (V), Horizontal (H), Moment (M)
- 11. Vertical



4. Centroid and Centre of Gravity

4.1 CENTROID:

In plane figures like square, rectangle, triangle, circle etc., the point at which whole area of the figure is concentrated, is called **centroid**.

Plane figures do not have mass.

They are two-dimensional.

4.2 CENTRE OF GRAVITY (C.G.):

In solid bodies like cube, cuboid, cone, sphere etc., the point at which whole mass of the body is concentrated, is called **centre of gravity (c.g.)**.

In solid bodies gravitational force act on c.g.

• Difference between centroid and C.G. :

| Centroid | Centre of gravity | | |
|---|--|--|--|
| (i) In case of plane figures the point at which whole area of the figure is concentrated is called centroid. (ii) Centroid word is used for 2-D figures. For example, Square, Rectangle, Triangle, Circle etc. | (i) In case of solid bodies the point at which whole mass is concentrated is called c.g. (ii) Centre of gravity word is used for 3-D figures. For example, cube, cuboid, cone, sphere etc. | | |

4.3 CENTROIDS OF STANDARD SHAPE: A . ONE DIMENSIONAL (WIRES)

| Sr.No | Geometrical Shape | Length | x | У | |
|-------|--|--------------|---|------------------------------|--|
| 1 | G L A Straight Wire AB | L | Centre of (L/ | | |
| 2 | G y Wire ring | 2πr | Centre of Circle (r) \$\overline{x} = r\$ \$\overline{y} = r\$ | | |
| 3 | Semicircular Wire AB | πr | | $\frac{2r}{\pi}$ | |
| 4 | Quarter-circular Wire AB | <u>π</u> r | <u>2r</u> π | <u>2r</u> π | |
| 5 | $ \begin{array}{c cccc} & & & & & & \\ \hline & & & \\ \hline & & & & $ | 2αr α:rad | $\frac{r \sin \alpha}{\alpha}$ α = angle in radians | On axis of Symmetry | |

B. TWO DIMENSIONAL FIGURES

| Sr.No | Geometrical Shape | Area | × | प्र |
|-------|--|---|------------------|----------------------------------|
| 1 | X \overline{\overline{\begin{array}{c} \overline{\begin{array}{c} \overline{\overline{\begin{array}{c} \overline{\begin{array}{c} \overline{\ | A=B.D | B <u>2</u> | <u>D</u> 2 |
| 2 | X y b b X Y Y Y Y Y Y Y Y Y Y Y Y Y Y Y Y Y | A= ¹ / ₂ b.h | <u>1</u> 3b | <u>1</u> 3h |
| 3 | X h G X Y Trapezium | A=(a+b) <u>h</u> | <u>b</u> 2 | <u>h</u> (<u>b+2a</u>) 3 (b+a) |
| .4 | X G Y X Circle | $A=\pi r^{2}$ OR $A=\frac{\pi}{4}d^{2}$ | | L. |
| 5 | X O IV | $A = \frac{\pi r^2}{2}$ | E III | <u>4.г.</u> उन्ह |
| 6 | y C Circle | $A = \frac{\pi}{4} r^2$ | 4 <u>r</u> 3π | 4r 3π |
| 7 | O G G G X Circular Sector | A=αr² α:rad | 2 <u>r sin α</u> | On axis of Symmetry |

Objective Civil Eng. \ 2016 \ 3

C. THREE DIMENSIONAL FIGURES (SOLIDS)

| Sr.No | Solid | Volume | × | у : |
|-------|---------------------------------------|------------------------|---|----------------|
| 1 | h G y X Cylinder | V=πr ² h | r | <u>h</u> 2 |
| 2 | Cone | $V=\frac{\pi}{3}r^2h$ | r | h 4 |
| 3 | radius=r G Sphere | $V=\frac{4}{3}\pi r^3$ | | |
| 4 | □ □ □ □ □ □ □ □ □ □ □ □ □ □ □ □ □ □ □ | $V=\frac{2}{3}\pi r^3$ | r | <u>3r</u> 8 |

IMPORTANT SHORT QUESTIONS

- 1. Centroid is defined as a point about which
- 2. Centre of gravity is defined as a point about which
- 3. The centroid of semi-circular area lies at distance of from base along the vertical axis.
- 4. The centroid of semi-circular wire lies at distance of from base along the vertical axis.
- 5. The C.G. of hemisphere lies at a distance of from its base along the vertical axis.
- 6. For T-section having flange 60 \times 10 mm and web 10 \times 60 mm, $\bar{x} = \dots$
- 7. What is symmetry of L-section?
- 8. The C.G. of a right circular cone of diameter (d) and height (h) lies at a distance of from the base measured along vertical axis.
- 9. The C.G. of equilateral triangle with each side (a) is from any of the three sides.
- 10. If the area is symmetrical about y-axis, the centroid lies on axis.

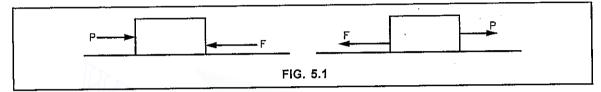
ANSWERS

- 1. The entire line, area or volume is assumed to be concentrated.
- 2. The entire weight (or mass) of the body is assumed to be concentrated.
- 3. $\frac{4 \text{ r}}{3 \pi}$
- 4. $\frac{2 r}{\pi}$
- 5. $\frac{3 \, \mathrm{r}}{8}$
- 6. 30 mm
- 7. L-section is not symmetrical about any axis.
- 8. $\frac{h}{4}$
- 9. $\frac{a}{2\sqrt{3}}$
- 10. y-axis.

5. Friction

5.1 FRICTION OR FRICTION FORCE:

When a body slide or tends to slide on a surface on which it is resting, a resisting force opposing the motion is produced at the contact surface. This resisting force is called **friction** or **friction force**.



P = external force

F = Friction force

- ⇒ Friction force (F), always act in the direction opposite to the movement of the body.
- ⇒ If contact surfce is smooth, friction force (F) will be less.

 If contact surface is rough, friction force (F) will be more.
- ⇒ In machine parts like piston, bearings, liner etc. attempts are made to reduce friction to increase life of parts and efficiency of machine.
- ⇒ Outer surface of vehicle tyres is made rough to increase friction between tyre and road surface for safe driving.

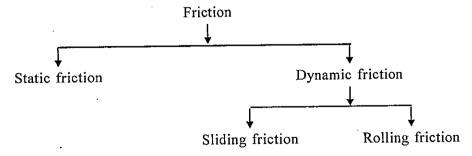
5.2 LIMITING FRICTION:

Consider a body resting on the surface. If a little force is applied on the body, it will not move, because friction force (F) will resist the motion.

With further increase in external force, body will not move. But, there is a limit of developing friction force. If external force becomes greater than friction force, body will move.

The maximum friction force that can be developed at the contact surface, when body is just on the point of moving is called limiting friction.

5.3 TYPES OF FRICTION:



21

• Static friction: Friction experienced by a body when it is at rest is called static friction.

In case of static friction,

$$P < F$$
 $P = external force$
 \therefore there is no motion $P = friction force$

• Dynamic friction:

Friction experienced by a body, when it is in motion is called **dynamic friction**. Dynamic friction is always less than static friction.

• Sliding friction: Friction experienced by a body when it slides over another body, is called sliding friction.

e.g. to push the table on floor.

• Rolling friction: Friction experienced by a body, when it rolls over another body, is called rolling friction.

e.g. - to move the drum by rolling.

- Ball bearing used in machines.

5.4 ANGLE OF FRICTION: 6

W = Weight of block

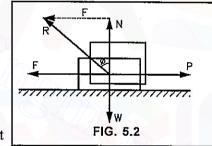
N = Normal reaction

P = external force

F = Friction force

R = Resultant of N and F.

The angle between normal reaction (N) and resultant force (R) is called **angle of friction**.



It is also called **limiting angle of friction.** The value of ϕ is more for rough surface as compared to smooth surface.

5.5 COEFFICIENT OF FRICTION : μ

Limiting friction (F) is proportional to the normal reaction (N).

$$\begin{array}{ccc} \therefore & F & \alpha & N \\ \therefore & F & = & \mu \cdot N \\ \therefore & \mu = \frac{F}{N} & \dots & (i) \end{array}$$

The ratio of Limiting fricion (F) and Normal reaction (N) is called coefficient of friction.

from figure 5.2,

$$tan \phi = \frac{F}{N}$$
 ... (ii)

From equation (i) & (ii),

$$\therefore \mu = \tan \phi$$

5.6 ANGLE OF REPOSE:

With increase in angle of the inclined surface, the maximum angle at which, body starts sliding down is called angle of repose.

Consider a body of weight W, is resting on the plane inclined at angle α with horizontal.

Weight (W) has two components,

Component parallel to the plane = W sin α

Component perpendicular to the plane = $W \cos \alpha$

Resolve 1 to plane,

Resolve | to plane

$$\therefore$$
 N = W cos α

$$\therefore F = W \sin \alpha$$

$$\therefore \mu = \frac{F}{N} = \frac{W \sin \alpha}{W \cos \alpha} = \tan \alpha \dots (i)$$

We know that,

$$\mu = \tan \phi ... (ii)$$

From equation (i) and (ii),

$$\tan \alpha = \tan \phi$$

Where, α = angle of inclined plane

$$\alpha = \phi$$

Hence, when the angle of inclined plane (α) , becomes equal to the limiting angle of friction (ϕ) , body will slide down the plane.

$$\therefore$$
 Angle of repose = angle of friction = ϕ

5.7 LAWS OF STATIC FRICTION:

Following are the laws of static friction.

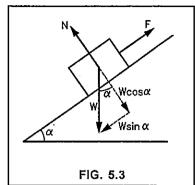
- 1. The friction force always act in a direction, opposite to that in which the body tends to move.
- 2. The magnitude of friction force is equal to the external force.

$$\therefore F = P$$

3. The ratio of limiting friction (F) and normal reaction (N) is constant.

$$\therefore \frac{F}{N} = \mu = constant.$$

- 4. The friction force does not depends upon the area of contact between the two surfaces.
- 5. The friction force depends upon the roughness of the surfaces.



23

IMPORTANT SHORT QUESTIONS

- 1. Maximum value of static friction is known as
- 2. Friction force acts in direction to the motion of the body.
- 3. Dynamic friction is also known as
- 4. Angle of repose is always equal to
- 5. The ratio of limiting friction (F) and normal reaction (N) is known as
- 6. The value of μ_k is found to be than μ_s for the same pair of contact surfaces.
- 7. Dry friction is also called
- 8. The coefficient of friction μ and angle of repose ϕ are related by the expression
- 9. The friction force depends upon the of the surfaces.
- 10. When a body is subjected to force P, and it is at rest, the relation between P and F is

ANSWERS

- 1. Limiting friction
- 2. Opposite
- 3. Kinetic friction
- 4. Angle of friction
- 5. Coefficient of friction (μ)

- 6. less
- 7. Coulomb friction
- 8. $\mu = \tan \phi$
- 9. roughness
- 10. P < F



24

6. Simple Lifting Machines

6.1 TECHNICAL TERMS RELATED TO SIMPLE MACHINES:

1. Mechanical advantage (MA):

The ratio of load lifted (W) and effort required (P) is called Mechanical advantage.

$$MA = \frac{load\ lifted}{effort\ required}$$

$$\therefore MA = \frac{W}{P}$$

$$P = effort$$

2. Velocity ratio (VR):

The ratio of distance moved by effort and the distance moved by load is called velocity ratio.

$$VR = \frac{\text{distance moved by effort}}{\text{distance moved by load}}$$

$$\therefore VR = \frac{y}{x}$$

3. Input:

Input = effort × distance moved by effort

$$\therefore \text{Input} = P \cdot y$$

4. Output:

Output = load × distance moved by load

$$\therefore \text{ Output = } \mathbf{W} \cdot \mathbf{x}$$

5. Efficiency (η):

The ratio of work done by the machine and work done on the machine is called efficiency of the machine.

Efficiency =
$$\frac{\text{output}}{\text{input}} \times 100\%$$

We know that

output =
$$W \cdot x$$

input =
$$P \cdot y$$

$$\therefore \quad \eta = \frac{\text{output}}{\text{input}} \times 100$$

$$= \frac{W \cdot x}{P \cdot y} \times 100$$

$$=\frac{\frac{W}{P}}{\frac{y}{x}} \times 100$$

$$\eta = \frac{MA}{VR} \times 100\%$$

$$\therefore \eta = \frac{\text{output}}{\text{input}} \times 100\% = \frac{\text{MA}}{\text{VR}} \times 100\%$$

6. Ideal machine:

A machine having 100% efficiency is called an ideal machine. In an Ideal machine friction is zero. For Ideal machine,

Output = Input

$$MA = VR$$

7. Effort lost in friction (P_f):

In a simple machine, effort required to overcome the friction between various parts of a machine is called effort lost in friction.

let, P = effort

P₀ = effort for Ideal machine

P_f = effort lost in friction

: effort lost in friction.

$$P_f = P - P_o$$

For Ideal machine

$$VR = \frac{W}{P_0}$$

$$VR = \frac{W}{P_o}$$
 $\therefore P_o = \frac{W}{VR}$

$$\therefore P_f = P - P_o$$

$$\therefore P_{f} = P - \frac{W}{VR}$$

For Ideal machine

$$MA = VR$$

$$P_o = \frac{W}{VR} = ideal effort$$

due to friction, $P > P_0$

8. Friction load (W_f):

Total friction force produced when machine is in motion is called friction load.

W_o = load for Ideal machine

Objective Civil Eng. \ 2016 \ 4

$$P = effort$$

For Ideal machine,

$$VR = \frac{W_o}{P}$$

$$\therefore W_o = P \times VR$$

Now,

Friction load,

$$W_f = W_o - W$$

$$W_f = (P \times VR) - W$$

∵ for ideal machine

$$MA = VR$$

$$W_0 = P \times VR = ideal load$$

9. Reversible machine:

If a machine is capable of doing some work in the reverse direction, after the effort is removed, is called reversible machine.

For reversible machine, $\eta \geq 50\%$

10. Non-reversible machine or self-locking machine:

If a machine is not capable of doing some work in the reverse direction, after the effort is removed, is called non-reversible machine or self-locking machine.

For non-reversible machine, $\eta < 50\%$

11. Condition for reversibility of machine:

W = load lifted

P = effort required

x =distance moved by load

y = distance moved by effort

 $P \cdot y = input$

 $W \cdot x = \text{output}$

Machine friction = $P \cdot y - W \cdot x$

for a machine to reverse,

output > machine friction

$$\therefore W \cdot x > P \cdot y - W \cdot x$$

$$\therefore$$
 2 W·x > P·y

$$\therefore \quad \frac{\mathbf{W} \cdot \mathbf{x}}{\mathbf{P} \cdot \mathbf{y}} \ge \frac{1}{2}$$

$$\therefore \quad \frac{\text{Output}}{\text{Input}} = 0.5$$

$$\therefore$$
 $\eta \geq 50\%$

 \therefore For a machine to reverse, $\eta \geq 50\%$

Applied Mechanics

27

12. Law of machine:

For a particular machine, if we record various values of effort required to lift the corresponding loads and plot a graph between effort and load, we shall get a straight line AB as shown in figure.

Mathematically, the law of machine is given by relation:

P = mW + C

where,

P = effort applied

W = load lifted

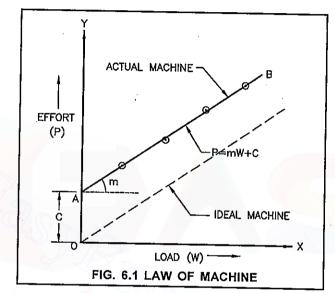
m = constant

(coefficient of friction)

= slope of line AB

C = constant

= machine friction



Following observations are made from the graph:

- 1. On a machine, if W = 0, effort C is required to run the machine. Hence, effort C is required to overcome machine friction.
- 2. If line AB crosses x-x axis, without effort (p), some load can be lifted, which is impossible. Hence, line AB never crosses x-x axis.
- 3. If line AB passes through origin, no effort is required to balance friction. Such a graph is for Ideal machine.

13. Maximum mechanical advantage:

We know that,

$$MA = \frac{W}{P}$$

from law of machine P = mW + C

$$\therefore MA = \frac{W}{mW + C}$$

$$= \frac{1}{m + \frac{C}{W}}$$
neglecting $\frac{C}{W}$

$$Maxi, MA = \frac{1}{m}$$

28

14. Maximum efficiency (η_{max}) :

We know that,

$$\eta = \frac{MA}{VR}$$

$$\vdots \qquad \eta = \frac{\frac{1}{m}}{VR}$$

$$\vdots \qquad \eta_{max} = \frac{1}{m \times VR}$$

· Substitute

$$MA = MA \max = \frac{1}{m}$$

6.2 SYSTEMS OF PULLEY:

3.

There are main three systems of Pulley

1. First system of pulley: $VR = 2^n$

where, n = no. of moving Pulley

2. Second system of pulley: VR = n

where, n = total no. of Pullies. Third system of pulley: $VR = 2^n - 1$

where, n = total no. of Pullies.

6.3 DIFFERENT SIMPLE MACHINES AND THEIR VELOCITY RATIO:

(1) Simple wheel and axle:

$$VR = \frac{D}{d}$$

D = diameter of wheel

d = diameter of axle

(2) Simple wheel and differential axle:

$$VR = \frac{2D}{d_1 - d_2}$$

D = dia. of wheel

 $d_1 = dia.$ of bigger axle

 $d_2 = dia.$ of smaller axle

(3) Weston's differential pully block:

$$VR = \frac{2D}{D-d}$$

D = dia. of bigger pulley

d = dia. of smaller pulley

Applied Mechanics

29

(4) Worm and worm wheel:

$$VR = \frac{RT}{r}$$

OR

$$VR = \frac{RT}{nr}$$

R = Radius of effort wheel

= Length of handle

r = Radius of load drum

T = no. of teeth on worm wheel

n = no. of worm thread

(5) Single purchase crab:

$$VR = \frac{l}{r} \times \frac{T_1}{T_2}$$

l = length of handle

r = radius of load drum

 $T_1 = No.$ of teeth on main gear (spur wheel)

 $T_2 = No.$ of teeth on pinion

(6) Double purchase crab:

$$VR = \frac{l}{r} \times \frac{T_1}{T_2} \times \frac{T_3}{T_4}$$

l = length of handle

r = radius of load drum

 T_1 , T_3 = No. of teeth on main gears (spur wheel)

 T_2 , T_4 = No. of teeth on pinions.

(7) Simple screw jack:

$$VR = \frac{2\pi l}{p}$$

l = length of handle

p = pitch of screw

30

IMPORTANT SHORT QUESTIONS

- 1. A lifting machine having an efficiency less than 50% is known as,
- 2. For an ideal machine,
- 3. The maximum mechanical advantage of a lifting machine is,
- 4. Write the equation for law of machine.
- 5. For the first system of pulleys with 4 pulleys, velocity ratio is,
- 6. In a law of machine, P = mW + C term C represents,
- 7. In an ideal machine, the mechanical advantage is to velocity ratio.
- 8. Write equation of VR for simple screw jack.
- 9. The maximum efficiency of a lifting machine is
- 10. What is condition for reversibility of a machine?

ANSWERS

- 1. Non-reversible machine
- 2. MA = VR, Output = Input, $\eta = 100\%$
- 3. $\frac{1}{m}$
- 4. P = mW + C
- 5. $2^n = 2^4 = 16$
- 6. Machine friction
- 7. equal
- 8. $VR = \frac{2 \pi l}{\text{pitch}}$
- 9. $\frac{1}{m \cdot VR}$
- 10. $\eta \ge 50$



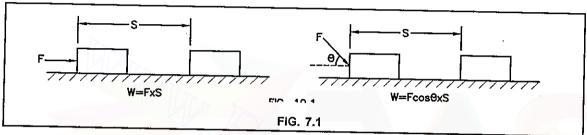
7. Work, Power, Energy

7.1 WORK:

Whenever a force acts on a body, and the body undergoes a displacement, some work is said to be done.

Work done = Force × displacement

$$W = F \times S$$



Unit of work:

In S.I. system, unit of work is Joule.

1 Joule = 1 N.m

1 Kilojoule = 1 kN.m

When a force of 1 N, displaces a body through 1 m, work done is 1 joule (1 N.m)

7.2 WORK NECESSARY FOR LIFTING THE BODY:

To lift mass (m) from position-1 to position-2, upward force F is required. upward force (F) = weight (W) of

body.

$$W = weight of body$$

 $F = W = m \cdot g$

: Work done

= Force × displacement

 $= F \times h$

= mgh

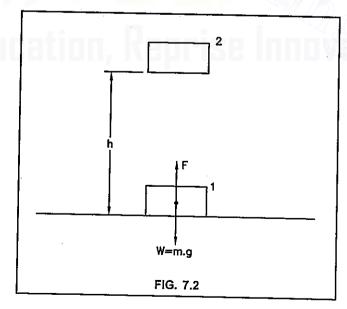
where,

m = mass (kg)

g = gravitational acceleration

 $= 9.80 \text{ m/s}^2$

h = height (m)



7.3 POWER:

The power may be defined as the rate of doing work. It indicates the speed of doing work by a force.

Power =
$$\frac{\text{work done}}{\text{time taken for work done}}$$

= $\frac{F \times S}{t}$ \therefore Velocity = $V = \frac{S}{t}$

Unit of power:

Unit of power is N.m/s or Joule/s.

 $P = F \times V$

In S.I. system unit of power is Watt.

$$1 \text{ N.m/s} = 1 \text{ J/s} = 1 \text{ Watt}$$

 $1 \text{ kW} = 1000 \text{ Watt}$

In gravitational unit,

unit of power is H.P. (Horse power)

7.4 ENERGY:

Energy may be defined as the capacity to do work.

Unit of energy is N.m or joule.

7.5 POTENTIAL ENERGY:

Energy stored in a body due to its position is called potential energy.

Potential energy = mgh

7.6 KINETIC ENERGY:

Energy possessed by a body, by virtue of its mass and velocity is called kinetic energy.

Kinetic energy =
$$\frac{1}{2}$$
 m·v²

7.7 LAW OF CONSERVATION OF ENERGY:

"The energy can neither be created nor destroyed though it can be transformed from one form into any of the forms, in which the energy can exist."

It means total energy remains constant without change.

Applied Mechanics

33

IMPORTANT SHORT QUESTIONS

- 1. What is the S.I. unit of work?
- 2. Define 1 Joule work.
- 3. If a mass m, is lifted through height h, what is work done?
- 4. Define power.
- 5. What is S.I. unit of power?
- 6. 1 H.P. = Watt
- 7. Energy stored in a body due to its position is known as
- 8. Energy possessed by a body, by virtue of its mass and velocity is known as
- 9. Give formula for kinetic energy.
- 10. Give formula for potential energy.
- 11. Give formula for work done by torque.

ANSWERS

- 1. Joule
- 2. When a force of 1 N, displaces a body through 1 m, work done is 1 Joule.
- 3. Work done = mgh
- 4. Power is defined as the rate of doing work.
- 5. Watt
- 6. 746
- 7. Potential energy
- 8. Kinetic energy
- 9. K.E. = $\frac{1}{2}$ mv²
- 10. P.E. = mgh
- 11. Work done by torque = $T \times \theta$

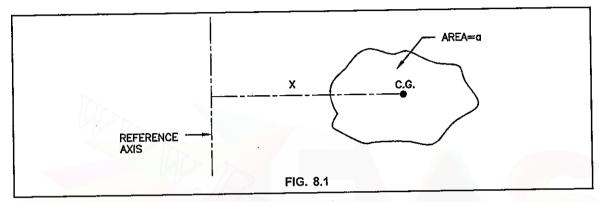


8. Moment of Inertia

8.1 MOMENT OF INERTIA (I):

The second moment of area about any axis is called moment of inertia.

The First moment of area = $a \cdot x$



The second moment of area
$$= (ax)x$$

 $= ax^2 = M.I$

 \therefore Moment of inertia = I = ax^2

The unit of M.I. is mm⁴, cm⁴ or m⁴.

8.2 SECTION MODULUS (Z):

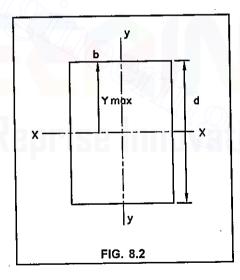
Section modulus
$$=$$
 $\frac{\text{M.I.}}{\text{distance of extreme fibre}}$

$$\therefore \quad \boxed{Z = \frac{I}{y_{max}}} \quad \text{from neutral axis}$$

unit of Z is mm³ or cm³.

For rectangular section,

$$Z_{xx} = \frac{I_{xx}}{d/2}$$
$$Z_{yy} = \frac{I_{yy}}{b/2}$$



8.3 RADIUS OF GYRATION (k):

The distance from the given axis, at which, if all the small elements of the given lamina are placed, the M.I. of the lamina about the given axis does not changes. This distance is called radius of gyration.

Mathematically,

Applied Mechanics

$$I = Ak^2$$

$$\therefore \quad k = \sqrt{\frac{I}{A}}$$

where,

I = momet of inertia

$$A = Area$$

k = radius of gyration.

8.4 MOMENT OF INERTIA OF SOME STANDARD SECTIONS:

(1) Rectangular Section:

$$I_{xx} = \frac{b \cdot d^3}{12}$$

$$I_{yy} = \frac{d \cdot b^3}{12}$$

$$\therefore Z_{xx} = \frac{I_{xx}}{y_{max}} = \frac{\frac{bd^3}{12}}{\frac{d}{2}} = \frac{bd^2}{6}$$

$$Z_{yy} = \frac{I_{yy}}{y_{max}} = \frac{\frac{db^3}{12}}{\frac{b}{2}} = \frac{db^2}{6}$$

$$I_{\text{base}} = \frac{bd^3}{3}$$

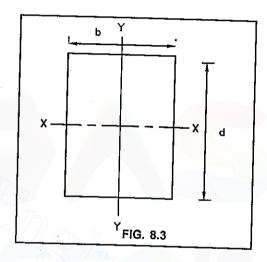
(2) Hollow rectangular section:

$$I_{xx} = \frac{BD^3}{12} - \frac{bd^3}{12}$$

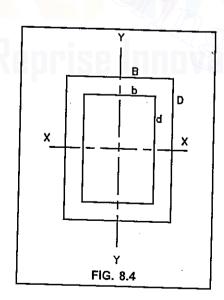
$$I_{yy} = \frac{DB^{3}}{12} - \frac{db^{3}}{12}$$

$$Z_{xx} = \frac{I_{xx}}{y_{max}} = \frac{\frac{BD^{3}}{12} - \frac{bd^{3}}{12}}{\frac{D}{2}} = \frac{\left(BD^{3} - bd^{3}\right)}{6D}$$

$$Z_{yy} = \frac{I_{yy}}{y_{max}} = \frac{\frac{DB^3}{12} - \frac{db^3}{12}}{\frac{B}{2}} = \frac{(DB^3 - db^3)}{6B}$$



35



Objective Civil Engineering

36

(3) Circular section:

$$I_{xx} = \frac{\pi}{64}.D^4$$
$$I_{yy} = \frac{\pi}{64}.D^4$$

$$Z_{xx} = \frac{I_{xx}}{y_{max}} = \frac{\frac{\pi}{64} \cdot D^4}{\frac{D}{2}} = \frac{\pi}{32} \cdot D^3$$

$$Z_{yy} = \frac{I_{yy}}{y_{max}} = \frac{\frac{\pi}{64} \cdot D^4}{\frac{D}{2}} = \frac{\pi}{32} \cdot D^3$$

(4) Hollow circular section:

$$I_{xx} = \frac{\pi}{64} (D^4 - d^4)$$

$$I_{yy} = \frac{\pi}{64} (D^4 - d^4)$$

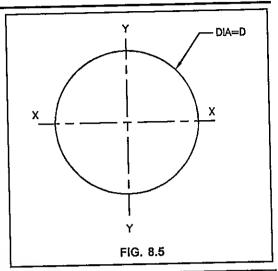
$$Z_{xx} = \frac{I_{xx}}{y_{max}} = \frac{\frac{\pi}{64} (D^4 - d^4)}{\frac{D}{2}} = \frac{\pi (D^4 - d^4)}{32 D}$$

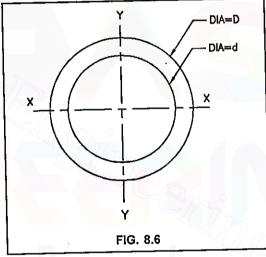
$$Z_{yy} = \frac{I_{yy}}{y_{max}} = \frac{\frac{\pi}{64} (D^4 - d^4)}{\frac{D}{2}} = \frac{\pi (D^4 - d^4)}{32 D}$$

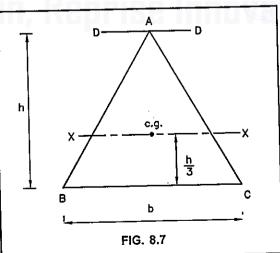
(5) Triangular section:

$$I_{base} = \frac{bh^3}{12}$$

I.c.g. =
$$\frac{bh^3}{36}$$
 $I_{DD} = \frac{bh^3}{4}$







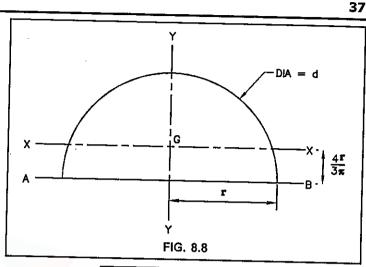
Applied Mechanics

(6) Semicircular section:

$$I_{xx} = 0.11 r^4$$

$$I_{AB} = \frac{\frac{\pi}{64} d^4}{2} = \frac{\pi}{128} d^4$$

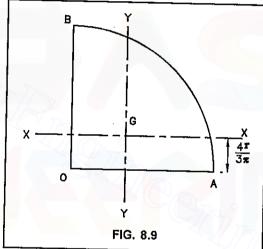
$$I_{yy} = \frac{\pi}{8} r^4$$
$$= 0.393 r^4$$
$$= \frac{\pi}{128} d^4$$



(7) Quarter circle:

$$I_{xx} = I_{yy} = 0.055 \text{ r}^4$$

$$I_{OA} = I_{OB} = \frac{\pi}{16} r^4$$



8.5 PARALLEL AXIS THEOREM:

"If Ig is the moment of inertia of a plane area about an axis passing through its centre of gravity, then, moment of inertia of the area about axis AB, parallel to the first axis, and at a distance h from centre of gravity is given by:

$$I_{AB} = I_g + ah^2$$

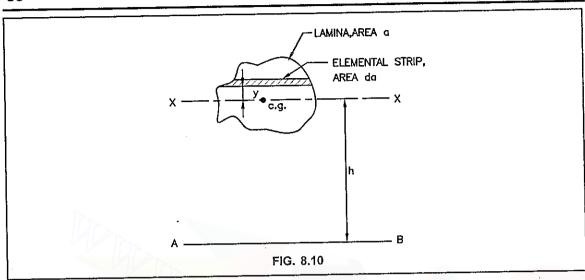
where,

 $I_{AB} = M.I.$ of area about AB

 $I_g = M.I.$ of area about c.g.

a = area of the section

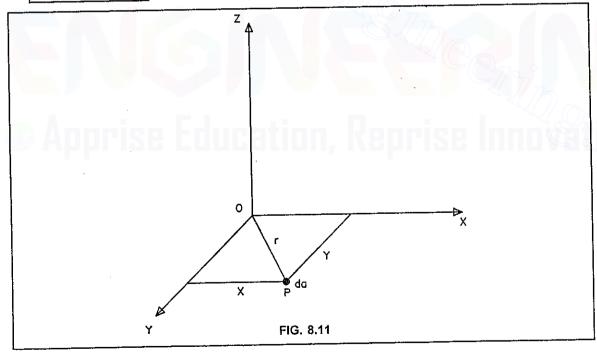
h = distance between c.g. of section and axis AB.



8.6 PERPENDICULAR AXIS THEOREM:

"If, I_{xx} and I_{yy} be the moments of inertia of a plane section about two perpendicular axes meeting at 0, the moment of inertia I_{zz} about the z-z axis, perpendicular to the plane and passing through the intersection of x-x and y-y axes is given by:

$$I_{zz} = I_{xx} + I_{yy}$$



MCQ'S

| _ | | | | | | | |
|-----|--|----------------------|----------|---|------------------------|--|--|
| 1. | The unit of force in S | S.I. system is | | | | | |
| | (a) Kilogram | (b) Newton | (c) | Dyne | (d) Watt | | |
| 2. | A force which can pro | oduce an accelera | tion of | $1 \text{ m/s}^2 \text{ in a m}$ | (d) Watt | | |
| | (a) 1 N | (b) 1 kgf | (c) | l dyne | | | |
| 3. | $1 \text{ kgf} = \underline{\hspace{1cm}} N$ | (| (0) | 1 dylle | (d) 1 watt | | |
| | (a) 8.91 | (b) 10 | (c) | 9.81 | (4) 10 | | |
| 4. | 1 Joule is equal to | (-) | (0) | 7.01 | (d) 12 | | |
| | (a) I N - m | (b) 9.81 N-m | (c) | 1 kgf – m | (d) Mi. | | |
| 5. | 1 MN = N | (-) 3101 I(III | (0) | r kgr – m | (d) None | | |
| | (a) 10^{12} | (b) 10^9 | (c) | 10 ⁶ | (1) 103 | | |
| 6. | Which is not a vector | ` ' | (0) | 10 | (d) 10^3 | | |
| | (a) Weight | (b) Velocity | (c) | Momentum | (4) D: | | |
| 7. | The resultant of two f | | ting of | Momentum | (d) Distance | | |
| | (a) $\sqrt{D^2 + O^2} = 2DO^{-1}$ | - a min Q ac | | | | | |
| | (a) $\sqrt{P^2 + Q^2 - 2PQ \text{ si}}$ | | | $\sqrt{P^2 + Q^2 + 2P^2}$ | - | | |
| | (c) $\sqrt{P^2 + Q^2 + 2PQ c}$ | osθ | (d) | $\sqrt{P^2 + Q^2 - 2}$ | PO cos A | | |
| 8. | The resultant of two force | es acting at a poin | t is max | imum when an | gle hetween thom. A is | | |
| | (a) 0° | (b) 45° | (c) | 90° | (d) 180° | | |
| 9. | The resultant of two fore | ces acting at a poin | t is min | imum when and | tle between them A is | | |
| | (a) 0° | (b) 45° | (c) | 90° | (d) 180° | | |
| 10. | Force is a | | (-) | | (d) 160 | | |
| | (a) Scalar quantity (b) | Vector quantity (c |) linea | r quantity (d)No | n-measurable quantity. | | |
| 11. | The component of a fo | rce (P) at right a | noles t | o its direction | will be | | |
| | | | | _ | will be | | |
| | (a) Zero | (b) P | (c) | 1 | (d) 2P | | |
| 12. | The resultant of two eq | ual forces P acti | | | one will be | | |
| | (a) 2P | (b) \sqrt{P} | | 0.707 P | | | |
| 13. | The concurrent forces I | P and O (P > O) | cting a | long the same | (d) zero | | |
| | The concurrent forces P and Q $(P > Q)$ acting along the same straight line, but in opposite direction, their resultant will be equal to | | | | | | |
| | / \ D | (b) P – Q | (c) | | (d) O/D | | |
| 14. | If P and Q are the two | | an angi | in A and their | (d) Q/P | | |
| | angle ∞ with P, then | arous acting at | an ang | ie o and their | resultant makes an | | |
| | | | | Oais | - 0 | | |
| | (a) $\tan \propto = \frac{Q \sin \theta}{P + Q \cos \theta}$ | - | (b) | $\tan \infty = \frac{Q \sin \alpha}{P + Q}$ | 10 | | |
| | | | ` / | P + Q | sın U | | |
| | (c) $\tan \infty = \frac{Q \sin \theta}{Q + P \cos \theta}$ | | | tan es - P sin | ıθ | | |
| | (c) $\tan \infty = \frac{1}{Q + P \cos \theta}$ | | (d) | $\tan \infty = \frac{P \sin Q}{Q + P}$ | cos θ | | |
| | | | | - | | | |

| 10 | | Objective Civil Engineering |
|-----------|--|---|
| 5. | When two forces each equal to P act at 9 | 0° to each other, then their resultant will be |
| | (a) $\sqrt{2} P$ (b) 2P | (c) 0.707 P (d) zero |
| 6. | If resultant of two equal forces is equ | tal to either of them, then angle between |
| | them is | (IES) |
| | (a) 30° (b) 60° | (c) 90° (d) 120° |
| 17. | Two forces of equal magnitude P act at | t an angle θ to each other. Their resultant |
| | is equal to | (IES) |
| | (a) $2 P \sin \frac{\theta}{2}$ (b) $2P \cos \frac{\theta}{2}$ | (c) $2P \cos \theta$ (d) $2P \cos 2\theta$ |
| 18. | Two forces of 6 kN and 8 kN act at right a | ingles to each other, the resultant force will be |
| | (a) 10 (b) 14 | (c) 28 (d) 100 |
| 19. | If a number of forces are acting at a [| point, their resultant is given by |
| | (a) $(\Sigma H)^2 + (\Sigma V)^2$ | (b) $\sqrt{(\Sigma H)^2 + (\Sigma V)^2}$ |
| | (c) $(\Sigma H)^2 + (\Sigma V)^2 + 2(\Sigma V) (\Sigma H)$ | (d) $\sqrt{(\Sigma H)^2 + (\Sigma V)^2 + 2(\Sigma V)(\Sigma H)}$ |
| 20. | If a number of forces are acting at a p | oint, the angle of resultant with horizontal |
| 20. | will be | |
| | | $\tan \theta = \Sigma V \times \Sigma H \text{ (d) } \tan \theta = \sqrt{(\Sigma V) \times (\Sigma H)}$ |
| | | |
| 21. | The set of forces, whose resultant is ze | ero, are known as |
| | (a) Coplanar forces. (b) Concurrent force | es.(c) Equilibrium forces. (d) Collinear forces. |
| 22. | The forces whose line of actions meet | at one point are cancu |
| | (a) Coplanar forces. (b) Concurrent force | es (c) Equilibrium forces. (d) Collinear forces. |
| 23. | The forces whose line of action lie on | the same line are known as |
| | (a) Concurrent forces. (b) Copianar to | rces.(c) Equilibrium forces. (d) collinear forces. |
| 24. | | (c) $\Sigma M = 0$ (d) $\Sigma H = 0$, $\Sigma V = 0$, $\Sigma M = 0$ |
| | (a) $\Sigma H = 0$ (b) $\Sigma V = 0$ | noint be in equilibrium, then each force is |
| 25. | "If three coplanar forces, acting at a proportional to the sine of the angle l | point be in equilibrium, then each force is |
| | | (b) Lami's theorem . |
| | (a) Laws of moments. | (d) Varignon's theorem. |
| | (c) Condition of equilibrium. | (u) variginar i sassi |
| 26. | | (c) N-m/sec (d) kgf |
| | (a) kg.m(b) N-m | es cause rotary motion of a body then this |
| 27. | when two equal unlike paramet force | |
| | force system is called as |) Principle of moments. (d) Parallel force system. |
| | (a) Moment of force. (b) Couple. (c | irface remains in equilibrium. |
| 28. | (1) -4-hla | (c) neutral (d) any of the above |
| | (a) unstable (b) stable | (-) |
| | | |

Applied Mechanics (MCQ'S)

41

- 29. Four forces P, 2P, 3P and 4P act along the sides taken in order of a square. The resultant force is
 - (a) zero
- (b) $2\sqrt{2} P$
- (c) 2P
- (d) $\sqrt{5} P$
- 30. A smooth sphere lying on a _____ is in neutral equilibrium.
 - (a) Convex surface
- (b) Concave surface (c) horizontal surface (d) inclined surface
- 31. For a sphere suspended as shown in fig. the tension T in the string is



- (d) 5 N

10 N

Sphere

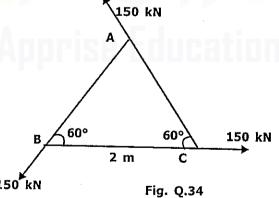
- 32. A 20 kN force is acting vertically upward. Its horizontal component is

- (b) 20 kN
- (c) 14.14 kN

Fig. 0.31

- 33. The statement, "effect of a force upon a body is the same at every point on its line of action" refers to
 - (a) Principle of superposition.
- (b) Principle of transmissibility.
- (c) Principle of moments.

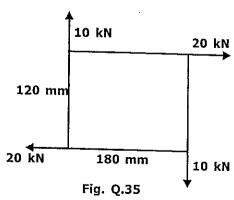
- (d) Lami's theorem.
- 34. The resultant of the force system shown below is
 - 150 kN
- (b) 450 kN
- 75 kN
- (d) 0



150 kN

- 35. The moment of the couple at the centre of rectangle shown in fig. is
 - (a) 8.4 kN.m
 - (b) 4.2 kN.m
 - (c) 4.8 kN.m
 - (d)

Objective Civil Eng. \ 2016 \ 6



43. The centroid of semicircular wire arc lies at a distance of ___ ____ from its base along the vertical axis.

42

Objective Civil Engineering

44. For a T-section having flange 60×10 mm and web 10×60 mm, $_{\rm X}^- = -$. (d) 20 mm

(a) 30 mm

(b) 60 mm

(c) 5 mm

45. The C.G. of a solid cone lying on its axis will be

(a) $\frac{h}{2}$ above the base. (b) $\frac{h}{3}$ above the base (c) $\frac{h}{4}$ above the base (d) $\frac{2}{3}$ h above the base.

46. Two identical balls, one made of gold and other one of steel will have

(a) their C.G. at variable positions.

(b) Their C.G. at the same position.

(c) The C.G. of steel ball will be nearer to the base as compared to gold.

(d) The C.G. of gold ball will be nearer to the base as compared to steel.

Applied Mechanics (MCQ'S)

43

- 47. The C.G. of a quadrant of a circle lies along its central radius at a distance of
 - (a) 0.3 R
- (b) 0.44 R
- (c) 5 R
- (d) 0.6 R
- 48. The centroid of plane lamina is not at its geometrical centre if it is a
 - (a) square.

(b) rectangle.

(c) circle.

- (d) right angled triangle.
- 49. M.I. of a rectangle of width (b) and depth (d) about its base is
 - (a) $\frac{bd^3}{12}$
- (b) $\frac{db^3}{12}$
- (c) $\frac{bd^3}{3}$
- (d) $\frac{bd^3}{36}$

- 50. Which statement is correct for M.I.
 - (a) It is the second moment of area
- (b) Its unit is mm⁴

(c) $I = Z. y_{max}$

- (d) all of the above
- 51. M.I. of a triangle of base (b) and height (h) about x-x axis is
 - (a) $\frac{bh^3}{12}$
- (b) $\frac{bh^3}{36}$
- (c) $\frac{bh^3}{4}$
- (d) $\frac{hb}{12}$
- 52. M.I. of a triangle of base (b) and height (h) about horizontal base is
 - (a) $\frac{bh^3}{12}$
- (b) $\frac{bh^3}{36}$
- (c) $\frac{bh^3}{4}$
- (d) $\frac{hb^3}{12}$
- 53. Moment of inertia of a circular section about its diameter (d) is
 - (a) $\frac{\pi}{16}$ d³
- (b) $\frac{\pi}{32} d^3$
- (c) $\frac{\pi}{32} d^4$
- (d) $\frac{\pi}{64} d^4$
- 54. M.I. of a circular section about an axis perpendicular to the plane of section is
 - (a) $\frac{\pi}{16} d^3$
- (b) $\frac{\pi}{32} d^3$
- (c) $\frac{\pi}{32} d^4$
- (d) $\frac{\pi}{64} d^4$
- 55. M.I. of a square of side (a) about axis passing through centre of gravity is
 - (a) $\frac{a^4}{36}$
- (b) $\frac{a^4}{12}$
- (c) $\frac{a^4}{8}$
- (d) $\frac{a^4}{4}$

- 56. Moment of inertia is the
 - (a) second moment of force.
- (b) second moment of area.
- (c) second moment of mass.
- (d) all of these.
- 57. The unit of M.I. of an area is
 - (a) $kg.m^2$
- (b) kg/m^2
- (c) m^4
- Iv (d) $kg-m-s^2$
- 58. A circular hole of 50 mm diameter is cut out from a circular disc of 100 mm diameter as shown in figure.

The centre of gravity of the section will lie

- (a) at O.
- (b) in the hole.
- (c) in the shaded area.
- (d) outside the disc.

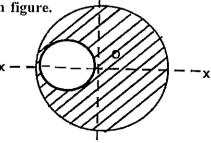


Fig. Q.58

44

| 59. The unit of modulus of section | 59. | The | unit | of | modulus | of | section | i |
|------------------------------------|-----|-----|------|----|---------|----|---------|---|
|------------------------------------|-----|-----|------|----|---------|----|---------|---|

- (a) $kg-m^2$
- (b) m^3 or cm^3 (c) m^4 or cm^4
- (d) m^2 or cm^2

60. If I is the M.I. and 'A' is the total area, then 'k' radius of gyration will be

(a)
$$k = \frac{I}{A}$$

(b)
$$k = \sqrt{\frac{I}{A}}$$
 (c) $k = \sqrt{I \cdot A}$ (d) $k = I \cdot A^2$

(c)
$$k = \sqrt{I.A}$$

(d)
$$k = I.A^2$$

61. In case of beams, greater the M.I. of the section

- (a) greater would be its load carrying capacity.
- (b) Smaller would be its load carrying capacity.
- (c) The load carrying capacity does not change.

62. The strength of a section

- (a) increase with increase in radius of gyration.
- (b) decrease with increase in radius of gyration.
- (c) does not change with change in radius of gyration.

63. The friction force always acts in the direction

- (a) of the force
- (b) opposite to the force
- (c) opposite to that in which the body tends to move
- (d) perpendicular to the force.

64. The force of friction between two bodies in contact

- (a) depends upon the area of their contact.
- (b) depends upon the roughness of the surfaces.
- (c) depends upon the relative velocity between them.
- (d) all of the above.

65. The magnitude of the force of friction between two bodies, one lying above the other depends upon the roughness of the

- (a) upper body.
- (b) lower body.
- (c) both the bodies.
- (d) body having more roughness.

66. Identify the correct statement.

- (a) When body is in equilibrium, the friction force is known as static friction force.
- (b) When body just start moving, the friction force is known as limiting friction force.
- (c) When body is in motion, the friction force is known as kinetic friction force.
- (d) all of the above

67. Identify the correct statement.

- (a) The force of friction does not depend upon the area of contact.
- (b) The magnitude of limiting friction bears a constant ratio to the normal reaction between two surfaces.
- (c) The static friction is slightly less than the limiting friction.
- (d) all of the above.

| Арр | lied | Mechanics (MCQ'S |) | | | | | | 45 |
|--|---|---|-------------------------------------|------------|---------------|-------------------------------|---------|-------------------|-----------------|
| 68. | The | efficiency of a scr | ew jack may | be incre | ease | ed by | | | |
| | | increasing its pitch | | | | ecreasing its | pitch | | |
| | | increasing the load | | | | _ | • | | lifted |
| 69. | | efficiency of a scr | | | | | | | |
| | | | | | | | | | |
| | | $45^{\circ} + \frac{\phi}{2}$ | ∠ | (c) | $\frac{1}{2}$ | ·+30° | (d) | $\frac{4}{2}$ - 3 | 0° |
| 70. | | tify the correct sta | | | | | | | |
| | | When resultant lies | | | | | | | |
| | | When resultant lies | | | | | | | |
| | | When resultant lies | | of cone | the | body will b | e in li | imiting | g equilibrium. |
| 71. | | ic friction is alway | | | | | | | |
| | (a) | less than dynamic | friction. | (t | b) | greater that | ı dyn | amic f | riction. |
| | | equal to the dynam | | | | | | | |
| 72. | | is the limiting fric | tion, N is the | normal 1 | rea | ction, then | coeffi | cient | of friction μ |
| | | be equal to | 0/ | | | | | | |
| | | $\mu = F + N$ | | | | | | | |
| 73. A ladder is resting on a smooth floor and leaning against a rough vertic | | | | | | ertical wall, | | | |
| | the force of friction acts | | | | | | | | |
| | | towards the wall at | | | | | | - | • |
| | | upward at the uppe | | | | | | | |
| 74. | | angle of inclinatio | n of the plan | e at whi | ch | the body to | ends | to mo | ve down the |
| | _ | e is called | 4) | | | | | | |
| | | angle of friction. | | | | | | | |
| 75. | | maximum friction | | | | | n a b | ody ji | ist begins to |
| | | over the surface | | - | | | | / D= D | |
| 7 | (a) | Sliding friction | (b) Dynamic | triction (| c) | Limiting fro | etion | (d) R | olling friction |
| /0. | Coulomb friction is | | | | | | | | |
| | | (a) Friction between bodies having relative motion. | | | | | | | |
| | (b) Friction between dry surfaces.(c) Friction between solids and liquids. | | | | | | | | |
| | | | - | | • | | | | |
| 77. | | Friction between el | = | _ | | | | | |
| 11. | | e angle of friction | is zero, a boo | • | - | | | | |
| ٠ | | Infinite friction. | | • | • | zero frictio | n. | | |
| | | The force of friction | | | | • | | | |
| 70 | ` ' | The force of friction | | | | | | | |
| / 0. | 11 fU | e angle of friction | is ψ, the effic | nency of | ı s(| crew jack is | i | | |
| | (-) | $\frac{1-\sin\phi}{1+\sin\phi}$ | (b) $\frac{1+\sin\phi}{1-\sin\phi}$ | , | . \ | $\frac{1+\sin\phi}{\cos\phi}$ | | $\frac{1}{2}$ | −sin φ |
| | (a) | $1 + \sin \phi$ | (b) $1-\sin\phi$ | , (c | :) | cos φ | | (d) - | cos φ |
| | | | | | | | | | |

| Арр | lied | Mechanics (MCQ'S) | | | | | 47 | | |
|------|------------------------------|--------------------------------|---------------------------|--------|---------------------|-------------------|---------------------------------------|--|--|
| | (a) | Coefficient of fricti | on | (b) | Machine friction | · · · · · · · · · | · · · · · · · · · · · · · · · · · · · | | |
| | (c) | Input | | (d) | Output | | | | |
| 92. | The | efficiency of a lifti | ng machine is the | ratio | of | | | | |
| | (a) | Output to input | | (b) | MA to VR | | | | |
| | (c) | Work done by mac | hine to work done c | n ma | chine | | | | |
| | (d) | all of the above | | | | | | | |
| 93. | In a | ı simple screw jack | with l as the leng | th of | handle and p a | s th | e pitch of the | | |
| | screw, its velocity ratio is | | | | | | | | |
| | (0) | $\frac{2 \pi p}{I}$ | (b) $\frac{2\pi l}{p}$ | (a) | $\frac{\pi l}{}$ | (4) | $\frac{\pi p}{2l}$ | | |
| | | • | 1 | | 2p | (u) | 21 | | |
| 94. | | a simple lifting ma | | | | | | | |
| | (a) | $\frac{W}{MA}$ | $(b) \frac{W}{}$ | (c) | $\frac{MA}{}$ | (d) | output | | |
| | | | | | | | input | | |
| 95. | | self locking machin | / | | | ••••• | *** | | |
| | ` ' | 50% | (b) more than 50% | | | | | | |
| 96. | | oiling the lifting ma | | | | 18 1 | C | | |
| 0.7 | | velocity ratio | | antage | e (c) efficiency (c | i) lav | v of machine | | |
| 97. | | rate of doing work | •)/ | (-) | 7 | (-1) | | | |
| 00 | • , | Work done | | (c) | energy | (a) | none of these | | |
| 98. | | .P. = wat | | (0) | 746 | (4) | 1046 | | |
| 00 | | 346 | (b) 546 | (c) | 746 | (u) | 1046 | | |
| 99. | | unit of power is | (h) watt | (0) | iaula | (4) | druso | | |
| 100 | • / | U | (b) watt | (c) | joule | (u) | dyne | | |
| 100. | | S.I. unit, the unit of Kg-m | (b) Newton | (c) | ara | (4) | Joule | | |
| 101 | | oule = N.n | | (0) | arg | (u) | Joule | | |
| 101. | (a) | | (b) 9.81 | (c) | 10 | (d) | 0.1 | | |
| 102 | . , | joule work means | • ' | (0) | 10 | (4) | 0.1 | | |
| 1021 | | Work done by a fo | | displa | ces a body throu | gh 1 | m. | | |
| | (b) | | rce of 1 kg when it | - | | | | | |
| | (c) | • | rce of 1 dyne when | - | | _ | | | |
| | (d) | None of these. | • | | | • | , | | |
| 103. | • / | att = | | | | | | | |
| | | 0.1 Joule/s | (b) 1 Joule/s | (c) | 10 Joule/s | (d) | 100 Joule/s | | |
| 104. | • • | W' is the weight of | | | | | | | |
| | | potential energy sto | | | - | • | | | |
| | | | | | | | | | |
| | (a) | <u>wn</u> | (b) $\frac{\text{Wh}}{2}$ | (c) | Wh | (d) | Wh^2 | | |
| | ` ' | 2 | 2 | • / | | . , | | | |

105. A body of mass 'm' is moving with a uniform velocity 'v', the kinetic energy stored by the body will be

(a)

(b) $\frac{\text{mv}^2}{2\text{g}}$

(c) $\frac{\text{mv}^2}{2}$

(d) mv^2

106. In terms of work, energy is defined as

(a) quantity of work.

(b) capacity of doing work.

(c) rate of doing work.

(d) rate of change of doing work.

107. When the spring of a watch is wound, it will possess

- (a) strain energy.
- (b) heat energy. (c) kinetic energy. (d) electrical energy.

108. The sum of potential and kinetic energy possessed by a moving body

- (a) varies from point to point.
- (b) is maximum in the start and minimum at the end.
- (c) is minimum in the start and maximum at the end.
- (d) is constant at all points.

109. A truck of mass 10,000 kg is moving with a velocity of 36 kmph. Kinetic energy will be

- (a) 50,000 N.m
- (b) 500,000 N.m (c) 1,00,000 N.m
- (d) 5,000 N.m

110. A block weighing 500 N is resting on a horizontal floor. A force of 200 N applied at 30° with horizontal moves the block to a distance of 60 m. Work done is

- (a) 10392.3 N.m
- (b) 12000 N.m
- (c) 6000 N.m
- (d) 5196 N.m

111. Two forces under equilibrium must be

- (a) collinear.
- (b) like parallel.
- (c) unlike parallel. (d) non parallel. _ only

112. Law of transmissibility is applicable for _

- (a) deformed bodies
- (b) solid bodies
- rigid bodies
- (d) any of these

113. Varignon's principle is used to find

- (a) Resultant of coplanar concurrent forces
- (b) Resultant of coplanar non-concurrent forces
- (c) Location of resultant of coplanar concurrent forces
- (d) Location of resultant of coplanar non-concurrent forces

114. On a ladder resting on the ground and leaning against a smooth vertical wall, the force of friction acts

- (a) downward at upper and.
- upward at upper end. (b)
- (c) towards the wall at lower end.
- away from the wall at lower end.

: ANSWERS:

| 1. (b) | 2. (a) | 3. (c) | 4. (a) | 5. (c) |
|---------|---------|---------|---------|---------|
| 6. (d) | 7. (c) | 8. (a) | 9. (d) | 10. (b) |
| 11. (a) | 12. (d) | 13. (b) | 14. (a) | 15. (a) |
| 16. (d) | 17. (b) | 18. (a) | 19. (b) | 20. (a) |

Applied Mechanics (MCQ'S)

| 4 | 9 |
|---|---|
| | |

| | | | | |
|----------|----------|----------|----------|-------------|
| 21. (c) | 22. (b) | 23. (d) | 24. (d) | 25. (b) |
| 26. (b) | 27. (b) | 28. (a) | 29. (b) | 30. (c) |
| 31. (b) | 32. (a) | 33. (b) | 34. (d) | 35. (b) |
| 36. (a) | 37. (b) | 38. (c) | 39. (b) | 40. (c) |
| 41. (a) | 42. (d) | 43. (b) | 44. (a) | 45. (c) |
| 46. (b) | 47. (d) | 48. (d) | 49. (c) | 50. (d) |
| 51. (b) | 52. (a) | 53. (d) | 54. (c) | 55. (b) |
| 56. (d) | 57. (c) | 58. (c) | 59. (b) | 60. (b) |
| 61. (a) | 62. (a) | 63. (c) | 64. (b) | 65. (c) |
| 66. (d) | 67. (d) | 68. (a) | 69. (b) | 70. (c) |
| 71. (b) | 72. (c) | 73. (c) | 74. (b) | 75. (c) |
| 76. (b) | 77. (b) | 78. (a) | 79. (c) | 80. (b) |
| 81. (b) | 82. (c) | 83. (d) | 84. (a) | 85. (c) |
| 86. (a) | 87. (b) | 88. (c) | 89. (a) | 90. (c) |
| 91. (b) | 92. (d) | 93. (b) | 94. (b) | 95. (c) |
| 96. (a) | 97. (b) | 98. (c) | 99. (b) | 100. (d) |
| 101. (a) | 102. (a) | 103. (b) | 104. (c) | 105. (c) |
| 106. (b) | 107. (a) | 108. (d) | 109. (b) | 110. (a) |
| 111. (a) | 112. (c) | 113. (d) | 114. (c) | |

EXPLANATIONS

12. (d)
$$P = P, Q = P, \theta = 180^{\circ}$$

 $\therefore R = \sqrt{P^2 + Q^2 + 2PQ\cos\theta}$
 $= \sqrt{P^2 + P^2 + 2PP(-1)}$
 $= \sqrt{P^2 + P^2 - 2P^2} = 0$

13. (b)
$$R = \sqrt{P^2 + Q^2 + 2PQ\cos\theta}$$
 $\theta = 180^\circ$
 $= \sqrt{P^2 + Q^2 + 2PQ(-1)}$
 $= \sqrt{P^2 - 2PQ + Q^2}$
 $= \sqrt{(P - Q)^2}$ $= P - Q$

15. (a)
$$P = P$$
, $Q = P$, $\theta = 90^{\circ}$
 $R = \sqrt{P^2 + Q^2 + 2PQ\cos\theta}$

$$cos 90^{\circ} = 0$$

Objective Civil Eng. \ 2016 \ 7

Objective Civil Engineering

50

$$= \sqrt{P^2 + P^2 + 2P \cdot P \cos 90^{\circ}}$$

$$= \sqrt{P^2 + P^2}$$

$$= \sqrt{2P^2} \qquad = \sqrt{2} P$$

16. (d)
$$P = P, Q = P, R = P$$

$$\therefore R = \sqrt{P^2 + Q^2 + 2PQ\cos\theta}$$

$$P = \sqrt{P^2 + P^2 + 2P^2\cos\theta}$$

$$P = \sqrt{2P^2(1 + \cos\theta)}$$

$$\therefore P^2 = 2P^2 (1 + \cos \theta)$$

$$\therefore 1 + \cos \theta = \frac{1}{2}$$

$$\cos \theta = -0.5$$

$$\therefore \theta = 120^{\circ}$$

17. (b)
$$P = P, Q = P$$

$$R = \sqrt{P^2 + P^2 + 2P.P\cos\theta}$$

$$R = \sqrt{2P^2 + 2P^2 \cos \theta}$$

$$R = \sqrt{2P^2 \left(1 + \cos\theta\right)}$$

$$R^2 = 2P^2 (1 + \cos\theta)$$

$$R^2 = 2P^2 \cdot 2\cos^2\frac{\theta}{2}$$

$$R^2 = 4P^2 \cos^2 \frac{\theta}{2}$$

$$\therefore R = 2 P \cos \frac{\theta}{2}$$

18. (a)
$$R = \sqrt{6^2 + 8^2 + 2 \times 6 \times 8 \times \cos 90^\circ}$$

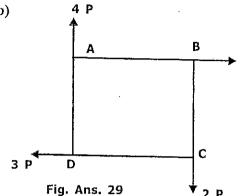
 $= \sqrt{6^2 + 8^2}$
 $= \sqrt{100}$
 $= 10 \text{ kN}$

$$1 + \cos \theta = 2 \cos^2 \frac{\theta}{2}$$

Applied Mechanics (MCQ'S)

51

29. (b)



$$\Sigma H = P - 3P = -2P$$

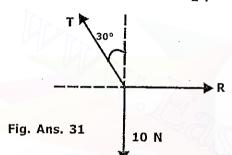
$$\Sigma V = 4P - 2P = 2P$$

$$R = \sqrt{(-2P)^2 + (2P)^2}$$

$$= \sqrt{8P^2}$$

$$= 2\sqrt{2} P$$

31. (b)



$$\Sigma V = 0$$
,

$$\therefore$$
 T cos 30° = 10

$$T = 11.55 \text{ N}$$

34. (d)
$$\Sigma H = 150 - 150 \cos 60^{\circ} - 150 \cos 60^{\circ}$$

= $150 - 75 - 75$
= 0

$$\Sigma V = 150 \sin 60^{\circ} - 150 \sin 60^{\circ}$$

= 0

$$\therefore$$
 R = 0

35. (b)
$$M = (20 \times 0.12) + (10 \times 0.18)$$

= 4.2 kN.m clockwise

36. (a)
$$\Sigma H = 20 - 20 = 0$$
 $\therefore R = 0$ $\Sigma V = 10 - 10 = 0$

39. (b)
$$R_A + R_B = 80 + W$$

 $60 + 60 = 80 + W$ $\therefore W = 40 \text{ kN}$

40. (c)
$$\Sigma M_A = 0$$

 $\therefore 60 \times 5 = (80 \times 2) + 40 x$
 $300 = 160 + 40 x$
 $\therefore x = 3.5 \text{ m}$

Objective Civil Engineering





47. (d)

<u>_</u> х Fig. Ans. 47

$$\overline{x} = \frac{4r}{3\pi}, \ \overline{y} = \frac{4r}{3\pi}$$

$$\therefore OG = \sqrt{\left(\frac{4r}{3\pi}\right)^2 + \left(\frac{4r}{3\pi}\right)^2}$$

$$= \sqrt{\frac{32r^2}{9\pi^2}}$$

$$= 0.6 \text{ r}$$

54. (c) Izz = Ixx + Iyy
=
$$\frac{\pi}{64} d^4 + \frac{\pi}{64} d^4$$

= $\frac{\pi}{32} d^4$

55. (b)
$$I = \frac{bd^3}{12} = \frac{a \cdot a^3}{12} = \frac{a^4}{12}$$

87. (b) For the first system of pulleys $VR = 2^n$

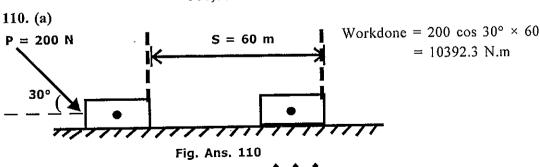
$$VR = 2^{n} = (2)^{4} = 16$$

109. (b) m = 10,000 kg

$$V = 36 \text{ kmph} = \frac{36 \times 1000}{3600} = 10 \text{ m/s}$$

Kinetic energy = $\frac{1}{2}$ mv²

$$= \frac{1}{2} \times 10,000 \times (10)^2$$
$$= 500,000 \text{ N.m}$$



2.

Strength of Materials

1: Mechanical Properties of Materials

1.1 IMPORTANT MECHANICAL PROPERTIES:

From an engineering point of view, the followings are the important mechanical properties of the materials.

- 1. Strength
- 2. Elasticity
- 3. Plasticity
- 4. Ductility

- 5. Brittleness
- 6. Malleability
- 7. Toughness
- 8. Hardness

- 9. Stiffness
- 10. Creep
- 11. Fatigue strength

1. Strength:

The capacity of material to withstand load is called strength.

The strength of material is its ability to sustain loads without undue distortion, rupture or collapse.

A material should have adequate strength in tension, compression, shear, bending or torsion.

The maximum stress that any material can withstand is called ultimate strength or tenacity.

2. Elasticity:

On a material, when external load is applied it undergoes deformation and on removal of the load, it returns to it's original shape. This property of a material is known as elasticity. Such a material is called elastic material.

Some materials possess elasticity up to considerable range of load (stress), while some materials possess elasticity for a very low range of stress.

The elastic properties of a material are of great importance to a design engineer since materials loaded beyond their elastic limit develop a permanent deformation or strain.

3. Plasticity:

If a material does not regain its original shape, on the removal of the external load, it is called a plastic materials.

Plasticity is the property of a material by virtue of which material can be moulded in to desired shape. It is the lack of elasticity.

4. Ductility:

If a material can undergo a considerable deformation, without rupture (e.g. if a material can be drawn into wires) it is called a ductile material.

Ductile materials undergoes large deformations during tension test. Ductile material is the most suitable material for tension member.

Steel, Copper, Wrought-iron, Aluminimum alloys are ductile materials, stainless steel is the most ductile material and Grey cast iron is the least ductile material.

Higher the percentage of elongation, more ductile is material.

As temperature increases, ductility increases.

Metals having elongation, more than 15% are ductile.

5. Brittleness:

If a material cannot undergo any deformation, when some external forces act on it and it fails by rupture, it is called a brittle material.

Brittle materials are stronger in compression and weak in tension.

C.I., Glass, Concrete, Brick, China-ware are the examples of brittle materials.

It is a compressive quality of a material.

Materials having less than 5% elongation are considered as brittle.

6. Malleability:

It is a property of a material by virtue of which material can be converted in to thin sheets by hammering.

Malleable material can be easily rolled and forged without cracking or breaking.

Gold, Silver, Copper, Aluminimum, Tin, Lead steel are the examples of malleable materials.

Gold is the most malleable materials while C.I. is the least malleable material.

This is also a compressive quality of a material.

7. Toughness:

Resistance to impact or shock loading is called toughness.

OR

Capacity of a material to absorb energy before rupture is called toughness.

OR

Work required to cause rupture, under static load is called toughness.

Toughness of a material is the ability to withstand large stresses and strains without fracture.

Mild steel, Wrought iron, Manganese steel etc. have good toughness property.

Toughness is the highly desirable quality in materials for structural and machine parts subjected to shocks and vibrations.

8. Hardness:

Resistance of a material to abrasion, indentation, wear and scratches is called hardness. High resistance to indentation is desired in a components like crank shafts, rails, gears, axles etc.

Strength of Materials

55

An alloyed C.I. is the hardest material while magnesium alloy is the softest material.

9. Stiffness:

Resistance to deformation or strain is called stiffness.

OR

Force required to produce unit deformation in a material is called stiffness.

The stiffness of a material is measured by its 'Modulus of Elasticity (E)'. The higher the value of E, the more is stiffness.

Steel is considered as a stiffer material.

10. Creep:

Increase in strain under sustained load is known as creep.

OR

Inelastic deformation due to sustained load is known as creep.

Metals, having low melting temperatures like Lead, Tin, Zinc may exhibit considerable creep at normal temperature.

Creep phenomena is important for soft metals used at room temperature, steel cables, Nuclear reactor field etc.

11. Fatigue strength:

When a material is subjected to repeated loading, it is said to be in a 'state of fatigue'.

The maximum stress, which a material can withstand, under repeated stress cycles without fracture is called "Fatigue strength'.

The capacity of a material to withstand repeated stress cycles is known as endurance of a material.

For most of the materials there is limiting stress below which a stress or a load may be repeated infinite number of times without causing failure. This limiting stress is known as endurance limit or Fatigue limit.

Homogeneous material:

Homogeneous material means that the material of the member is of the same kind through its length.

For example, if a beam is made of steel, the material of beam does not change throughout the length of the beam.

Isotropic material:

Isotropic means it possesses the same elastic properties in all the directions.



2: Direct Stress and Strain

2.1 SRESS, STRAIN, δl CALCULATIONS:

1. Stress (σ) : On a body when external force (load) is applied, it undergoes some deformation and internal resisting forces are set up.

This resistance to force per unit cross sectional area is called stress.

$$Stress = \frac{Force}{Area}$$

$$\therefore \quad \sigma = \frac{P}{A} = \frac{R}{A}$$

P = external force

R = Resisting force

S.I. unit of stress is N/mm² (MPa)

$$1 \text{ Pa} = 1 \text{ N/m}^2$$

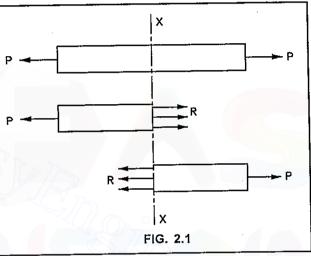
$$1 \text{ KPa} = 10^3 \text{ Pa} = 10^3 \text{ N/m}^2$$

$$1 \text{ MPa} = 10^6 \text{ Pa} = 1 \text{ N/mm}^2$$

$$1 \text{ GPa} = 10^9 \text{ Pa} = 10^3 \text{ N/mm}^2$$

$$K = Kilo = 10^3$$
, $M = Mega = 10^6$,
 $G = Gega = 10^9$





2. Strain (E): It is defined as the ratio of change in length to the original length of the member

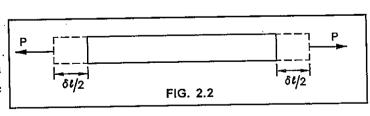
Change in length

$$\therefore Strain = \frac{Change in length}{Original length}$$

$$\varepsilon = \frac{\delta l}{l}$$

Strain has no unit.

It is a measure of deformation produced in a member due to the loads acting on it.



3. Direct stress (Normal stress): The stresses which act normal to the plane on which the forces act are called as normal stress or direct stress.

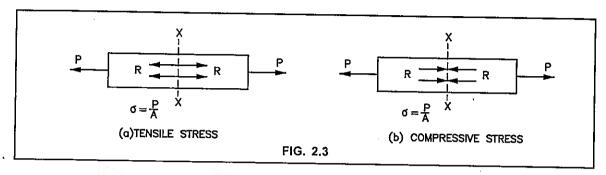
These are two types:

(i) Tensile stress: When a body is subjected to two equal and opposite pulls, the stress produced is known as tensile stress.

Strength of Materials

57

(ii) Compressive stress: When a body is subject to two equal and opposite pushes, the stress produced is known as compressive stress.

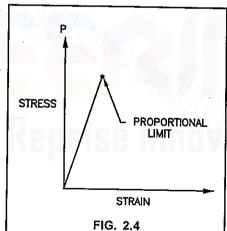


- · If the stress is tensile, the corresponding strain is known as tensile strain.
- · If the stress is compressive, the corresponding strain is known as compressive strain.
- 4. Elasticity: It is the property of a body to regain its original position after the removal of external force.

If a body is elastic, it will deform on application of external force, but as soon as the force is removed, it comes back to its original shape and size.

- 5. Elastic limit: Maximum value of force, up to and within which the deformation entirely disappears on removal of the force, is called elastic limit
- removal of the force, is called elastic limit.

 6. Hooke's Law: It states, "Within elastic limit, stress



Stress o strain

is proportional to strain".

$$\sigma = \text{constant} \times \epsilon$$

$$\therefore \quad \frac{\sigma}{\varepsilon} = constant$$

7. Modulus of elasticity (Young's modulus): E It is the ratio of direct stress to the direct strain.

$$\therefore \quad \text{Modulus of elasticity} = \frac{\text{direct stress}}{\text{direct strain}}$$

$$\therefore E = \frac{\sigma}{\varepsilon}$$

Objective Civil Eng. \ 2016 \ 8

58

S.I. unit of E is N/mm².

For mild steel $E = 2 \times 10^5 \text{ N/mm}^2$

8. Equation of δl :

We know that,

$$\sigma = \frac{P}{A}$$

$$\varepsilon = \frac{\delta l}{l}$$

$$E = \frac{\sigma}{\epsilon}$$

$$\therefore \quad E = \frac{\sigma}{\varepsilon} = \frac{P/A}{\delta l/l} = \frac{Pl}{A.\delta l}$$

$$\therefore \quad \delta l = \frac{P l}{AE}$$

Remember:

1. Stress,
$$\sigma = \frac{P}{A}$$

2. Strain,
$$\varepsilon = \frac{\delta l}{l}$$

3. Modulus of elasticity,
$$E = \frac{\sigma}{\epsilon}$$

4.
$$\delta l = \frac{P \cdot l}{AE}$$

• Deformation of a body due to self weight (For uniform section) :

$$\delta l = \frac{wl^2}{2AE}$$
 ...where $w = \text{weight per unit length}$

• Stresses in bars of uniformly tapering circular section:

$$\delta l = \frac{4Pl}{\pi E d_1 d_2}$$

where,

 $d_1 = dia.$ at larger end

 $d_2 = dia$. at smaller end

2.2 COMPOSITE SECTION:

If a bar is made up of two or more different materials, it is called **composite section**. For example,

MATERIAL

FIG. 2.5

Strength of Materials

59

RIGID PLATE

MATERIAL

- steel and copper
- steel and brass
- copper and aluminium
- concrete and steel, etc.

Consider the composite section shown in figure.

Let,

P = total load on the bar

 $P_1 = load$ shared by bar 1,

 P_2 = load shared by bar 2,

 A_1 = Cross sectional area of bar 1,

 A_2 = Cross sectional area of bar 2,

 $E_1 = modulus of elasticity of bar 1,$

 E_2 = modulus of elasticity of bar 2,

For composite bar,

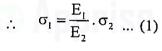
elongation or contraction of each bar is same

$$\therefore \quad \delta l_1 = \delta l_2 \text{ and } l_1 = l_2$$

:. Strain in material 1 = Strain in material 2

$$\therefore \quad \varepsilon_1 = \varepsilon_2$$

$$\therefore \frac{\sigma_1}{E_1} = \frac{\sigma_2}{E_2}$$



Here, $\frac{E_1}{E_2}$ is known as modular ratio :

$$E_1 > E_2$$

Now,

Total load = load on material 1 + load on material 2

$$\therefore \quad P = P_1 + P_2$$

$$: \sigma = \frac{P}{\Delta}$$

$$\therefore P = \sigma_1 A_1 + \sigma_2 A_2 \dots (2)$$

2.3 ELASTIC CONSTANTS:

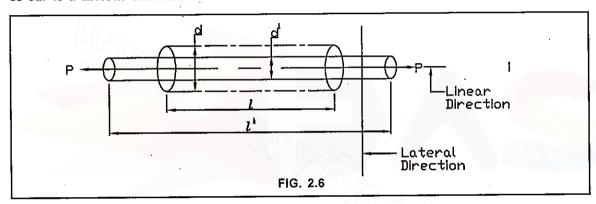
Consider a bar subjected to tensile force "P' as shown in figure.

Due to tensile force P,

Length increases from l to l'

Diameter decreases from d to d'

The direction of force is called linear direction. The direction of force perpendicular to the linear direction is called lateral direction. length of bar is a linear dimension. Diameter of bar is a lateral dimension.



1. Linear strain (ε):

It is defined as the ratio of change in length to the original length of the member.

$$\therefore \quad \varepsilon = \frac{\delta l}{l}$$

Strain has no unit.

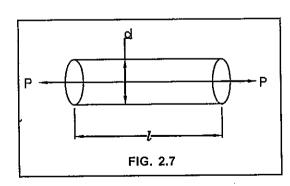
Linear strain and strain are same

2. Lateral strain (ε'):

· For circular cross section:

 $Lateral strain = \frac{Change in diameter}{Original diameter}$

$$\therefore \quad \boxed{\varepsilon' = \frac{\delta d}{d}}$$



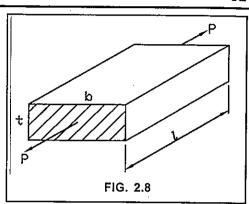
61

For rectangular cross section:

$$\varepsilon' = \frac{\delta b}{b}$$

or

$$\varepsilon' = \frac{\delta t}{t}$$



3. Poisson's ratio (μ or $\frac{1}{m}$):

The ratio of lateral strain to linear strain is called Poisson's ratio.

Poisson's ratio =
$$\frac{\text{Lateral strain}}{\text{Linear strain}}$$

$$\mu = \frac{1}{m} = \frac{\varepsilon}{\varepsilon}$$

For steel, $\mu = 0.25$ to 0.33

For concrete, $\mu = 0.08$ to 0.18

4. Volumetric strain (ε_v) :

 $Volumetric strain = \frac{Change in volume}{Original volume}$

$$\varepsilon_{v} = \frac{\delta V}{V}$$

Equation to find change in volume:

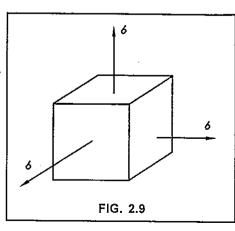
$$\frac{\delta V}{V} = \epsilon (1 - 2 \mu)$$

5. Bulk modulus (K):

When a body is subjected to three mutually perpendicular stresses of equal intensity, the ratio of direct stress to the corresponding volumetric strain is known as **bulk modulus**.

Bulk modulus =
$$\frac{\text{Direct stress}}{\text{Volumetric Strain}}$$

$$\therefore \quad K = \frac{\sigma}{\epsilon_v} \dots N/mm^2$$



Objective Civil Engineering

62

• Shear modulus (Modulus of rigidity) G or N or C:

Modulus of rigidity =
$$\frac{\text{shear stress}}{\text{Shear strain}} = \frac{\tau}{\phi} \text{ N/mm}^2$$

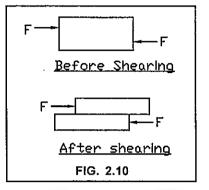
6. Shear stress and shear strain:

Shear stress:

When a body is subjected to two equal and opposite forces acting tangetially across the resisting section, as a result of which the body tends to shear off across the section, the stress induced is called **shear stress**.

Shear stress =
$$\frac{\text{Shear force}}{\text{shearing Area}}$$

Unit of shear stress is N/mm²



Shear strain:

Consider a cube of length l fixed at bottom face CD.

Let force F is applied to face DC tangetially to the face AB.

As a result the cube will deformed from ABCD

to A'B'CD

Shear strain =
$$\frac{\text{deformation}}{\text{original length}}$$

$$\tan \phi = \frac{AA'}{AD} = \frac{BB'}{BC}$$
, but ϕ is very small.

$$\therefore$$
 tan $\phi = \phi$

$$\therefore \quad \phi = \frac{AA'}{AD} = \frac{BB'}{BC}$$

7. Complimentary shear stress:

In order to cause an equilibrium. a shear stress (τ) across a plane, is always accompanied by a balancing shear stress (τ ') across the plane and normal to it. This balancing shear stress is called **complimentary shear stress**.

Condiser a block ABCD. shear stress (t) is acting on faces AD and CB.

:. Forces acting on faces AD and CB.

$$P = \tau \times AD = \tau \times CB$$

This forces will form a couple.

 \therefore Moment of couple = M_1 = force × distance

۲'

FIG. 2.12

τ

$$M_1 = (\tau \times AD) \times AB....(i)$$

If the block is in equilibrium, resisting couple with shear stress τ ' on faces AB and CD will be set up.

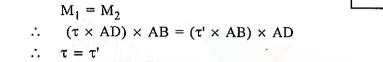
.. forces acting on faces AB and CD. $P = \tau' AB = \tau' \times CD$

This forces will also form a couple.

Moment of this couple,

$$M_2 = (\tau' \times AB) \times AD....$$
 (ii)

equating two moments,



8. Modulus of rigidity or shear modulus (G or N or C)

Modulus of rigidity =
$$\frac{\text{Shear stress}}{\text{Shear strain}}$$

$$\therefore G = \frac{\tau}{\phi} \dots N/mm^2$$

· Relation between E and K:

$$K = \frac{mE}{3(m-2)}, \frac{1}{m} = Poisson's ratio$$

· Relation between E and G:

$$G = \frac{mE}{2(m+1)}$$

· Relation between E, G and K:

$$E = \frac{9GK}{G + 3K}$$

• Volumetric strain of a rectangular body subjected to only axial force :

$$\frac{\delta V}{V} = \varepsilon \left(1 - 2\mu \right)$$

$$\mu = \frac{1}{m}$$

$$V = \text{volume of the body}$$

$$\varepsilon = \frac{\sigma}{E}$$

• Volumetric strain of a rectangular body subjected to three mutually perpendicular Forces:

resultant strain in x-direction

$$\varepsilon_{x} = \pm \frac{\sigma_{x}}{E} \pm \frac{\sigma_{y}}{mE} \pm \frac{\sigma_{z}}{mE}$$

$$\varepsilon_{y} = \pm \frac{\sigma_{y}}{E} \pm \frac{\sigma_{x}}{mE} \pm \frac{\sigma_{z}}{mE}$$

$$\varepsilon_{z} = \pm \frac{\sigma_{z}}{E} \pm \frac{\sigma_{x}}{mE} \pm \frac{\sigma_{y}}{mE}$$

Volumetric strain,

$$\frac{\delta V}{V} = \epsilon_x + \epsilon_y + \epsilon_z$$

Note:

tension = + Ve

Compression = - Ve

Take normal sign for linear strain
(first strain) and opposite to normal
sign for two lateral strains.

2.4 THERMAL STRESS:

1. Thermal stress:

With increase in temperature material expands and with decrease in temperature it contracts. If this free expansion or contraction is prevented, Stress will be generated in the material. It is called thermal stress.

If temperature of a material is increased or decreased and it is allowed to expand or contract freely, no stress will be produced in the material.

Free deformation of a material due to change in temperature,

$$\delta l = l \times \alpha \times \mathbf{t}$$

where,

l = length of a member,

 α = coefficient of thermal expansion,

t = change in temperature

Let,

l =original length of bar

If temperature of bar is increased by to C,

Increase in length.

$$\delta l = l \alpha t$$
.

If this expansion is to be prevented, external compressive load P is required.

Hence, if temperature of a bar is increased and expansion is prevented compressive stress will be produced.

If temperature of bar is reduced by to c,

decrease in length,

$$\delta l = l \alpha t$$

If this contraction is to be prevented, external tensile load P, is required.

Hence, if temperature of a bar is decreased and contraction is prevented, tensile stress will be produced.

65

2. Thermal Strain:

Thermal Strain = $\frac{\text{Change in length due to change in temperature}}{\text{Original length}}$

$$\varepsilon = \frac{l\alpha t}{l}$$

$$\varepsilon = \alpha t$$

3. Thermal stress for yielding and non-yielding supports:

(a) For supports do not yield:

We know that, thermal strain, $\varepsilon = \alpha t$

: thermal stress,

$$\sigma = \varepsilon.E$$

$$\therefore \quad \sigma = \alpha t E$$

 $\therefore \quad E = \frac{\varepsilon}{\alpha}$

(b) For supports are yielding:

let, yielding of supports = δ

$$\therefore \quad \delta l = l \alpha t - \delta$$

$$\therefore \quad \frac{\delta l}{l} = \alpha t - \frac{\delta}{l}$$

$$\therefore \quad \boxed{\varepsilon = \alpha t - \frac{\delta}{l}} \quad \text{ Thermal strain}$$

Now,

$$\sigma = \epsilon . E$$

$$\sigma = \left(\alpha \, t - \frac{\delta}{l}\right) \, \mathbf{E} \quad \dots \quad \text{Thermal stress}$$

• Thermal stress in bars of tapering circular section :

$$\sigma = \alpha \ tE \frac{d_1}{d_2} \dots \text{ maximum stress}$$

$$\sigma = \alpha \ tE \frac{d_2}{d_1} \dots$$
 minimum stress

where, $d_1 = dia$, at larger end

 $d_2 = dia.$ at smaller end

• Thermal stress in compound bars:

For compound bar,

$$P_1 = P_2$$

$$\therefore \quad \sigma_1 A_1 = \sigma_2 A_2 \dots (i)$$

$$\delta l = \delta l_1 + \delta l_2$$

Objective Civil Eng. \ 2016 \ 9

$$= \frac{\sigma_1 l_1}{E_1} + \frac{\sigma_2 l_2}{E_2} \dots (ii)$$

• Thermal stresses in composite bars:

For composite bar,

$$P_1 = P_2$$

$$\therefore \quad \sigma_1 A_1 = \sigma_2 A_2 \dots (i)$$

$$\epsilon_1 = \frac{\sigma_1}{E_1}$$

$$\epsilon_2 = \frac{\sigma_2}{E_2}$$

2.5 STRAIN ENERGY AND IMPACT LOADING:

1. Strain Energy (u):

When a body is strained within elastic limit, energy stored in it. This energy is called strain energy.

Strain energy = work done on a body.

$$\therefore \quad u = \frac{\sigma^2}{2E} \times v$$

where,

u = Strain energy

 $\sigma = Stress$

v = Volume of a bar

The unit of strain energy is N.m.

2. Resilience:

Total strain energy stored in a body, within elastic limit is called resilience.

Resilience =
$$u = \frac{\sigma^2}{2E} \times v$$

3. proof resilience:

The maximum strain energy that can be stored in a body at elastic limit is called **Proof resilience.**

$$\therefore \text{ Proof resilience} = u_p = \frac{(\sigma_E)^2}{2E} \times v$$

where, σ_E = Stress at elastic limit

4. Modulus of resilience (u_m):

The maximum strain energy that can be stored in a body per unit volume, at elastic limit is called modulus of resilience.

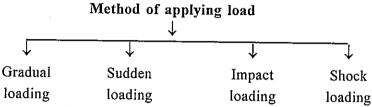
Modulus of resilience =
$$\frac{\frac{(\sigma_E)^2}{2E} \times v}{v} = \frac{(\sigma_E)^2}{2E}$$

The unit of modulus of resilience is N.mm/mm³

5. Instantaneous stress:

When a body is subjected to sudden load or Impact load the stress produced is called instantaneous stress.

6. Methods of applying load:



Gradual load: The load is increasing gradually from zero to P.

Stress,
$$\sigma = \frac{P}{A}$$

· Sudden load: When a body is subjected to total load P at a time, without small increments, it is called sudden load.

Stress,
$$\sigma = \frac{2P}{A}$$

· Impact load: When a load fall on a body from some height, it is called impact load.

$$\sigma = \frac{P}{A} \left[1 + \sqrt{1 + \frac{2EAh}{P.1}} \right]$$

Strain energy is,

$$u = P (h + \delta l)$$

$$\therefore \frac{\sigma^2}{2E} \times V = P (h + \delta l) \dots \text{ equation to find impact load.}$$

where,

 σ = stress due to impact load

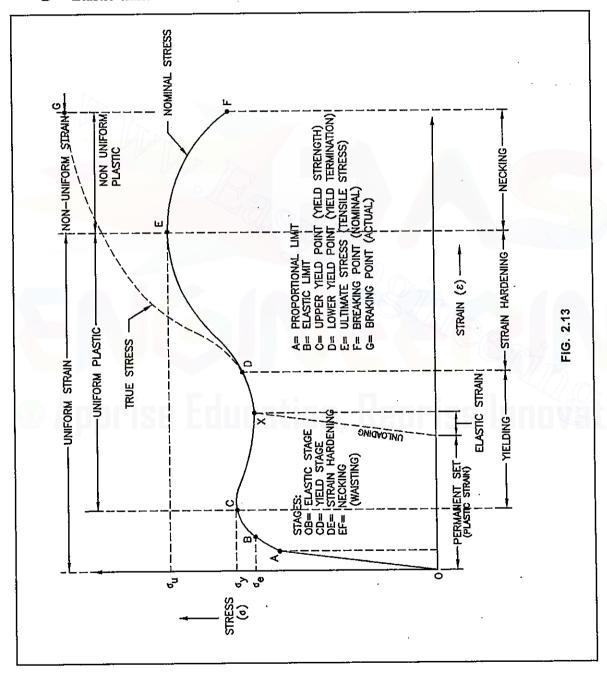
P = Impact load

h = height of fall of load

 δl = deformation of a body

2.6 TENSION TEST ON MILD STEEL:

- · Tension test specimen:
- · Stress-strain curve for tension test on M.S:
- A = Proportional limit
- B = Elastic limit



69

C = Upper yield point

D = Lower yield point

E = ultimate load point

F = Breaking point (Nominal)

G = Breaking point (Actual)

Proportional limit: The limit of stress up to which, stress is proportional to strain is called proportional limit.

Elastic limit: The value of force up to and within which, the deformation entirely disappear on removal of the force is called elastic limit.

Yield stress: When specimen is stressed beyond elastic limit, Strain increases more rapidly than the stress. Because, sudden elongation of the specimen takes place, without appreciable increase in the stress (or load). This phenomena is known as yielding. The Stress corresponding to point "C" in the graph is called yield stress.

Yield stress =
$$\frac{\text{yield load}}{\text{original c/s area}}$$

$$\therefore \quad \sigma_{y} = \frac{P_{y}}{A_{o}}$$

$$A_0 = \frac{\pi}{4} \times (d_0)^2$$

 $A_0 = \frac{\pi}{4} \times (d_0)^2$ $d_0 = \text{Original diameter of bar}$

ultimate stress:

ultimate stress =
$$\frac{\text{ultimate load (Maxi. load)}}{\text{original c/s area}}$$

$$\therefore \quad \sigma_{u} = \frac{P_{u}}{A_{o}}$$

Nominal breaking stress:

Breaking load Nominal breaking stress = original c/s area

$$\therefore \sigma_b = \frac{P_b}{A_o}$$

Actual breaking stress:

Actual breaking stress =
$$\frac{\text{Breaking load}}{\text{Final c/s area}}$$

$$\therefore \quad \sigma_b = \frac{P_b}{A}$$

$$A' = \frac{\pi}{4} \times (d')^2$$

d' = Final diameter

Percentage elongation:

% elongation =
$$\frac{\text{(Final G.L. - original G.L.)}}{\text{original G.L.}} \times 100 \%$$

$$= \frac{(L - L_o)}{L_o} \times 100 \%$$

For mild steel % elongation is 23 to 25%

Percentage reduction in area:

% Reduction in area =
$$\frac{\left(\text{original c/s} - \text{Final c/s}\right)}{\text{area}} \times 10$$

$$= \frac{(A_o - A')}{A_o} 100\%$$

For mild steel % reduction in area is about 40 to 65%

Gauge length: During tension test on mild steel bar, two points A and B are marked at equal distance from centre. This length AB is known as original gauge length.

Minimum gauge length =
$$L_0 = 5.65 \sqrt{A_0}$$

A_o = Original c/s area



3: Shear force and Bending Moment diagrams

3.1 SHEAR FORCE: (SF):

Algebric sum of unbalanced vertical forces to the left or right side of the section is called **shear force** at that section.

Unit of S.F. is N or kN.

3.2 BENDING MOMENT (B.M.):

Algebric sum of moments to the left or right side of the section is called **Bending** moment at that section.

Unit of B.M.is N.M. or kN.m.

3.3 POINT OF CONTRAFLEXURE:

The point in a B.M. diagram at which, B.M. changes sign from +ve to -ve or -ve to +ve is called **Point of contraflexure.**

At point of contraflexure B.M. is zero.

3.4 RELATION BETWEEN S.F. AND B.M. :

1. The rate of change of S.F. w.r.t. distance (or slope of the S.F. curve) is equal to the intensity of loading.

$$\therefore \frac{\delta F}{\delta x} = w \dots (i)$$

2. The rate of change of B.M. w.r.t. distance (or slope of B.M. curve) is equal to the S.F. at the section.

$$\therefore \frac{\delta M}{\delta x} = -F \dots (ii)$$

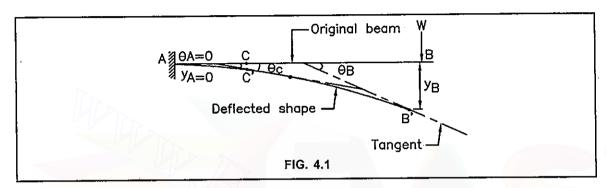
3. The point at which S.F. changes sign, B.M. will be maximum.



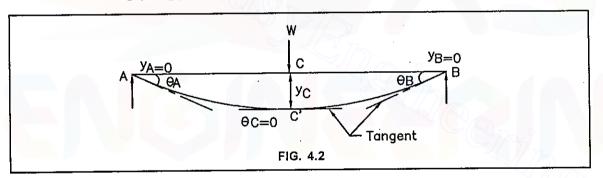
4: Slope and deflection of Beams

4.1 SLOPE AND DEFLECTION:

For cantilever beam:



For simply supported beam:



• Slope (θ) : At any point, angle made by tangent drawn to the deflected shape of a beam, with horizontal is called slope at that point.

The unit of slope is radian or degree.

It is denoted by θ .

1 degree =
$$\frac{\pi}{180}$$
 radian

OR

$$1 \text{ radian} = \frac{180}{\pi} \text{ degree}$$

Deflection (y):

At any point, vertical distance between the axis of original beam and the axis of deflected beam is known as deflection at that point.

73

unit of deflection is mm or cm.

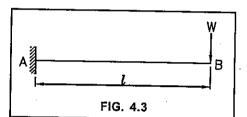
Flexural rigidity: Modulus of elasticity (E), Moment of inertia (I)

Flexural rigidity = EI

- 4.2 SLOPE DEFLECTION EQUATIONS:
 - 1. Cantilever beam with point load at free end:

$$\theta_{\rm B} = \frac{W l^2}{2 \, \rm EI}$$

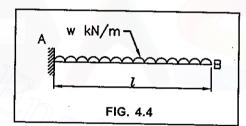
$$y_B = \frac{Wl^3}{3EI}$$



2. Cantilever beam with u.d.l. on entire span:

$$\theta_{\rm B} = \frac{{\rm w}l^3}{6\,{\rm E\,I}}$$

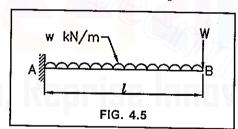
$$y_B = \frac{wl^4}{8EI}$$



3. Cantilever beam with point load at free end and u.d.l. on entire span :

$$\theta_{\rm B} = \frac{Wl^2}{2EI} + \frac{wl^3}{6EI}$$

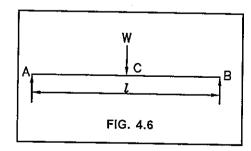
$$y_{B} = \frac{Wl^{3}}{3EI} + \frac{wl^{4}}{8EI}$$



4. Simply supported beam with central point load:

$$\theta_{A} = \theta_{B} = \frac{Wl^{2}}{16EI}$$

$$y_c = \frac{Wl^3}{48EI}$$



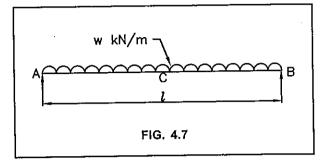
Objective Civil Eng. \ 2016 \ 10

74

5. Simply supported beam with u.d.l. on entire span:

$$\theta_{A} = \theta_{B} = \frac{wl^{3}}{24 \, \text{E} \, \text{I}}$$

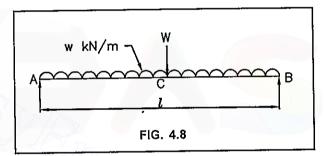
$$y_c = \frac{5}{384} \times \frac{wl^3}{EI}$$



6. Simply supported beam with central point load and u.d.l. on entire span:

$$\theta_{\rm A} = \theta_{\rm B} = \frac{Wl^2}{16EI} + \frac{wl^3}{24EI}$$

$$y_c = \frac{Wl^3}{48EI} + \frac{5}{384} \frac{wl^4}{EI}$$



Apprise Education, Reprise Innovations

5: Bending stresses in Beams and Shear Stresses

5.1 PURE BENDING STRESS:

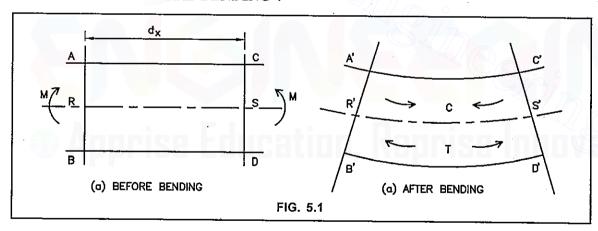
When a beam length is subjected to a constant amount of bending moment and a zero shear force, the stresses set up across the cross-section of the beam, due to bending, is known as **Pure bending stress**.

5.2 ASSUMPTIONS IN THE THEORY OF PURE BENDING:

The following assumptions are made in the theory of pure bending:

- 1. The material of the beam is perfectly homogeneous (i.e. of the same kind throughout) and isotropic (i.e. of equal elastic properties in all directions).
- 2. The beam is stressed within its elastic limit and Hook's law is valid.
- 3. The transverse sections, which were plane before bending remain plane after bending.
- 4. Each layer of beam is free to expand or contract, independently.
- 5. The value of Young's modulus (E) is the same in tension and compression.

5.3 THEORY OF PURE BENDING:



Consider a simply supported beam subjected to a bending moment M.

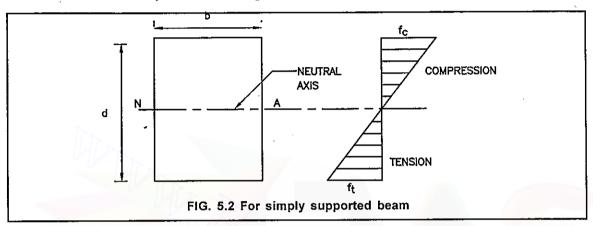
dx = small length of beam

Due to bending of beam, the top layer AC suffered compression and reduced to A'C'. As we proceed towards the lower layers of the beam, they also suffered compression, but to a lesser degree, until we come across the layer RS which has suffered no change in its length.

As we further proceed towards the lower layers, we find the layers have suffered tension. The layer BD has suffered maximum tension and has been stretched to B'D'.

• Neutral layer (Neutral plane):

Due to bending of beam, all the layers above RS layer are compressed and all the layers below RS layer are stretched. But, layer RS which is neither compressed nor stretched, is known as neutral layer or neutral plane.



Neutral Axis :

The line of intersection of the neutral layer, with any normal cross section of a beam is known as neutral axis of that section.

If section of a beam is subjected to pure sagging bending moment, stresses above N.A. are compressive and stresses below N.A. are tensile. At the neutral axis (NA), there is no stress of any kind.

• Moment of resistance:

When a beam length is subjected to a constant amount of bending moment, on one side of neutral axis there are compressive stresses and on the other there are tensile stresses. These stresses form a couple, whose moment must be equal to external moment M. The moment of this couple, which resists the external bending moment, is known as moment of resistance.

5.4 BENDING EQUATION:

 $\frac{M}{I} = \frac{f}{v} = \frac{E}{R}$... It is called bending equation or flexure equation.

where, M = Moment of resistance

I = M.I. of the section about x-x axis

f = bending stress (maxi)

y = distance of extreme fibre from N.A.

E = Modulus of elasticity

R = Radius of curvature of the beam

77

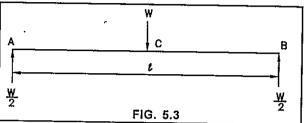
5.5 MAXIMUM BENDING MOMENT:

Maximum B.M. or Moment of Resistance (M) for some simple beams is given below.

(1) Simply supported beam with central point load:

Maxi. N.M. will occure at C.

$$M = \frac{W}{2} \cdot \frac{l}{2} = \boxed{\frac{Wl}{4}}$$
 (sagging)



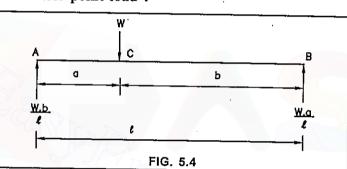
(2) Simply supported beam with eccentric point load:

Maxi, N.M. will occur at C.

$$\therefore M = \frac{Wa}{l} \cdot b = \frac{Wab}{l}$$

OR

$$M = \frac{Wb}{l} \cdot a = \frac{Wab}{l}$$
 (sagging)

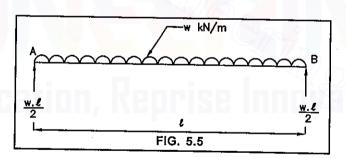


(3) Simply supported beam with U.D.L. on entire span:

Maxi B.M. will occur at the mid span

$$M = \frac{wl}{2} \times \frac{l}{2} - w \times \frac{l}{2} \times \frac{l}{4}$$
$$= \frac{wl^2}{4} - \frac{wl^2}{8}$$

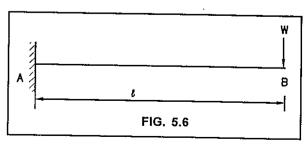
$$=\frac{wl^2}{8}$$
 (sagging)



(4) Cantilever beam with point load at free end:

Maxi. B.M. will occur at fixed end A.

$$\therefore M = W. L. \text{ (hogging)}$$



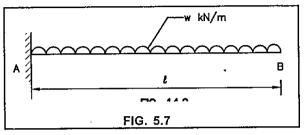
Objective Civil Engineering

78

(5) Cantilever beam with U.D.L. on entire span:

Maxi. B.M. will occur at fixed end A.

$$M = w. l. \frac{1}{2}$$
$$= \frac{wl^2}{2} \text{ (hogging)}$$



5.6 EQUATION OF SHEAR STRESS:

$$\tau = \frac{FA\overline{y}}{I.b}$$
where,

t = shear stress at a layer

F = Shear force

A = Area of the section above the layer where shear stress is required.

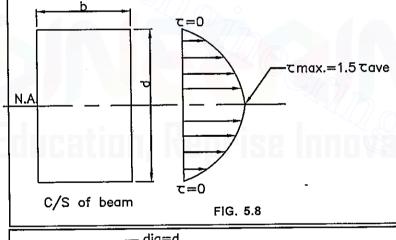
 \overline{y} = distance of c.g. of the area above layer from N.A.

I = M.I. about x-x axis

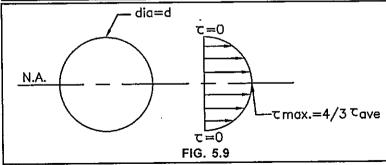
b = width of the layer on which shear stress is required.

5.7 SHEAR STRESS DISTRIBUTION DIAGRAMS:

(a) Rectangular Section:



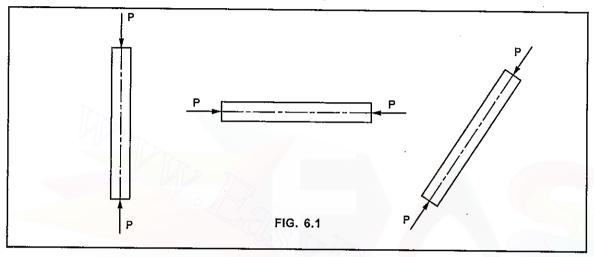
(b) Circular Section:



6: Columns and Struts

6.1 STRUT:

A structural member subjected to axial compressive force is called strut.



- · Strut may be vertical, horizontal or inclined.
- · The cross sectional dimensions of strut are small.
- · Normally, struts carry smaller compressive loads.
- · Struts are used in roof truss and bridge trusses.

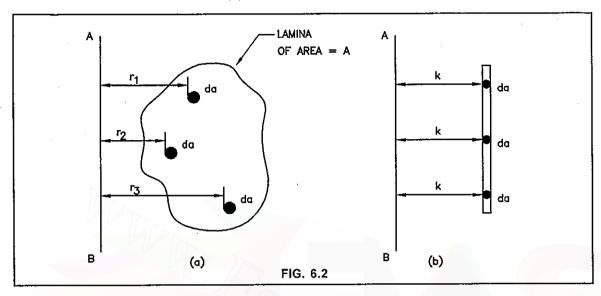
6.2 COLUMN:

- · When strut is vertical it is known as column.
- · The cross sectional dimensions of column are large.
- · Normally, columns carry heavy compressive loads.
- · Columns are used in concrete and steel buildings.

• Difference between strut and column:

| Strut | | Column | |
|-------|--|--|--|
| | It carry light axial compressive force. It may be vertical, horizontal or inclined. | It carry heavy compressive force. It is always vertical. | |
| | The c/s dimensions of strut are small. It is used in roof trusses and bridge truss. | The c/s dimensions of column are large. It is used in concrete and steel buildings. | |

6.3 RADIUS OF GYRATION (k):



The distance from the given axis at which, if all the small elements of the lamina are placed, the M.I. of the lamina about the given axis does not changes. This distance is called Radius of gyration.

Mathematically,

$$k = \sqrt{\frac{I}{A}}$$

where,

k = radius of gyration

I = Moment of inertia

A = c/s area

6.4 SLENDERNESS RATIO : (λ) :

effective length of Column

Slenderness ratio = $\frac{1}{\text{Minimum radius of gyration}}$

$$\therefore \quad \lambda = \frac{l_e}{k_{min}}$$

- \Rightarrow If λ for column is more, its load carrying capacity will be less.
- \Rightarrow If λ for column is less, its load carrying capacity will be more.

6.5 LONG COLUMN:

When length of column is more as compared to its cross - sectional dimension, it is called long column.

For long columns,

$$\frac{l_{e}}{d} \ge 12$$
OR
$$\lambda = \frac{l_{e}}{k_{min}} \ge 50$$

where,

 $l_{\rm e}$ = effective length of column

d = least lateral dimension of column.

k_{min} = minimum radius of gyration.

For mild steel if $\lambda \geq 80$, it is called long column.

6.6 SHORT COLUMN: When length of column is less as compared to its cross - sectional dimension, it is called short column. For short columns,

$$\frac{l_{\rm e}}{\rm d} < 12$$
OR
$$\lambda = \frac{l_{\rm e}}{\rm k_{\rm min}} < 50$$

where,

le = effective length of column

d = least lateral dimension of column.

k_{min} = minimum radius of gyration.

6.7 CRUSHING LOAD:

In case of short columns, with increase in axial compressive load, compressive stress increases. After some load, the column fails by crushing.

The load at which, short column fails by crushing is called crushing load.

The stress corresponding to crushing load is called crushing stress.

6.8 CRIPPLING LOAD OR BUCKLING LOAD OR CRITICAL LOAD:

In case of long columns, with increase in axial compressive load, compressive stress increases. After some load, the column starts buckling (bending) and bending stress also produces. Finally, the column fails by buckling.

The load at which, long column starts buckling is called buckling load or crippling load.

· Buckling load is always less than crushing load.

Buckling of column depends upon the following factors.

- 1. Amount of load
- 2. Length of column
- 3. End conditions of column
- 4. Cross sectional dimensions of column
- 5. Material of column.

Objective Civil Eng. \ 2016 \ 11

82

6.9 COLUMN END CONDITIONS AND EFFECTIVE LENGTH:

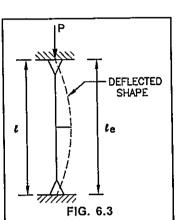
We may consider the following four end conditions.

- 1. Both ends hinged.
- 2. Both ends fixed
- 3. One end fixed and other hinged.
- 4. One end fixed and other free.
- (i) Both ends hinged:

$$l_{\rm e} = l$$

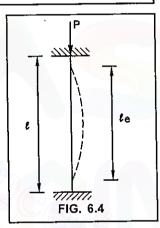
where

 $l_{\rm e}$ = effective length of column l = actual length of column



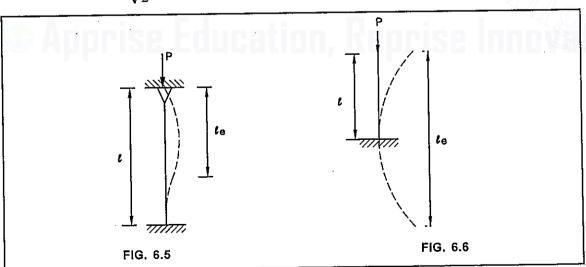
(ii) Both ends fixed:

$$l_{\rm e} = \frac{l}{2}$$



(iii) One end fixed and other hinged:

$$l_{\rm e} = \frac{l}{\sqrt{2}}$$



83

(iv) One end fixed and other free:

$$l_{\rm e} = 2l$$

6.10 EULER'S FORMULA FOR CRIPPLING LOAD:

$$P_{E} = \frac{\pi^{2} E I}{(l_{e})^{2}}$$

where,

P_E = Euler's crippling load

E = Modulus of elasticity

I = M.I. (Minimum)

 $l_{\rm e}$ = Effective length of column

6.11 ASSUMPTIONS OF EULER'S FORMULA:

1. The material of column is elastic, homogeneous and isotropic.

2. The column is long.

3. The load on column is truely axial.

4. Failure of column is due to buckling.

5. The cross section of column is uniform throughout its length.

6. The Hooke's law is valid.

7. The column is straight before application of load.

8. The shortening of column due to axial compressive load is neglected.

• Limitation of Euler's formula:

For mild steel,

If $\lambda \leq 80$ short column

If $\lambda > 80$ long column

Euler's formula is valid for long columns only.

• Rankine Formula:

$$P_{R} = \frac{P_{c} \cdot P_{E}}{P_{c} + P_{E}}$$

where,

P_c = crushing load (for short column)

P_E = Euler's crippling load (for long column)

P_R = Ranking crippling load

 $P_{R} = \frac{f_{c}. A}{1 + \alpha \left(\frac{le}{k}\right)^{2}}$

where,

le = effective length of column

k = minimum radius of gyration

 $\alpha = \frac{1}{7500}$ for mild steel

7: Principal Planes and Principal stresses

7.1 PRINCIPAL PLANE:

The plane on which only direct stress (normal stress) is acting is called principal plane.

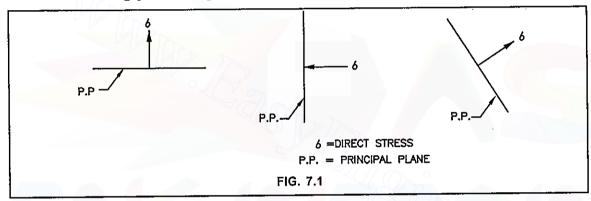
 σ = direct stress

P.P. = Principal plane

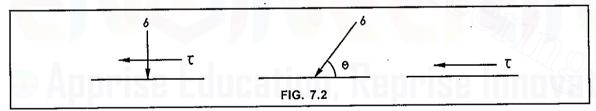
The magnitude of shear stress (τ) on principal plane is zero.

The principal plane may be horizontal, vertical or inclined.

The following planes are principal planes. Fig. 7.1

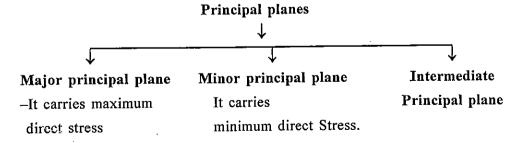


The following planes are not principal planes. Fig. 7.2



7.2 PRINCIPAL STRESS:

The magnitude of direct stress across a principal plane is known as principal stress.



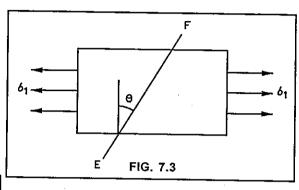
7.3 DIFFERENT CASES OF STRESSES IN MATERIALS:

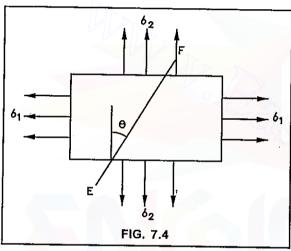
Case – 1: Only one direct stress (σ_1)

is acting:

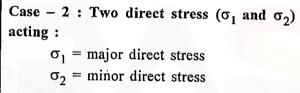
 σ_1 = major direct stress

 θ = angle of inclined plane EF with vertical plane.

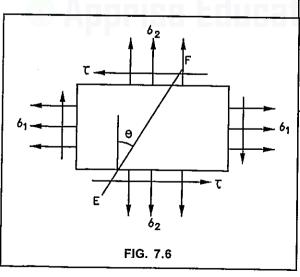


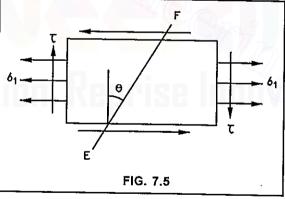


 σ_1 = major direct stress τ = shear stress



Case -3: One direct stress (σ_1) and shear stress (τ) are acting:





Case -4: Two direct stresses and shear stress are acting:

 σ_1 = major direct stress

 σ_2 = minor direct stress

 $\tau = shear stress$

In the above four cases the nature of σ_1 and σ_2 may be tensile or compressive.

7.4 EQUATIONS OF σ_n , σ_t , σ_r :

$$\sigma_{n} = \frac{(\sigma_{1} + \sigma_{2})}{2} + \frac{(\sigma_{1} - \sigma_{2})}{2} \cdot \cos 2 \theta + \tau \sin 2 \theta$$

$$\sigma_{t} = \frac{(\sigma_{1} - \sigma_{2})}{2} \sin 2 \theta - \tau \cos 2 \theta$$

$$\sigma_{r} = \sqrt{\sigma_{n}^{2} + \sigma_{t}^{2}}$$

where,

 σ_n = Normal stress on EF plane

 σ_t = Tangential stress on EF plane

 σ_r = Resultant stress on EF plane

 σ_1 = major direct stress (bigger)

 σ_2 = Minor direct stress (Smaller)

 τ = shear stress

 θ = angle of inclined plane EF with vertical plane.

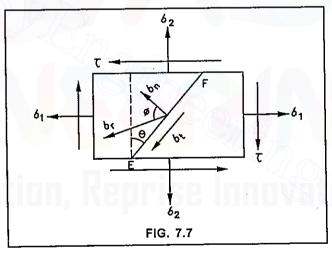
- The above equations of σ_n , σ_t , σ_r can be applied to any of the four cases.
- To use the equations of σ_n , σ_t , σ_r

For case
$$-1$$
, $\sigma_2 = 0$

 $\tau = 0$

For case
$$-2$$
, $\tau = 0$

For case -3, $\sigma_2 = 0$



7.5 SIGN CONVENTIONS:

Take σ_1 as always positive (whether tensile or compressive). If the nature of σ_2 is similar to σ_1 , take σ_2 as positive.

If the nature of σ_2 is opposite to σ_1 , take σ_2 as negative.

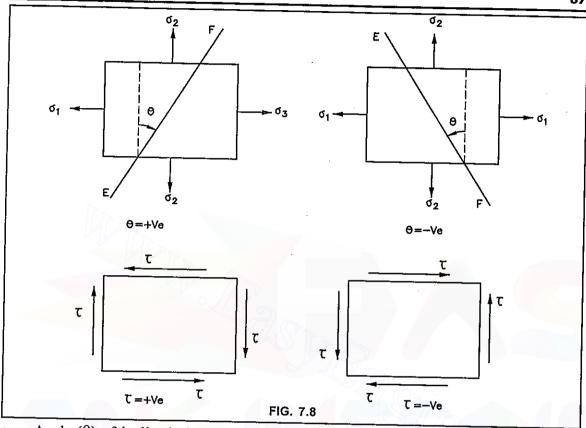
For example,

If,
$$\sigma_1$$
 is tensile
$$\sigma_2$$
 is tensile
then take,
$$\sigma_1 = + \text{ ve}$$

$$\sigma_2 = + \text{ ve}$$

If,
$$\sigma_1$$
 is tensile
 σ_2 is compressive
then take,
 $\sigma_1 = + \text{ ve}$
 $\sigma_2 = - \text{ ve}$

87



Angle (θ) of inclined plane EF with vertical measured clockwise is taken positive.

- θ clockwise = +ve
- θ Anticlockwise = -ve
- · Clockwise shear stress (τ) on vertical plane is taken +ve.
- · Anticlockwise shear stress (τ) on vertical plane is taken -ve.

For Graphical method (Mohr circle Method):

- Draw σ_1 , always on right side of O.
- If nature of σ_2 is similar to σ_1 , then draw σ_2 also on right side of O.
- · If nature of σ_2 is opposite to σ_1 , then draw σ_2 on left side of O.
- If $\tau = +$ ve draw below OX line
 - If $\tau = -$ ve draw above OX line.
- If $\theta = +$ ve, measure 2θ above OX line
 - If $\theta = -ve$, measure 2θ below OX line
- · If shear stress (τ) is acting, CA' line represents vertical plane.
 - If θ = + ve, measure 2θ above CA' line

If $\theta = -ve$, measure 2θ below CA' line.

If σ_i is above OX line, it is + ve

If σ_t is below OX line, it is - ve

7.6 PRINCIPAL PLANES AND PRINCIPAL STRESSES:

$$\sigma_{n1} = \frac{\sigma_1 + \sigma_2}{2} + \sqrt{\left(\frac{\sigma_1 - \sigma_2}{2}\right)^2 + \tau^2}$$

$$\sigma_{n2} = \frac{\sigma_1 + \sigma_2}{2} - \sqrt{\left(\frac{\sigma_1 - \sigma_2}{2}\right)^2 + \tau^2}$$

$$\sigma_{tmax} = \frac{\sigma_{n1} - \sigma_{n2}}{2}$$

$$\tan 2\alpha_1 = \frac{2\tau}{\sigma_1 - \sigma_2}$$

$$\alpha_2 = \alpha_1 + 90^\circ$$

where,

σn₁ = Major principal stress

σn₂ = Minor principal stress

 σ_{tmax} = Maxi. shear stress (Maxi. tangential stress)

 α_1 = Angle of major P.P. with vertical plane

 α_2 = Angle of minor P.P. with vertical plane.

• Angle of obliquity (ϕ) : The angle made by resultant stress with normal stress is called angle of obliquity.

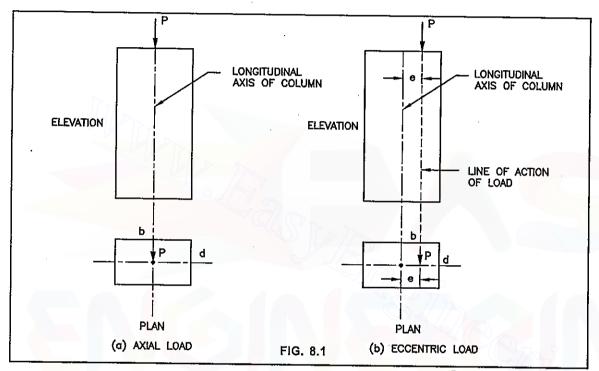
$$\tan \phi = \frac{\sigma_t}{\sigma_n}$$



8: Combined Direct and Bending Stress

8.1 AXIAL LOAD AND ECCENTRIC LOAD:

(1) Axial load: If load is acting on the longitudinal axis of column, it is called axial load. For axial load e = 0



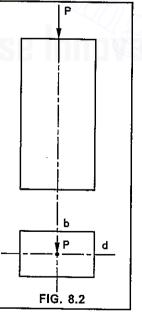
(2) Eccentric load:

If load is acting away from the longitudinal axis of column, it is called eccentric load.

(3) Eccentricity (e):

The horizontal distance between the longitudinal axis of column and the line of action of load is called eccentricity.

e = eccentricity.



Objective Civil Eng. \ 2016 \ 12

8.2 STRESS DUE TO ECCENTRIC LOAD:

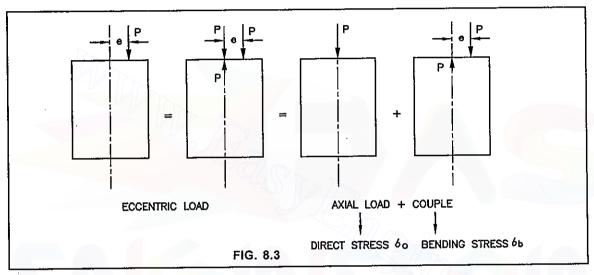
P = axial load

Axial load produces only direct stress

$$\sigma_o = \frac{P}{A}$$
 where, $\sigma_o = \text{direct stress}$

But, eccentric load produces both direct stress (σ_o) and bending stress (σ_b).

Let us consider the following figure.



Eccentric load _____ Axial load

couple

It produces

It produces

direct stress (σ_0)

bending stress (σ_b)

Thus, eccentric load produces both direct stress (σ_0) and bending stress (σ_b).

• Maximum and minimum stresses in column:

$$\begin{aligned} \mathbf{Maximum stress} & (\sigma_{max}) \\ \sigma_{max} &= \frac{\text{direct}}{\text{Stress}} + \frac{\text{bending}}{\text{stress}} \\ &= \sigma_o + \sigma_b \\ &= \frac{P}{A} + \frac{M}{Z} \\ &= \frac{P}{A} + \frac{M}{I}.y \end{aligned}$$

$$\sigma_{min} = \frac{\text{direct}}{\text{stress}} - \frac{\text{bending}}{\text{stress}}$$

$$= \sigma_{o} - \sigma_{b}$$

$$= \frac{P}{A} - \frac{M}{Z}$$

$$= \frac{P}{A} - \frac{M}{I}.y$$

91

where,

 σ_0 = direct stress

 σ_b = bending stress

M = Moment = P.e.

e = eccentricity

 $Z = Section modulus = \frac{I}{y}$

I = M. I.

y = distance of extreme fibre from c.g.

Sign conventions:

Compressive stress = + ve

Tensile stress = - ve

σ_{max} is always compressive (+)

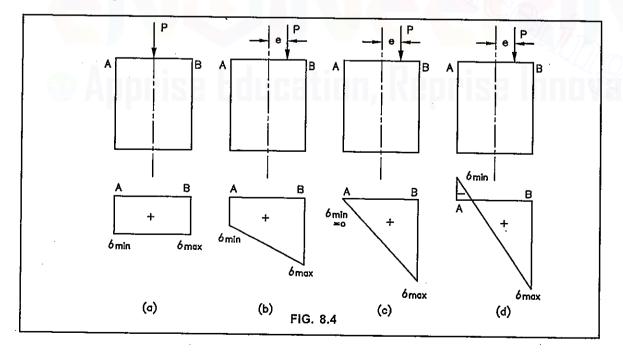
If $\sigma_o > \sigma_b$, σ_{min} is compressive (+)

If $\sigma_o < \sigma_b$, σ_{min} is tensile (-)

8.3 STRESS DISTRIBUTION IN COLUMN:

For a column, Let's understand how stresses at two edges of column changes as the load moves away from the centre.

(a) When load is axial (e = 0), stresses at both the edges of column will be same and compressive in nature.



- 92
- (b) As the load moves away from the centre, compressive stress at B increases and compressive stress at A decreases.
- (c) As the load moves away from the centre, σ_{\min} gradually decreases and becomes zero. The eccentricity of load when $\sigma_{\min} = 0$ is called limit of eccentricity.
- (d) If load goes beyond limit of eccentricity, the value of σ_{min} becomes negative i.e. stress at A becomes tensile.

8.4 LIMIT OF ECCENTRICITY:

The maximum distance of load from the centre of column, up to which there is no tensile stress in column, is known as limit of eccentricity.

When load is at limit of eccentricity,

$$\sigma_{\min} = 0$$
 or $\sigma_{o} = \sigma_{b}$

• No tension condition:

Column is a compression member. There should not be tension in the column. If tensile stress produced in the column, it indicates failure of the column.

Hence, for "no tension" in column,

- · Load must act within limit of eccentricity.
- The value of σ_{min} should not be negative.

•
$$\sigma_o \ge \sigma_b$$

$$\therefore \quad \frac{P}{A} \ge \frac{M}{Z}$$

$$\therefore \quad \frac{P}{A} \ge \frac{P.e}{Z}$$

$$\therefore \quad \frac{Z}{A} \ge e$$

$$\therefore$$
 $e \le \frac{Z}{A}$... No tension condition.

8.5 CORE OF SECTION OR KERNAL:

Core of section: The central part of column joining the points of elimit, is known as core of section or kernal.

If load act within the area of core, there is no tension in the column.

(1) Rectangular section:

$$e \le \frac{Z}{A}$$

$$Z_{xx} = \frac{I_{xx}}{y} = \frac{\frac{bd^3}{12}}{\frac{d}{2}} = \frac{bd^2}{6}$$

$$A = b.d$$

$$e \le \frac{Z}{A}$$

$$e \le \frac{bd^2}{6}$$

$$\therefore e \leq \frac{d}{6}$$

Similarly :





$$e \le \frac{Z}{A}$$

$$Z_{xx} = \frac{I_{xx}}{y}$$

$$= \frac{\frac{BD^3 - bd^3}{12}}{\frac{D}{2}}$$
$$= \frac{BD^3 - bd^3}{6D}$$

$$A = BD - bd$$

$$\therefore \qquad e \le \frac{Z}{A}$$

$$\leq \frac{BD^3 - bd^3}{6D}$$

$$\leq \frac{BD - bd}{BD - bd}$$

$$e \le \frac{(BD^3 - bd^3)}{6D(BD - bd)}$$

$$e = \frac{(DB^3 - db^3)}{6B(BD - bd)}$$

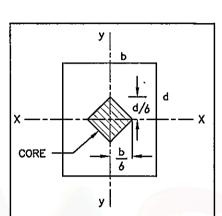
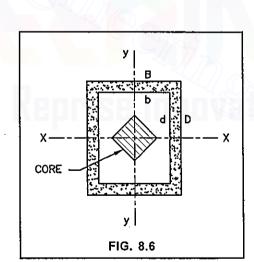


FIG. 8.5

93



94

Objective Civil Engineering

(3) circular Section:

$$e \le \frac{Z}{A}$$

$$Z_{xx} = \frac{I_{xx}}{y}$$

$$= \frac{\frac{\pi}{64} \times d^4}{\frac{d}{2}} = \frac{\pi}{32} \cdot d^3$$

$$A = \frac{\pi}{4} d^2$$

$$e \le \frac{Z}{A}$$

$$\leq \frac{\frac{\pi}{32}d^3}{\frac{\pi}{4}d^2} \leq \frac{d}{8}$$

$$x - \frac{d}{8}$$

CORE

FIG. 8.7

$$\therefore e \le \frac{d}{8}$$

(4) Hollow circular Section:

$$e \le \frac{Z}{A}$$

$$Z_{xx} = \frac{I_{xx}}{y}$$

$$=\frac{\frac{\pi}{64}\left(D^4-d^4\right)}{\frac{D}{2}}$$

$$=\frac{\pi}{32}\frac{(D^4-d^4)}{D}$$

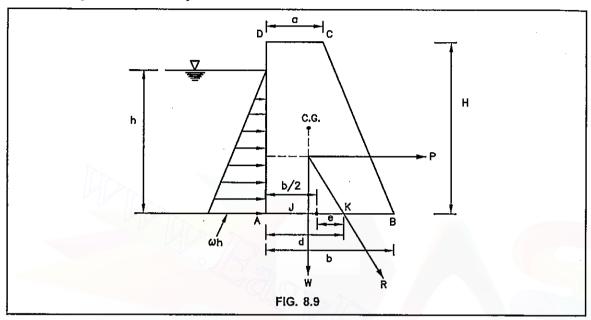
$$A = \frac{\pi}{4} (D^2 - d^2)$$

$$\therefore e^{\frac{Z}{A}} \leq \frac{\frac{\pi}{32} \frac{\left(D^4 - d^4\right)}{D}}{\frac{\pi}{4} \left(D^2 - d^2\right)} = \frac{1}{8D} \frac{\left(D^2 + d^2\right) \left(D^2 - d^2\right)}{\left(D^2 - d^2\right)}$$

$$e^{\frac{Z}{A}} \leq \frac{\frac{\pi}{32} \frac{\left(D^4 - d^4\right)}{D}}{\frac{\pi}{4} \left(D^2 - d^2\right)} = \frac{1}{8D} \frac{\left(D^2 + d^2\right) \left(D^2 - d^2\right)}{\frac{\pi}{8D}}$$

8.6 MAXIMUM AND MINIMUM PRESSURE AT THE BASE OF DAM:

The figure shows a trapezoidal dam section.



where,

a = top width of dam

b = Bottom width of dam

H = Total height of dam

h = height of water

P = Total water pressure acting on dam

W = Total weight of dam

R = Resultant of P and W

(1) Weight of Dam: (For 1 m length)

Weight = cross sectional area of dam × density of dam material

$$\therefore \quad W = (a+b) \times \frac{H}{2} \times \rho$$

where, $\rho = \text{density of dam material in kN/m}^3$

(2) Total water pressure on dam: (For 1 m length)

Total water pressure = Area of water pressure diagram

$$\therefore \quad P = \frac{1}{2} \times wh \times h$$

$$\therefore \quad P = \frac{wh^2}{2}$$

Objective Civil Engineering

where, w = density of water= 1000 kg/m³

 $= 10 \text{ kN/m}^3$

(3) Eccentricity (e):

96

Total water pressure (P) acts horizontally at height $\frac{h}{3}$ from the base of dam. Total weight of dam (W) acts vertically downwards.

R is the resultant of P and W.

$$R = \sqrt{P^2 + W^2}$$

Resultant (R) cut the base at point K.

distance
$$JK = x = \frac{P}{W} \times \frac{h}{3}$$

distance AJ =
$$\frac{a^2 + ab + b^2}{3(a + b)}$$

$$\therefore d = AJ + JK$$

$$\therefore \quad \text{eccentricity} = \boxed{e = d - \frac{b}{2}}$$

(4) Maximum and Minimum Pressure:

Maximum Pressure,

$$\sigma_{\text{max}} = \frac{W}{b} \left(1 + \frac{6e}{b} \right)$$

Minimum Pressure,

$$\sigma_{\min} = \frac{W}{b} \left(1 - \frac{6e}{b} \right)$$

- Stability conditions for Dam:
 - (i) No tension at base:

$$e \le \frac{b}{6}$$

(ii) No overturning:

Resultant R must cut within base AB.

(iii) No Sliding:

Resisting force > sliding force

$$\mu W > P$$

(iv) No Crushing:

$$\sigma_{\text{max}} < \sigma_{\text{c}}$$

 σ_{max} = maximum stress produced at base

 σ_c = Permissible crushing stress of dam material.



9: Thin Cylindrical Shells

- If $t \le \frac{d}{10}$ thin cylindrical shell
 - If $t > \frac{d}{10}$ thick cylindrical shell

9.1 FOR THIN CYLINDRICAL SHELL:

(i) Hoop Stress or circumferential stress:

$$\sigma_{\rm c} = \frac{pd}{2t}$$

p = internal pressure

d = internal dia.

t = wall thickness

(ii) Longitudinal stress:

$$\sigma_{l} = \frac{pd}{4t}$$

for riveted joints,

$$\sigma_{\rm c} = \frac{\rm pd}{2\rm t\eta}, \qquad \sigma_{\rm f} = \frac{\rm pd}{4\rm t\eta}$$

 $\eta = efficiency of joint$

(iii) Circumferential strain:

$$\varepsilon_1 = \frac{\sigma_c}{E} - \frac{\sigma_l}{mE} = \frac{pd}{2tE} \left(1 - \frac{1}{2m} \right)$$

(iv) Longitudinal strain:

$$\varepsilon_2 = \frac{\sigma_I}{E} - \frac{\sigma_c}{mE} = \frac{pd}{2tE} \left(\frac{1}{2} - \frac{1}{m}\right)$$

 \therefore Change in dia. $\delta d = \epsilon_1 \cdot d$

 \therefore Change in length $\delta l = \epsilon_2 \cdot l$

(v) Change in volume : $\delta v = v (\epsilon_2 + 2\epsilon_1)$

$$\tau_{\text{max}} = \frac{\sigma_{\text{c}} - \sigma_{\text{l}}}{2} = \frac{\text{pd}}{8t}$$

9.2 THIN SPHERICAL SHELL:

stress,
$$\sigma = \frac{pd}{4t}$$

$$\varepsilon = \frac{\delta d}{d} = \frac{pd}{4tE} \left(1 - \frac{1}{m} \right)$$

change in volume,

$$\delta v = \frac{\pi . \ p.d^4}{8tE} \left(1 - \frac{1}{m} \right)$$

Objective Civil Eng. \ 2016 \ 13



10: Torsion and Springs

10.1 TERMS RELATED TO TORSION:

1. Torsion:

In factories and workshops, shaft is used to transmit energy from one end to the other end.

When turning force (P) is applied on the pulley mounted on the shaft deformation is produced in the shaft. This deformation is called 'torsion'.

2. Torque or turning moment or twisting moment (T):

To transmit the energy by shaft, turning force (P) is applied on the shaft. Due to this turning force, moment is produced in the shaft. This moment is known as 'torque'.

Turning moment = $P \times 2R$

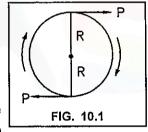
$$T = P \times 2R$$

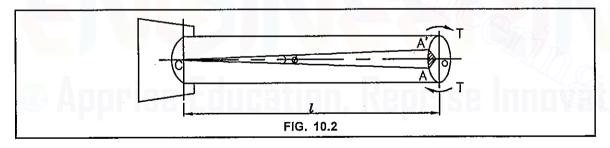
Torque is denoted by T.

Unit of torque is N.m or kN.m.

Shear strain in shaft (φ):

When shaft is subjected to torque (T), line CA on the surface of the shaft comes to new position CA'. The angular deformation ϕ is known as shear strain in shaft.



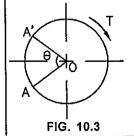


4. Angle of twist (θ) :

When shaft is subjected to torque (T), point A on the surface of the shaft comes to new position A'. The angle AOA' at the centre of the shaft is known as angle of twist.

5. Shear stress in shaft (τ) :

When shaft is subjected to torque (T), a stress resisting the torque is produced in the material of the shaft. This stress is known as **shear** stress in shaft.



Shear stress = shear strain \times modulus of rigidity





$$\tau = \phi \times C$$

6. Polar moment of inertia (J):

The moment of inertia of a plane area, with respect to an axis perpendicular to the plane of the lamina is called **polar moment of inertia**.

For circular shaft:

Polar M.I. =
$$I_{zz} = I_{xx} + I_{yy}$$

= $\frac{\pi}{64} \times D^4 + \frac{\pi}{64} \times D^4$
$$J = \frac{\pi}{32} \times D^4$$

unit of J is mm⁴

7. Polar section modulus (Z_p) :

Polar section modulus = $\frac{\text{Polar M.I.}}{\text{Radius of shaft}}$

$$\therefore \quad Z_{p} = \frac{J}{R}$$
unit of Z_{p} is mm³

8. Torsional rigidity:

Torsional rigidity = Modulus of rigidity × polar M.I.

$$= C \times J$$

unit is N.mm²

10.2 EQUATION OF TORSION:

$$\frac{T}{J} = \frac{C \cdot \theta}{l} = \frac{\tau}{R}$$
 where, $T = Torque (N.mm)$
$$J = Polar M.I. (mm^4)$$

$$C = Modulus of rigidity (N/mm^2)$$

$$\theta = angle of twist (radians)$$

$$\tau = shear stress in shaft (N/mm^2)$$

$$R = radius of shaft (mm)$$

10.3 ASSUMPTIONS IN THE THEORY OF TORSION:

- 1. The material of shaft is uniform throughout the length.
- 2. The twist along the shaft is uniform.
- 3. The cross section of shaft is uniform throughout the length.
- 4. The cross sections of the shaft, which are plane before twist, remain plane after twist.
- 5. All radii which are straight before twist remain straight after twist.

10.4 IMPORTANT EQUATIONS:

Solid circular shaft

(i) Torque (T):

$$T = \frac{\pi}{16} \times \tau \times D^3$$

(ii) Polar M.I. (J):

$$J = \frac{\pi}{32} \times D^4$$

(iii) Polar section Modulus (Z_P)

$$Z_{\rm P} = \frac{\pi}{16} \times D^3$$

10.5 EQUATIONS OF POWER:

(a) Power in horse power (h.P.):

$$P = \frac{2\pi NT}{4500} \text{ h.p.}$$

where, P = Power in h.p.

Hollow circular shaft

 $T = \frac{\pi}{16} \times \tau \times \left(\frac{D^4 - d^4}{D} \right)$

 $J = \frac{\pi}{32} \times (D^4 - d^4)$

 $Z_p = \frac{\pi}{16} \times \frac{\left(D^4 - d^4\right)}{D}$

$$N = R.P.M.$$
 $T = Torque (kg.m)$

(b) Power in watt:

$$P = \frac{2\pi NT}{60} \text{ watt}$$

where, T = Torque (N.m)

1 kW = 1000 watt

1 H.P. = 746 watt = 0.746 kW

10.6 SHAFT SUBJECTED TO COMBINED BENDING AND TORSION:

When a shaft is subjected to twisting moment (T) and bending moment (M),

(i) bending stress produced in shaft :

$$\sigma = \frac{M}{I} \times y = \frac{M}{\frac{\pi}{64} \times D^4} \times \frac{D}{2} = \frac{32 \text{ M}}{\pi D^3}$$

(ii) Shear stress produced in shaft

$$\tau = \frac{T}{J} \times R = \frac{T}{\frac{\pi}{32} \times D^4} \times \frac{D}{2} = \frac{16T}{\pi D^3}$$

$$\frac{\sigma}{\tau} = 2$$

$$\sigma_{n_1=} \frac{\sigma}{2} + \sqrt{\left(\frac{\sigma}{2}\right)^2 + \tau^2}$$

$$= \frac{16}{\pi D^3} \left[M + \sqrt{M^2 + T^2} \right]$$
Minor principal stress
$$(\text{Mini. normal stress})$$

$$\sigma_{n_2} = \frac{\sigma}{2} - \sqrt{\left(\frac{\sigma}{2}\right)^2 + \tau^2}$$

$$= \frac{16}{\pi D^3} \left[M - \sqrt{M^2 + T^2} \right]$$

$$\tau_{max} = \frac{\sigma_{n_1} - \sigma_{n_2}}{2} = \frac{16}{\pi D^3} \left[\sqrt{M^2 + T^2} \right]$$

where,

equivalent bending moment (Me)

$$Me = \frac{1}{2} \left(M + \sqrt{M^2 + T^2} \right)$$

equivalent twisting moment (Te)

$$Te = \sqrt{M^2 + T^2}$$

10.7 CLOSED-COIL HELICAL SPRING:

W. R =
$$\frac{\pi}{16} \times \tau \times d^3$$

d = dia. of spring wire

R = Radius of spring coil

deflection,

$$\delta = \frac{64 \text{ W R}^3 \text{ n}}{\text{C d}^4}$$

stiffness of spring,

$$S = \frac{W}{\delta} = \frac{C d^4}{64 R^3 n}$$

10.8 LEAF SPRING OR CARRIAGE SPRING:

Maximum bending stress,

$$\sigma = \frac{3Wl}{2nbt^2}$$

deflection.

$$\delta = \frac{3Wl^3}{8Enbt^3}$$

where,

w = load on the spring

l = span of spring

t = thickness of plates

b = width of plates

n = number of plates

E = Young's modulus of plate material

102

MCQ'S

| | | | · · · · · · · · · · · · · · · · · · · | | | | | | | |
|-----|---|--------|---------------------------------------|--------|------------------|--|--|--|--|--|
| 1. | Modulus of elasticity is the ratio of | | | | | | | | | |
| | (a) Stress to strain | (b) | Strain to stress | | | | | | | |
| | (c) Force to cross sectional area | (d) | Stress to origina | ıl len | gth. | | | | | |
| 2. | The property by virtue of which a body | retui | rns to its origina | l shaj | e after removal | | | | | |
| | of the force is called | | | | | | | | | |
| | (a) ductility (b) plasticity | (c) | Elasticity | (d) | malleability | | | | | |
| 3. | Beyond elastic limit, tensile strain | | | | | | | | | |
| | (a) increases more quickly. | (b) | decreases more | quick | ly. | | | | | |
| | (c) increase in proportion to stress. | (d) | decrease in prop | ortio | n to stress. | | | | | |
| 4. | The phenomenon of slow extension of a called | mater | ials with time u | nder | constant load is | | | | | |
| | (a) yielding (b) Creeping | (c) | breaking | -(d) | none of these | | | | | |
| 5. | Every material obeys Hooke's law with | ` ' | _ | ` / | | | | | | |
| | (a) elastic limit. | | plastic limit. | | | | | | | |
| | (c) limit of proportionality. | | None of these. | | | | | | | |
| 6. | The stress at which extension of a mat | erial | take place more | quic | kly as compared | | | | | |
| | to the increase in load, is called | | 77 | | 4.0 | | | | | |
| | (a) elastic point (b) Plastic point | (c) | breaking point | (d) | yield point | | | | | |
| 7. | The maximum stress produced in a ba | | | is at | | | | | | |
| | (a) Larger end (b) Smaller end | | middle | | any where | | | | | |
| 8. | In a composite section, the number of | diffe | rent materials is | | | | | | | |
| | (a) one only (b) two only | | two or more | | all of these | | | | | |
| 9. | Which is the correct statement for con | nposi | te section ? | | | | | | | |
| | (a) It consists of two or more different materials. | | | | | | | | | |
| | (b) Strain in each material is same. | | | | | | | | | |
| | (c) Load on each material may be different. | | | | | | | | | |
| | (d) all of these. | | | | · | | | | | |
| 10. | The rerm 'modular ratio' is the ratio | of | | | | | | | | |
| | (a) Stress and strain | (b) | Strain and stres | S | | | | | | |
| | (c) Elasticity of two materials. | (d) | Change in lengt | th to | original length | | | | | |
| 11. | The ratio of the elongations of a conic | cal ba | ar under the act | ion o | f its own weigh | | | | | |
| | and that of a prismatic bar of the san | | | | | | | | | |
| | 1 1 | | 1 | / 45 | 1 | | | | | |
| | (a) $\frac{1}{2}$ (b) $\frac{1}{3}$ | (c) | 6 | (d) | 8 | | | | | |
| | | | | | | | | | | |

| Str | engtl | of Materials | | | | | | • | 103 |
|-----|-------|-----------------------------|--------|---------------------------------|----------|--------------------|------------|-----------------------------|------------|
| 12. | For | a uniformly | taper | ing circular ba | ar of l | ength l_1 , larg | er diam | eter d ₁ , sma | ıller |
| | dia | meter d ₂ , elon | gatio | n is equal to | | | | | |
| | | P1 | | PI | | 4P <i>l</i> | | Pl | |
| | (a) | $\frac{Pl}{AE}$ | (b) | $\frac{Pl}{A_1 A_2 E}$ | (c) | $\pi E d_1 d_2$ | (d) | $\frac{Pl}{4\pi E d_1 d_2}$ | |
| 13. | A n | nild steel bar | under | tension shows | prope | rty of | • | | |
| | | Malleability | | (b) ductility | | • | y | | |
| 14. | The | ability of a r | nater: | ial to resist def | | | - | • | |
| | (a) | toughness . | | | | Strength | 4 1 | brittleness | |
| 15. | The | ability of ma | terial | to deform wit | hout b | reaking is cal | | | |
| | (a) | creep | (b) | plasticity | | resilience | 4 | elasticity | |
| 16. | The | modulus of e | lastic | ity of mild stee | el is ap | proximately e | qual to | | |
| | (a) | 110 GPa | (b) | 80 GPa | (c) | 210 GPa | (d) | 10 GPa | • |
| 17. | 1 M | Pa = | N/mn | n^2 | | | | - | |
| | (a) | 1 | (b) | 10 | (c) | 10 ³ | (d) | 106 | • |
| 18. | The | steel used for | r cutt | ing tools is | | | | | |
| | (a) | Mild steel | | | (b) | Medium carb | on steel | | |
| | (c) | High carbon | steel | | (d) | None of thes | e | : 1 | |
| 19. | Whi | ich of the foll | owing | is a proper se | quence | ? | | | |
| | (a) | Elastic limit, | propo | rtional limit, yi | elding, | failure | | 4 | 4 |
| | (b) | Proportional 1 | limit, | elastic limit, yie | elding, | failure 💮 💮 | | | |
| | (c) | Yielding, pro | portio | <mark>nal</mark> limit, elastic | : limit, | failure | | | |
| | (d) | None of these | • | | | | | | |
| 20. | If a | material has i | dentic | al elastic prope | rties in | all directions | , it is sa | id to be (GA' | TE) |
| | (a) | homogeneous | (b) | elastic | (c) | isotropic | (d) | Visco elastic | 3 |
| 21. | | | | he properties o | f a ma | terial when | subjecte | d to repeate | edly |
| | | lied stress is t | erme | d as | | | | ** | . . |
| | | isotropic | ` . | creep | (c) | elasticity | (d) | fatigue | |
| 22. | 1.0 | I. Units, the | | | | _ | | | |
| | (a) | N/mm ² | (b) | N/cm ² | (c) | kg/cm ² | (d) | kg/mm ² | |

23. The unit of strain is (a) N-mm

(a) Shear stress

(a) Stress

(b)

(b)

(b)

25. The deformation per unit length is called

N/mm

Strain

tensile stress

24. Internal resistance offered per unit area by the body against force is called

(c) mm

(c) resistance

(c) Compressive stress

(d) no unit

(d) friction

(d) Strain

ultimate strength

(b) creep (c)

(a) endurance limit

(d) residual stress

| Strength of Materials |
|-----------------------|
|-----------------------|

105

| | 103 |
|-----|--|
| 37. | Ductile fracture is defined as one for which the plastic deformation before fracture |
| | (IES) |
| | (a) is smaller than the elastic deformation |
| | (b) vanishes |
| | (c) is equal to the elastic deformation |
| | (d) is much larger than the elastic deformation |
| 38. | A test specimen is stressed slightly beyond the yield point is then unloaded. Its |
| | yield strength will (GATE) |
| | (a) decrease (b) increase |
| | (c) remains same (d) becomes equal to ultimate tensile strength |
| 39. | |
| | can yield (total) by 0.01 cm. If temperature of the bar is raised by 100°C, then |
| | stress induced in the bar for $\alpha_c = 2 \times 10^{-6}$ and $E_c = 1 \times 10^6$ kg/cm ² . (IES) |
| | (a) $2 \times 10^2 \text{ kg/cm}^2$ (b) $4 \times 10^2 \text{ kg/cm}^2$ |
| | (c) $8 \times 10^2 \text{ kg/cm}^2$ (d) $16 \times 10^2 \text{ kg/cm}^2$ |
| 40. | When a bar fixed at ends is cooled to -10°C, it will develop |
| | (a) no stress (b) shear stress (c) tensile stress (d) Compressive stress |
| 41. | If the ends of a body yield, the magnitude of thermal stress will |
| | (a) Increase (b) decrease (c) remain the same (d) none of these |
| 42. | |
| | (a) tensile stress will be produced |
| | (b) compressive stress will be produced. |
| | (c) no stress will be produced. |
| 43. | When the temprature of a body having rigid ends is increased, the stress induced |
| | will be |
| | (a) Tensile (b) Compressive (c) both (a) and (b) (d) neither (a) nor (b) |
| 44. | If a composite bar is cooled, then the nature of stress in the part with higher |
| | coefficient of thermal expansion will be, |
| | (a) tensile (b) zero (c) compressive (d) none of these |
| 45. | A composite bar made up of steel and copper is subjected to rise in temperature, |
| | $\alpha_{\rm s} < \alpha_{\rm c}$. The stress induced in copper will be |
| | (a) Tensile (b) compressive (c) Zero (d) None of these |
| 46. | A steel rail is 15 m long and is laid at a temperature of 20°C. The maximum |
| | temperature in summer expected is 45°C. The minimum gap required between |
| | two rails is $\alpha = 12 \times 10^{-6}$ /°C. |
| | (a) 4.5 mm (b) 9.0 mm (c) 2.25 mm (d) no gap required. |
| | () 5-p roduitou. |

| 4 | ^ | c |
|---|---|---|
| ı | | п |

| 47. | The maximum thermal stress in a circular tapering section is | | | | | | | |
|-----|---|---|--|--|--|--|--|--|
| | (a) directly proportional to the smaller diameter. | | | | | | | |
| | (b) directly proportional to the bigger diameter. | | | | | | | |
| | (c) inversely proportional to the bigger diameter. | | | | | | | |
| | (d) both 'a' and 'c'. | | | | | | | |
| 48. | For a linearly elastic, isotropic and hon | ogeneous material, the number of elastic | | | | | | |
| -: | constants required to relate stress and | strain is (Civil Services) | | | | | | |
| | (a) two (b) three | (c) four (d) six | | | | | | |
| 49. | Which one of the following favours brit | tle fracture in a ductile material? | | | | | | |
| | (a) elevated temperature(b) | Slow rate of straining | | | | | | |
| | (c) Presence of notch | (d) Circular cross-section | | | | | | |
| 50. | The value of Poisson's ratio for any ma | terial can not exceed (IES) | | | | | | |
| | (a) 2.0 (b) 1.414 | (c) 1.0 (d) 0.5 | | | | | | |
| 51. | The value of Poisson's ratio for steel va | ries from | | | | | | |
| | (a) 0.20 to 0.25 (b) 0.25 to 0.35 | (c) 0.35 to 0.40 (d) 0.40 to 0.55 | | | | | | |
| 52. | Which relation does not hold good? | (GTU, April 2010) | | | | | | |
| | (a) $\varepsilon_{\rm v} = \varepsilon (1 - 2\mu)$ | (b) $E = 3 K (1 - 2 \mu)$ | | | | | | |
| | G+3K | | | | | | | |
| | (c) $E = \frac{G + 3K}{9GK}$ | (d) $E = 2 G (1 + \mu)$ | | | | | | |
| 53. | The ratio of lateral strain to the linear | strain is called | | | | | | |
| | (a) Modulus of elasticity | (b) Bulk modulus | | | | | | |
| | (c) Poisson's ratio | (d) Modulus of rigidity | | | | | | |
| 54. | Volumetric strain of a rectangular bod | y subjected to an axial force is, | | | | | | |
| | (a) $\varepsilon \left(1 - \frac{2}{m}\right)$ (b) $\varepsilon \left(1 + \frac{2}{m}\right)$ | (c) $\varepsilon \left(2 - \frac{1}{m}\right)$ (d) $\varepsilon \left(2 + \frac{1}{m}\right)$ | | | | | | |
| 55 | | nutually perpendicular stresses of equal | | | | | | |
| - | intensity, the ratio of direct stress to v | | | | | | | |
| | (a) Young's modulus | (b) Shear modulus | | | | | | |
| | (c) Poisson's ratio | (d) Bulk modulus | | | | | | |
| 56. | | forces acting tangentially to the surface of | | | | | | |
| | a body, the stress produced is called | | | | | | | |
| • | (a) Tensile stress (b) shear stress | (c) Compressive stress (d) direct stress. | | | | | | |
| 57. | Lateral strains are longitudinal strains | • | | | | | | |
| | (a) always less than (b) sometimes l | | | | | | | |
| 58. | Poisson's ratio is ratio of | | | | | | | |
| | (a) longitudinal to lateral strain. | (b) lateral to longitudinal strain. | | | | | | |
| | (c) shear stress to shear strain. | | | | | | | |
| | , | | | | | | | |

| 5 9. | Ma | ximum p | ossible valu | e of Pe | nisson's r | atio : | ie . | | | - | 107 |
|-----------------|------|-------------------------------|---|----------------------|--------------------------|---------------|----------------------------|----------------------|-----------------|---------------|------------|
| | | 0.5 | (b) | 0.75 | | (c) | | • | - | | |
| 60. | | | rial has the | | st Poisson | ٠, | | | | | |
| | (a) | | (b) | Rubbe | | | Steel | | (d) | Coppe | r · |
| 61. | Poi | isson's ra | tio for mos | t of the | materia | ٠, | | 0 | () | COPPO | - |
| | (a) | 0.5 | (b) | 0.25 | | (c) | 0.33 | | (d) | 0.20 | |
| 62. | Th | e ratio of | f Young's m | odulus | of elasti | city | to mod | lulus of | rigidit | y for a | material |
| • | | | son's ratio (| | | | | | Ū | | Services) |
| | (a) | $\frac{12}{5}$ | (b) | 5 | • | (-) | 5 | (d) | 14 | | - |
| | | , | | 12 | | | 1.1 | | J | | ٠, |
| 63. | The | e relation | between Y | oung's | modulus | (E), i | modul | us of rig | idity (| G) and | Poisson's |
| | rat | io $\left(\frac{1}{m}\right)$ | is given by | | | | | (Civil S | Service | s) (GAT | ΓE 2007) |
| | (a) | E = 2G | $(1-\frac{1}{m})$ | | | (b) | E = 2 | 2G (1 + n | n) | | |
| | (c) | $E = \frac{1}{2}$ | G (1 + m) | | | (d) | E = 2 | G (1 + | $\frac{1}{m}$) | : | |
| 64. | The | rigidity | modulus of | a mate | erial who | se E | = 2 × | 10 ⁶ kg/c | m² and | d Poisso | n's ratio |
| | is 0 | .25 will l | be | | | | | | 0 | | (IES) |
| | | | 06 kg/cm ² | | | | | 10 ⁶ kg/c | | | |
| | | 0.5×10^{-1} | | | | | | 06 kg/cm | | | |
| 65. | If a | material | has moduli | is of el | asticity of | f 2.1 | \times 10 ⁶] | kgf/cm ² | and m | odulus d | of rigidly |
| | of (| 0.8×10^6 | kgf/cm ² the | n appro | o <mark>xi</mark> mate v | alue | of the | Poisson | 's ratio | of the | material |
| | | ıld be | | | | | | | | | (IES) |
| ., | ` ′ | 0.26 | | 0.31 | .nati | • • | 0.47 | l | ` ' | 0.5 | |
| 66. | Ab | ar 4 cm | in diameter | is sub | jected to | an a | xial lo | ad of 4t | . The | extensio | on of the |
| | | | auge length | | cm is 0.0 | 3 cm | . The | decrease | in di | ameter | |
| | | | sson's ratio | | | | | | | | (IES) |
| 67 | | 0.25 | ` ' | 0.30 | | (c) | | _ | ` ' | 0.35 | |
| 67. | to = | en inat i Jone etve | or a elemen | t in a | poay of h | iomo | geneou | ıs isotro | pic ma | iterial s | ubjected |
| | ro b | nane stres | ss; ε _x , ε _y and | ı e _z are | e normal s | strair | ıs in x, | y and z | direct | ions res | pectively |

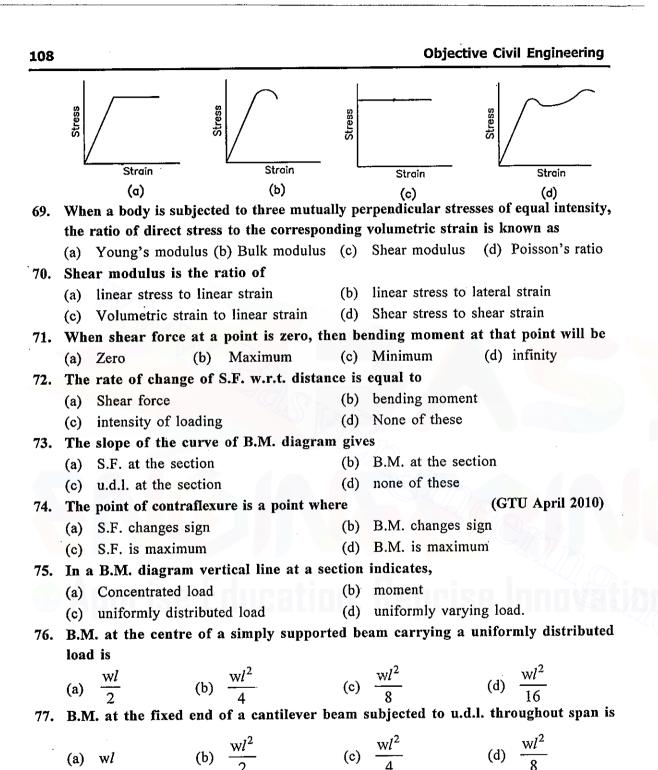
and μ is Poisson's ratio, then magnitude of unit volume change of the element is

 $(a) \quad \epsilon_x + \epsilon_y + \epsilon_z \quad (b) \quad \epsilon_x - \mu \; (\epsilon_y + \epsilon_z) \quad (c) \; \; \mu \; (\epsilon_x + \epsilon_y + \epsilon_z) \; \; (d) \frac{1}{\epsilon_x} + \frac{1}{\epsilon_y} + \frac{1}{\epsilon_z}$

The stress-strain curve for an ideally plastic material is

given by

(IES)



78. B.M. at the free end of a cantilever beam subjected to any type of load is

minimum

(b)

(c) maximum

(a) wl

(a) Zero

(d) equal to load.

| Str | ength of Materials | | | 109 |
|-------------|---|--------|---------------------------------|------------------|
| 79. | 8 | ed su | pport. | |
| | (a) always maximum | | always zero. | |
| 80. | The shape of shear force diagram for end is | cantil | lever beam subjected to coup | le at fre |
| | (a) horizontal straight line | (b) | zero | |
| | (c) parabola | (d) | inclined straight line | |
| 31. | The maximum B.M. of a cantilever be | am li | | |
| | (a) the free end | (b) | the fixed end | |
| | (c) Middle of its length | (d) | $\frac{1}{4}$ from fixed end | |
| 82. | The B.M. diagram for a cantilever beend will be | eam s | subjected to bending momen | t at free |
| | (a) a triangle (b) a rectangle | (c) | a parabola (d) a cubic para | abola |
| 33. | The B.M. diagram for a cantilever bean | n carr | ying uniformly distributed loa | d is |
| | (a) a triangle (b) a parabola | (c) | a cubic parabola (d) a rectar | ngle |
| 84. | The number of independent elastic co | onstar | nts for a linear elastic isotro | pic and |
| | homogeneous material is | | | E 2010) |
| | (a) 4 (b) 3 | (c) | 2 (d) 1 | |
|) J. | Shear force along the beam will be | | | (IES) |
| | | (a) | uniformly varying | |
| | 3 _A | (b) | Uniform | |
| | В | (c) | Zero | |
| | P | (d) | concentrate at A and B only | |
| 36. | FIG. Q-85 Shear force diagram (SFD) and bendingure. | ng me | oment diagram (BMD) are s | hown in (IES) |
| | SFD | | · | |
| | | | | |
| | -Ve | | 1 t.m | |
| | BMD | - | | |
| | The corresponding loads | ra dia | gram would be | |
| | A | | B 1 t.m | |
| ٠ | (a) | | | |
| | Į | | | |
| | • | | | |

Objective Civil Engineering

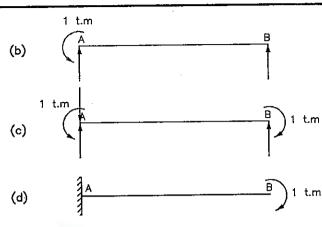


FIG. Q-86

87. Two people weighing W each are sitting on plank of length L floating on water at L/4 from either end. Neglecting the weight of the plank, the bending moment at the centre of the plank is (GATE 2010)

(a) $\frac{WL}{R}$

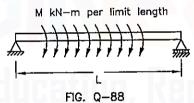
110

(b) $\frac{\text{WL}}{16}$

(c) $\frac{WL}{32}$

(d) Zero

88. For a simply supported beam of length L, subjected to a uniformly distributed moment M kN-m per unit length as shown in figure, the bending moment (in kN - m) at the mid span of the beam is (Gate 2010)



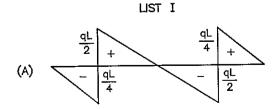
(a) Zero

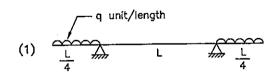
(b) M

(c) ML

(d) M/L

89. Match List I (shear force diagrams) beams with List II (diagrams of beams with supports and loading) and select the correct answer by using the codes given below the lists. (Gate 2009)

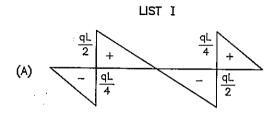


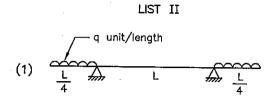


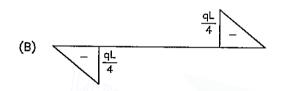
LIST II

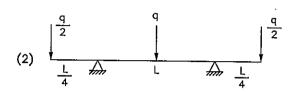
Strength of Materials

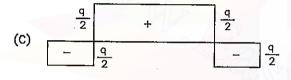
111

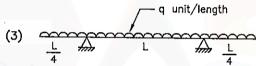


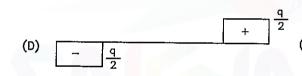














Codes:

FIG. Q-89

- A В C D
- (a) 3 2
- (b) 3 2
- 2 3 (c) (d) 2 4 3 1
- 90. A simply supported beam AB has the bending moment diagram as shown in the following figure. (GATE 2006)

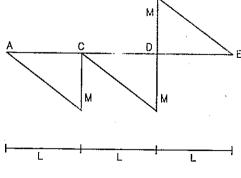


FIG. Q-90

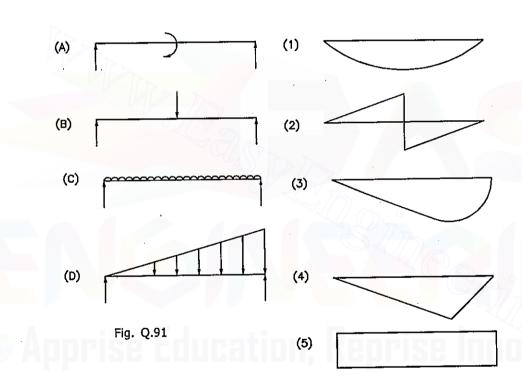
Objective Civil Engineering

- (a) Couple of M at C and 2M at D (b) Couple of 2M at C and M at D
- (c) Concentrated loads $\frac{M}{L}$ at C and $\frac{2M}{L}$ at D
- (d) Concentrated loads $\frac{M}{L}$ at C and couple of 2M at D
- 91. List I shows different loads acting on 4 beam and list II shows different bending moment diagrams. Match the load with the corresponding moment diagram.

(GATE 2003)

LIST I

LIST II



Codes:

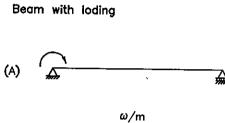
112

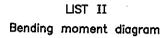
- A B C D
- (a) 4 2 1 3
- (b) 2 5 3 1
- (c) 5 4 1 3
- (d) 2 4 1 3
- 92. Match List I with List II and select the correct answer using the codes given below the lists:

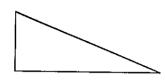
Strength of Materials

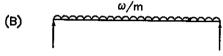
LIST I

113

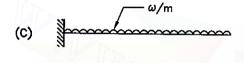




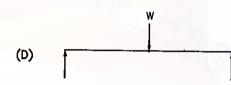














Codes:

A В D (a) 3 4 2 (b) 2 4 (c) 3 2 (d) 2 3

FIG. Q-92

(1)

93. Figure shows a stress-strain diagram A, B and C represents

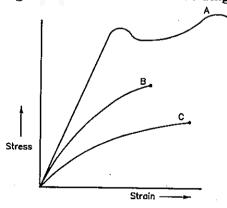


FIG. Q-93

- (a) Mild steel, soft brass, glass
- (b) Mild steel, glass, soft brass
- (c) Soft brass, mild steel, glass
- (d) Soft brass, glass, mild steel

Objective Civil Eng. \ 2016 \ 15

114

94. A beam extending beyond the support is called

- (a) Simply supported beam
- (b) Fixed beam

(c) Overhanging beam

(d) Cantilever beam

95. For a beam shown in fig., shear force at B is

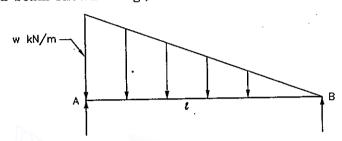


FIG. Q-95

(a)
$$\frac{wl}{6}$$

(b)
$$\frac{wl}{3}$$

(d)
$$\frac{2wl}{3}$$

96. For a beam shown in Q. 95, maximum B.M. occur at a distance from end B.

(a)
$$\frac{l}{2}$$

(b)
$$\frac{1}{3}$$

(c)
$$\frac{l}{\sqrt{2}}$$

(d)
$$\frac{l}{\sqrt{3}}$$

97. For a beam shown in Q. 95, maximum B.M. is

(a)
$$\frac{wl^2}{3\sqrt{3}}$$

(b)
$$\frac{wl^2}{6\sqrt{3}}$$

(c)
$$\frac{wl^2}{9\sqrt{3}}$$

$$(d) \quad \frac{wl^2}{12\sqrt{3}}$$

98. For a simply supported beam of span *l* carrying central point load W, maximum deflection is

(a)
$$\frac{Wl^4}{8EI}$$

(b)
$$\frac{Wl^3}{6EI}$$

(c)
$$\frac{Wl^3}{48EI}$$

(d)
$$\frac{5}{384} \frac{Wl^4}{EI}$$

99. A cantilever beam of span l, carrying u.d.l. on entire span, slope at free end will be

(a)
$$\frac{Wl^2}{2EI}$$

(b)
$$\frac{Wl^3}{3EI}$$

(c)
$$\frac{Wl^3}{6EI}$$

(d)
$$\frac{Wl^4}{8EI}$$

100. For a beam shown in fig. maximum deflection is

(GATE, IES)

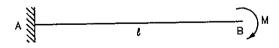


FIG. Q-100

(a)
$$\frac{ML}{EI}$$

(b)
$$\frac{\text{ML}^2}{2\text{EI}}$$

(c)
$$\frac{ML^3}{3EI}$$

(d)
$$\frac{\text{ML}^3}{6\text{EI}}$$





101. For a beam shown in Q. 100, maximum slope at B is

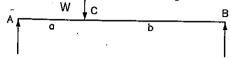


(b)
$$\frac{\text{ML}^2}{2\text{EI}}$$

(c)
$$\frac{\text{ML}^3}{3\text{EI}}$$

(d)
$$\frac{\text{ML}^3}{6\text{EI}}$$

102. For a beam shown below, deflection below point load is



(a)
$$\frac{Wl^3}{48EI}$$

(b)
$$\frac{Wa^2b^2}{3FU}$$

(c)
$$\frac{5}{384} \frac{Wl^2}{EI}$$

(d)
$$\frac{Wl^3}{24EI}$$

103. The maximum deflection of a fixed beam of length l carrying a central point load W is

(a)
$$\frac{Wl^3}{48EI}$$

(b)

(c)
$$\frac{Wl^3}{192EI}$$

(d)
$$\frac{Wl^3}{384EI}$$

104. The maximum deflection of a fixed beam of length / carrying an u.d.l. w kN/m - over whole length is

(a)
$$\frac{Wl^4}{48EI}$$

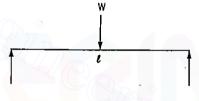
(a) $\frac{Wl^4}{48EI}$ (b) $\frac{Wl^4}{96EI}$

(c) $\frac{Wl^4}{192EI}$

105. Two beams 'A' and 'B' carrying a central point load W are shown below. The deflection of beam A will be times the deflection of B.



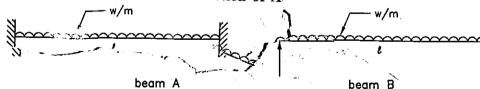
beam A



beam B

FIG. Q-105

106. Two beams 'A' and 'B' carrying u.d.l. are shown below. The maxi. deflection of B will be times the maxi. deflection of A



(a) 2

(b)

FIG. Q-106 (c)

(d) 5

116

| 107 | A f | neutral | axis | bending | stress | is |
|-----|-----|---------|------|---------|--------|----|
| | | | | | | |

- (a) Minimum
- maximum (b)
- (c) zero
- (d) infinity

108. The purpose of flitched beam is to improve

- (a) shear force over the section
- moment of resistance over the section. (b)
- (c) appearance of the section
- all of these. (d)

109. When a cantilever beam is loaded at its free end, maximum compressive stress shall develop at

- (a) bottom fibre
- top fibre (b)
- (c) neutral axis (d) centre of gravity.

110. In the theory of simple bending, the bending stress in the beam section varies

- (a) linearly
- parabolically (b)
- elliptically (c)
- (d) none of these

111. A beam of uniform strength has constant

Shear force

- bending moment (b)
- (c) Cross sectional area
- (d) deflection.

112. The section modulus of a circular section of diameter (d) is

- (a) $\frac{\pi}{32}$ (d)² (b) $\frac{\pi}{32}$ (d)³ (c) $\frac{\pi}{64}$ (d)³
- (d) $\frac{\pi}{64}$ (d)⁴

113. The stepped cantilever is subjected to moments M as shown in figure below. The vertical deflection at the free end (neglecting the self weight) is

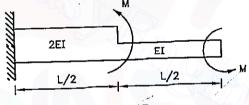
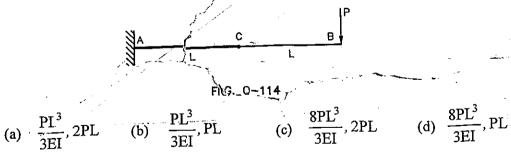


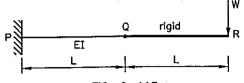
FIG. 0-113

- (d) Zero

114. Consider beam AB shown in the figure. Part AC of the beam is rigid while part CB has the flexural rigidity EI. Identify the correct combination of deflection at (GATE 2006) end B and bending moment at end A, respectively.



115. In the cantilever beam PQR shown below the segment PQ has flexural rigidity EI and the segment QR has infinite flexural rigidity. The deflection at Q is (GATE 2009)



(a) $\frac{5\text{WL}^3}{6\text{EI}}$

FIG. Q-115 (c) $\frac{\text{WL}^3}{2\text{EI}}$

116. The product EI is known as

(a) Polar moment

(b) Flexural rigidity

(c) Stiffness

(d) Modulus of rigidity

117. When a rectangular simply supported beam is loaded transversely the maximum compressive stress is developed on the

(a) top layer

(b) bottom layer

(c) neutral axis

(d) every cross section

118. The neutral axis of a beam is subjected to stress.

(a) maximum tensile

(b) minimum tensile

(c) maximum compressive

(d) zero

119. For a beam subjected to pure bending

all fibres experiences uniform stress.

The nature of stress in all fibres is the same.

The stress intensity in any fibre is proportional to the distance of the fibre from N.A.

The stress intensity in any fibre is proportional to the distance of the fibre from supports.

120. Two beam sections, one is circular and other square made from same materials and having same cross-sectional area, when subjected to bending

(a) Moment of resistance for both beams is same.

(b) Circular section has higher M.R.

Square section has higher M.R.

(d) None of these

121. If M is the bending moment, f is bending stress and Z is section modulus, then relation between them is

(a) f = M.Z

(b)

(c) $f = \frac{Z}{M}$ (d) $Z = \frac{f}{M}$

122. A beam of uniform strength can be obtained by

(a) Keeping the width uniform and varying the depth.

(b) Keeping the depth uniform and varying the width.

Varying the width and depth both.

(d) any one of the above.

(GATE) 123. Castellated beams are used for

- (a) Light construction
- (b) Resisting bending moment only
- (c) Loads not passing through the shear centre
- (d) Sections subjected to alternate compressive and shear stress
- 124. The shear in a beam subjected to pure positive bending is
- (c) negative
- (d) indeterminate

125. The ratio of flexural strength of two beams of square cross-section, the first beam being placed with its top and bottom sides horizontally and the second beam being placed with one diagonal horizontally is,

- (b) 3
- (c) $\frac{1}{\sqrt{5}}$

126. A propped cantilever beam of span 'l' carries u.d.l. of 'w' per unit run over its entire span. The value of prop reaction to keep the beam horizontal is (IES) (GATE)

- (b) $\frac{3}{9}$ wl
- (c) $\frac{wl}{2}$
- (d) $\frac{5}{9}$ wl

127. In a triangular section, the maximum shear stress occurs at,

- (a) apex of the triangle
- (b) mid of the height

(c) $\frac{1}{3}$ of the height

base of the triangle.

128. For a rectangular section the ratio of maximum shear stress to the average shear (d) 1.75 stress is

- (c) 1.25

129. For a circular section the ratio of maximum shear stress to the average shear stress is

- (a) 1.13
- 1.23 (b)
- 1.33 (c)
- (d) 1.43

130. For any section shear stress at the top edge is

- (c) Zero

131. In case of inverted T - section subjected to shear force F, The maximum shear stress will occur at

(a) top of section

- Neutral axis
- (c) Junction of flange and web
- (d) bottom of section.

132. The bending equation is

The bending equation is

(a)
$$\frac{M}{I} = \frac{\sigma}{y} = \frac{E}{R}$$
 (b) $\frac{M}{y} = \frac{\sigma}{I} = \frac{E}{R}$ (c) $\frac{T}{J} = \frac{\tau}{R} = \frac{C\theta}{I}$ (d) $\frac{T}{R} = \frac{\tau}{J} = \frac{C\theta}{I}$

133. The point of contraflexure occurs in

(a) Cantilever beams

(b) Simply supported beams

(c) Overhanging beams

(d) Fixed beams

(a) $\frac{1}{750}$

(b) $\frac{1}{1600}$

| 134. The basic assumption of plane section normal to the neutral axis, before bending, | | | | | | | | |
|--|--|--|--|--|--|--|--|--|
| remaining plane and normal to the axis after bending, leads to | | | | | | | | |
| (a) uniform strain over the beam cross-section. | | | | | | | | |
| (b) Uniform stress over the cross-section. | | | | | | | | |
| (c) linearly varying strain over the cross-section. | | | | | | | | |
| (d) stress which are proportional to strains at all cross-sections. | | | | | | | | |
| 135. The load at which column just starts buckling is called | | | | | | | | |
| (a) crippling load (b) buckling load (c) critical load (d) any one of these | | | | | | | | |
| 136. For long columns, the value of buckling load is crushing load. | | | | | | | | |
| (a) equal to (b) less than (c) more than | | | | | | | | |
| 137. A column that fails due to direct stress is called | | | | | | | | |
| (a) Short column (b) long column (c) Weak column (d) Slender column | | | | | | | | |
| 138. Euler's equation for crippling load when both ends of column are hinged is | | | | | | | | |
| $\pi^2 \mathrm{EI}$ $4\pi^2 \mathrm{EI}$ $\pi^2 \mathrm{EI}$ $2\pi^2 \mathrm{EI}$ | | | | | | | | |
| (a) $\frac{\pi^2 \text{EI}}{l^2}$ (b) $\frac{4\pi^2 \text{EI}}{l^2}$ (c) $\frac{\pi^2 \text{EI}}{4l^2}$ (d) $\frac{2\pi^2 \text{EI}}{l^2}$ | | | | | | | | |
| 139. For a long column with one end fixed and other end hinged, effective length is | | | | | | | | |
| L | | | | | | | | |
| 2 | | | | | | | | |
| 140. For a long column with both ends fixed, effective length is | | | | | | | | |
| (a) L (b) $\frac{L}{2}$ (c) $\frac{L}{\sqrt{2}}$ (d) 2L | | | | | | | | |
| . ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ | | | | | | | | |
| 141. The buckling load for a given column depends upon | | | | | | | | |
| (a) cross sectional area of column | | | | | | | | |
| (b) length and least radius of gyration | | | | | | | | |
| (c) modulus of elasticity of column material | | | | | | | | |
| (d) all of the above | | | | | | | | |
| 142. Euler's formula is not valid, for mild steel column when slenderness ratio is | | | | | | | | |
| (a) more than 80 (b) less than 80 (c) more than 120 (d) more than 30 | | | | | | | | |
| 143. An electric pole is 5 m high from the ground level. Its effective length for design | | | | | | | | |
| purpose will be | | | | | | | | |
| (a) 2.5 m (b) 3.53 m (c) 5.0 m (d) 10 m | | | | | | | | |
| 144. If slenderness ratio for a column is 100, then it is said to be | | | | | | | | |
| (a) Short column (b) long column (c) medium column | | | | | | | | |
| 5. The Rankine constant for a mild steel column is | | | | | | | | |

| 146. For the | case of a | slender | column | of length | l and | flexural | rigidity | E |
|--------------|-------------|------------|-----------|---------------|-------|----------|----------|-----|
| built-in | at base and | free at to | p, Euler' | s critical lo | ad is | | (GA | TE) |

- (a) $\frac{4\pi^2 \text{ EI}}{I^2}$ (b) $\frac{2\pi^2 \text{ EI}}{I^2}$ (c) $\frac{\pi^2 \text{ EI}}{I^2}$ (d) $\frac{\pi^2 \text{ EI}}{AI^2}$

147. When a column is fixed at both ends then corresponding Euler's critical load is

- (a) $\frac{\pi^2 \text{ EI}}{l^2}$ (b) $\frac{2\pi^2 \text{ EI}}{l^2}$ (c) $\frac{3\pi^2 \text{ EI}}{l^2}$ (d) $\frac{4\pi^2 \text{ EI}}{l^2}$

148. If $P_{\rm E}$ is crippling load given by Euler, $P_{\rm C}$ is load at failure due to direct compression, PR is load in accordance with the Rankine's criterion of failure, (Civil Services) than P_R is given by

- (a) $\frac{\left(P_E + P_c\right)}{2}$ (b) $\sqrt{P_E \times P_c}$ (c) $\frac{P_c \cdot P_E}{P_c + P_F}$
- (d) None of these

149. The buckling load will be maximum for a column, if

(IES)

- (a) one end of column is clamped and other free.
- (b) Both ends of the column are clamped.
- (c) Both ends of the column are hinged.
- (d) One end of the column is hinged and other clamped.

150. For a circular column having its ends hinged the slenderness ratio is 160. The \(^{1}\)_d (IES) ratio of the column is

- (b)
- (c) 40
- (d) 20

151. The shear stress distribution over a rectangular cross-section of a beam follows (IES)

- (a) a straight line path
- (b) a circular path

(c) a parabolic path

an elliptical path

152. The buckling load P = P_{cr} for the column AB in figure, as k_T approaches infinity,

becomes $\alpha \frac{\pi^2 EI}{\tau^2}$, where α is equal to

(GATE 2006)

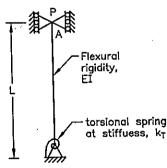


FIG. Q-152

0.25 (a)

(b) 1.0

2.05 (c)

(d) 4.0

| 153. | The effective length of column of leng | th L, fixed against | |
|------|---|------------------------------------|--|
| | at one end is | | (GATE 2010) |
| | (a) 0.5 L (b) 0.7 L | (c) 1.414 L | (d) 2 L |
| 154. | A steel column pinned at both ends, ha | | |
| | is restrained against lateral movemen | t at its mid – heigh | |
| | be | () 400 137 | (GATE 2007) |
| | | (c) 400 kN | (d) 800 kN |
| 155. | On a principle plane the magnitude of | | |
| | (a) Maximum (b) Minimum | (c) Zero | (d) infinity. |
| 156. | . When a body is subjected to direct | | |
| | directions, tangential stress across inc | | |
| | (a) $\theta = 0^{\circ}$ (b) $\theta = 45^{\circ}$ | - · | (d) $\theta = 180^{\circ}$ |
| 157. | . When a body is subjected to two te | | |
| | mutually perpendicular planes, the ra | | |
| | (a) Zero (b) maximum | • • | (d) infinity |
| 158. | . The angle between major principal pla | | |
| | (a) greater than 90° (b) Less than 90° | (c) equal to 90° | (d) equal to 180° |
| 159. | . The angle of obliquity is equal to | | |
| | (a) $\tan^{-1} (\sigma_n - \sigma_t)$ (b) $\tan^{-1} (\sigma_n/\sigma_t)$ | (c) $tan^{-1} (\sigma_t/\sigma_n)$ | $) \qquad (d) \tan^{-1} \left(\sigma_t - \sigma_n \right)$ |
| 160. | . When a body is subjected to a direct | | |
| | normal stress on an oblique section of | | |
| | of the section is equal to. | | |
| | (a) $\sigma \sin \theta$ (b) $\sigma \cos \theta$ | (c) $\sigma \sin^2 \theta$ | (d) $\sigma \cos^2 \theta$ |
| 161. | . The direct stress across a principal p | lane is known as p | rincipal stress. |
| | (a) True (b) False | | |
| 162 | . When a body is subjected to a direct to | ensile stress (v) in (| one plane, then maximum |
| | normal stress occurs at a section incl | | |
| | (a) 0° (b) 30° | (c) 45° | (d) 90° |
| 163 | . The maximum shear stress is equal to | the radius of Mol | hr's circle. |
| | (a) True (b) False | | • |
| 164 | A steel wire of 20 mm diameter is ber | it into a circular sh | ane of 10 m radius. If E. |
| 107 | the modulus of elasticity is 2×10^6 k | | |
| | wire is | . , | (IES) |
| | (a) 10 ³ kg/cm ² | (b) $2 \times 10^3 \text{ kg/}$ | cm ² |
| | (c) $4 \times 10^3 \text{ kg/cm}^2$ | (d) $6 \times 10^3 \text{ kg/}$ | |
| | (c) The residual | (a) 0 · 10 kg/ | |
| Obie | ective Civil Eng. \ 2016 \ 16 | | |

| 165. Match list - I and List - II and se | lect correct answer using the codes given below |
|--|---|
| the lists: | (IES) |

List - I

- A. Moment of inertia
- B. Elongation
- C. Neutral axis
- D. Top fibre

List II

- 1. Tensile stress
- 2. Modulus of rupture
- 3. Zero shear stress
- 4. Zero longitudinal stress

Codes:

A B C D 2 1 3 4

- (a) 2 1 3 4 (b) 1 2 4 3
- (c) 3 4 1 2
- (d) 2 1 4 3

166. The shear centre of a section is defined as that point

- (a) through which the load must be applied to produce zero twisting moment on the section.
- (b) At which shear force is zero.
- (c) at which the shear force in maximum.
- (d) at which the shear force is minimum.

167. The plane of maximum shear stress has normal stress that is

(IES)

- (a) maximum
- (b) minimum
- (c) zero
- (d) none of these

168. Consider the following statements:

In a uni-directional stress-system, the principal plane is defined as one on which the (IES)

- 1. Shear stress is zero
- 2. Normal stress is zero
- 3. Shear stress is maximum
- 4. Normal stress is maximum

of these statements:

- (a) 1 and 2 are correct
- (b) 2 and 3 are correct
- (c) 1 and 4 are correct

(d) 3 and 4 are correct

169. A Mohr's circle reduces to a point when the body is subjected to

(IES)

- (a) Pure shear.
- (b) Uniaxial stress only.
- (c) Equal and opposite axial stresses on two mutually perpendicular planes, the planes being free of shear.
- (d) Equal axial stresses on two mutually perpendicular planes, the planes being free of shear.

| 170. | When | two | mutually | perpe | ndicular | principal | stresses | are | unequal | but | alike, | then |
|------|-------|------|------------|---------|----------|-----------|----------|-----|---------|-----|--------|------|
| | maxim | um : | shear stre | ss is r | epresent | ed by | | | | | (| IES) |

- (a) diameter of the Mohr's circle
- (b) half the diameter of the Mohr's circle
- (c) One-third the diameter of the Mohr's circle
- (d) One-fourth the diameter of the Mohr's circle

171. At a point in a strained material, if two mutually perpendicular tensile stresses of 2000 kg/cm² and 1000 kg/cm² are acting, then intensity of tangential stress on a plane inclined at 15° to the axis of minor stress will be (IES)

- (a) 125 kg/cm^2
- (b) 250 kg/cm^2
- (c) 500 kg/cm^2
- (d) 1000 kg/cm^2
- 172. An axially loaded bar is subjected to normal stress of 173 MPa. The shear stress in the bar is (GATE 2007)
 - (a) 75 MPa
- (b) 86.5 MPa
- (c) 100 MPa
- (d) 122.3 MPa

173. Mohr's circle for the state of stress defined by $\begin{vmatrix} 30 & 0 \\ 0 & 30 \end{vmatrix}$ MPa is a circle with

- (a) Centre at (0, 0) and radius 30 MPa
- (b) Centre at (0, 0) and radius 60 MPa
- (c) Centre at (30, 0) and radius 30 MPa
- (d) Centre at (30, 0) and radius zero

174. Consider the following statements:

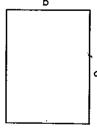
(GATE 2009)

- 1. On a principal plane, only normal stress acts
- 2. On a principal phone, both normal and shear stresses act
- 3. On a principal plane, only shear stress acts
- 4. Isotropic state of stress is independent of frame of reference

Which of the above statements is/are correct?

- (a) 1 and 4
- (b) 2 only
- (c) 2 and 4
- (d) 2 and 3

175. For the section shown, second moment of area about an axis d/4 distance above the bottom of the area is



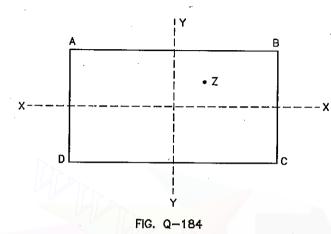
- (a) $\frac{bd^3}{48}$
- (b) $\frac{bd^3}{12}$
- (c) $\frac{7bd^3}{48}$
- (d) $\frac{bd^3}{3}$

| 176. When a column is subjected to eccentr | ic load | l , | |
|---|---------------------|-----------------------------|---------------------------------------|
| (a) Only direct stress is produced | | | · · · · · · · · · · · · · · · · · · · |
| (b) Only bending stress is produced | | _ | |
| (c) Both direct stress and bending stres | s is pro | oduced | |
| (d) None of these | | _ | claw awags spation |
| 177. For no tension condition in the base of | of a sho | ort column (| of circular cross section, |
| the limit of eccentricity is | | | • |
| (a) $\frac{d}{3}$ (b) $\frac{d}{3}$ | ,(c) | <u>d</u> | (d) $\frac{d}{8}$ |
| (a) $\frac{d}{2}$ (b) $\frac{d}{3}$ | .(0) | 4 | 8 |
| 178. For no tension in column | | - | |
| (a) Load must act within elimit | (b) | $\sigma_{o} > \sigma_{b}$ | |
| (c) $e \leq \frac{Z}{A}$ | | any one of | |
| 179. A column having moments of inertia | I _{xx} and | l I _{yy} , will fa | il in the direction of |
| (a) axis of load | (0) | maximum N | 11.1. |
| (c) Minimum M.I. | (d) | Maximum r | adius of gyration |
| 180. The slenderness ratio is the ratio of | | | |
| (a) M.I. to area of column | | | |
| (b) Length of column to least radius of | of gyrat | ion | |
| (c) Least radius of gyration to area of | colum | n o | |
| (d) Least radius of gyration to length | of colu | ımn | |
| 181. A reinforced cement concrete beam i | is consi | idered to be | made of |
| . (a) homogeneous material | (b) | heterogene | ous material |
| (c) composite material | (d) | isotropic m | naterial |
| 182. In engineering materials, the rigidity | modu | lus | |
| (a) is always less than Young's modu | ılus. | | |
| (b) is always higher than Young's mo | odulus. | | |
| (c) is always less than half the value | of Yo | ung's modulu | is. |
| vis visites agreet to Voung's modul | us. | | • |
| too to least column of external diamet | er D | and interna | d diameter D ₂ carries an |
| TALL STATE OF THE PROPERTY OF | MITIME | THE ECCENTURE | city in the common of the city |
| D^2 D^2 $D^2 - D^2$ | | $D_1^2 - D_2^2$ | $D_2^2 - D_1^2$ |
| (a) $\frac{D_1^2 - D_2^2}{8D_2}$ (b) $\frac{D_2^2 - D_1^2}{8D_2}$ | (c) | $\frac{1}{8D_1}$ | |
| 184. A column ABCD of rectangular sec | tion is | subjected to | an eccentic load at 2 3. |
| | | | |
| shown in fig. Under the compressive load, direct | stress | 1S 4UV t/m~ 2 | the benning prices |
| | | | |

 $\sigma_{\rm bx} = \pm 1200 \text{ t/m}^2, \ \sigma_{\rm by} = \pm 800 \text{ t/m}^2$

The stress at corner A will be

(Civil Services)



- (a) 800 t/m² (compressive)
- (b) 1600 t/m² (tensile)
- (c) 800 t/m² (tensile)
- (d) 2000 t/m² (compressive)

185. A simply supported beam of uniform cross section is subjected to maximum bending moment of 2.25 t.m. If it has rectangular cross section, with width 15 cm and depth 30 cm, then maximum bending stress induced in the beam will be

(Civil services)

- (a) 50 kg/cm^2
- 100 kg/cm^2 (b)
- (c) 150 kg/cm^2
- (d) 225 kg/cm^2

186. A beam has a solid circular section having diameter d. If a section of the beam is subjected to a shear force F, then maximum shear stress in the cross section is given by (Civil Services)

- (b) $\frac{16F}{3\pi d^2}$
- (c) $\frac{8F}{3\pi d^2}$

187. The stress produced by sudden load is times that is produced by gradual load.

- (a) $\frac{1}{2}$
- (b) 1
- (c) 2
- (d) 4

188. Strain energy is the

- (a) Energy stored in a body when strained within elastic limit.
- (b) Maximum strain energy which can be stored in a body.
- (c) Energy stored in a body when strained up to the breaking of a specimen.
- (d) Proof resilience per unit volume.

189. The strain energy stored in a body when load is applied gradually is

- (b) $\frac{\sigma^2}{2F} \times V$ (c) $\frac{\sigma^2 E}{2V}$ (d) $\frac{\sigma^2}{2EV}$

| 190. | | | | ed in a body, w ne load is applie | | uddenly loaded, dually. | is | the strain | |
|---|---|---------------------------------|---------------------|--------------------------------------|---------|----------------------------------|--------|--------------------|--|
| | | | | one-half | | twice | (d) | four times | |
| 191. | ` ' | - | | d in a body due | | | (-) | | |
| | | | | | | | | 2C | |
| | | - 20 | | • • | | $\frac{\tau^2}{4C} \times V$ | | | |
| 192. | _ | | | | | a prismatic bar | | | |
| | | | | | in ene | ergy u stored in | the s | | |
| | com | bined action o | of P ₁ a | and P ₂ will be | | | | (GATE 2007) | |
| | | | | | | $u < u_1 + u_2$ | | | |
| 193. | A cy | ylindrical pres | sure v | vessel is said to | be th | in walled if 't' i | s giv | en by: | |
| | (a) | $t = \frac{d}{10}$ | (b) | $t < \frac{d}{10}$ | (c) | $t > \frac{d}{10}$ | | | |
| 194. | For | a thin cylind | lrical | shell having d | iamet | ter (d), pressure | e (p) | length (1) and | |
| | thic | kness (t), the | hoop s | stress is | | | | | |
| | (.) | pd | (1-) | pd 4t | (n) | pd | (d) | pd | |
| | (a) | 2t | (0) | 4t | (6) | 6t | (a) | 8t | |
| 195. | The | design of thir | e cylin | drical shell is b | ased | on | | | |
| | (a) | hoop stress | | | | | | | |
| | (b) | longitudinal s | tress | | | | | | |
| | (c) arithmetic mean of hoop and longitudinal stress | | | | | | | | |
| | (d) | none of these | | | | | | | |
| 196. | Hoo | p <mark>stress</mark> in a t | hin cy | ylindrical <mark>sh</mark> ell i | s | | | | |
| | (a) | longitudinal s | tress. | | (b) | Compressive str | ess. | | |
| | (c) | Circumferenti | al tens | sile stress. | (d) | radial stress. | | | |
| 197. The maximum shear stress in a thin cylindrical shell subjected to internal pressure p is | | | | | | | | | |
| | | pd | | pđ | | pd | (1) | pd | |
| | (a) | 2t | (b) | 4t | (c) | pd 6t | (a) | 8t | |
| 198. | For | a thin spheric | al shel | ll subjected to in | terna] | l pressure p, stre | ss pr | oduced is | |
| | | pd t | (b) | pd | (c) | pd | (d) | pd | |
| | (a) | t | (b) | $\overline{2t}$ | (c) | 4t | (a) | 8t | |
| 199. | The | longitudinal st | ress in | a riveted cylindr | ical sl | hell, subjected to i | intern | al pressure (p) is | |
| | | pd | | pd | | · pd | | pd | |
| ٠. | (a) | pd tn | (b) | pd 2tn | (c) | pd 4tn | (d) | 8tn | |
| 200. | For | a thin cylindri | cal sh | ell subjected to i | nterna | al pressure (p), th | ie vol | umetric strain is | |
| | | $\varepsilon_2 + \varepsilon_1$ | | | | $\varepsilon_2 + 2\varepsilon_1$ | | | |
| | | | | | | | | | |
| | | | | • | | | | | |

Strength of Materials

| Strength of Materials | internal diameter of the |
|---|--------------------------------------|
| Strength of Materials 201. A thin cylinder contains fluid at a pressure of 500 N/m ² , shell is 0.6 m and tensile stress in the material is to be li | mited to 9000 N/m ² . The |
| shell is 0.6 m and tensile stress in the shell must have a minimum wall thickness of nearly | |
| shell must have a minimum war (a) 17 mm | (d) 21 mm |

- 17 mm (c)
- 202. A thin walled cylindrical pressure vessel having a radius of 0.5 m and wall thickness of 25 mm is subjected to an internal pressure of 700 kPa. The hoop stress developed is
 - (a) 14 MPa
- 1.4 MPa (b)
- 0.14 MPa (c)
- (d) 0.014 MPa

203. The shear stress intensity is minimum at

(a) axis of shaft.

- surface of the shaft.
- (c) any inside layer of the shaft.
- (d) all the above are correct.

204. The criteria for the design of shaft is the stress at

(a) axis of the shaft.

the surface of the shaft.

(c) any inside layer.

all the above are wrong.

205. A masonry dam may fail due to

- (a) overturning of the dam.
- tension in the masonry at the base. (b)
- (c) crushing of masonry at the base.
- (d) any one of the above.

206. In the torsion equation $\frac{T}{J} = \frac{\tau}{R} = \frac{C\theta}{l}$, the term $\frac{J}{R}$ is called

- (d) none of these section modulus (c) polar modulus (a) shear modulus (b)
- 207. For a solid shaft of diameter D, torque (T) is given by

(a) $\frac{\pi}{16} \times \tau \times D^3$ (b) $\frac{\pi}{32} \times \tau \times D^3$ (c) $\frac{\pi}{16} \times \tau \times D^4$ (d) $\frac{\pi}{32} \times \tau \times D^4$ 208. Polar M.I. of a solid circular shaft of diameter D is given by (a) $\frac{\pi}{16}$ D³ (b) $\frac{\pi}{32}$ D³ (c) $\frac{\pi}{16}$ D⁴ (d) $\frac{\pi}{32}$ D⁴

209. The power transmitted by shaft is given by

The power transmitted by shaft is given by

(a)
$$P = \frac{2\pi NT}{60}$$
 watt (b) $P = \frac{2\pi NT}{4500}$ watt (c) $P = \frac{2\pi NT}{120}$ watt (d) $P = \frac{2\pi NT}{120}$ h.p.

210. When a shaft of diameter (D) is subjected to twisting moment (T) and bending moment (M), the equivalent twisting moment (Tc) is given by

(a) $\sqrt{M^2 + T^2}$

- (b) $\sqrt{M^2 T^2}$
- (c) $\frac{1}{2} \left(M + \sqrt{M^2 + T^2} \right)$
- (d) $\frac{1}{2} \left(M \sqrt{M^2 + T^2} \right)$

| 128 | | (T) and bending |
|---|--|---|
| 211. When a shaft of diameter (D) is sub- | ojected to twisting | moment (1) and bending |
| | | |
| (a) $\sqrt{M^2 + T^2}$ (b) $\sqrt{M^2 - T^2}$ (c) | $\frac{1}{2}\left(M + \sqrt{M^2 + T^2}\right)$ | (d) $\frac{1}{2} \left(M - \sqrt{M^2 + T^2} \right)$ |
| 7 C 41. | material. T | he diameter of share b x |
| 212. Two shafts A and B are made of the twice that of shaft A. The ratio of p | ower which can be | transmitted by shaft A to |
| twice that of shaft A. The ratio of p | OWC1 WHICH SW | (GATE) |
| that of shaft B is | 1 | 1 |
| (a) $\frac{1}{2}$ (b) $\frac{1}{4}$ | (c) $\frac{1}{8}$ | (d) $\frac{1}{16}$ |
| (a) 2 · · · 4 | c. : a travice its ins | ide diameter. The ratio of |
| (a) $\frac{1}{2}$ (b) 4 213. The outside diameter of a hollow s | t of solid shaft of | the same material and the |
| its torque carrying capacity to tha | t or some same | (GATE) |
| same outside diameter is | _ | |
| (a) $\frac{15}{16}$ (b) $\frac{3}{4}$ | (c) $\frac{1}{2}$ | (d) $\frac{1}{16}$ |
| ` 10 | hiloz e no boita | circular shaft. If maximum |
| (a) 16 4 214. Bending moment M and torque T is | hoor stress develope | d than, M is equal to (IES) |
| 214. Bending moment M and torque T is bending stress equals to maximum s | near stress | |
| (a) $\frac{T}{2}$ (b) T | (c) 2T | (d) 4T |
| (a) $\frac{1}{2}$ 215. For two shafts connected in parall | is which of the fol | lowing statement is true? |
| 215. For two shafts connected in paran | lei, which of | (IES) |
| | | |
| (a) Torque in each shaft is the san | ne | |
| (b) Shear stress in each shaft is th | e same | |
| (c) Angle of twist of each shaft is | the same | |
| (d) Torsional stiffness of each sha | it is the same | wist of 1° in a length of 120 |
| (d) Torsional stiffness of each sha 216. A circular shaft is subjected to to | rsion undergoes a t | 1000 kg/cm ² and modulus of |
| 216. A circular shaft is subjected to to cm. If maximum shear stress ind | uced is limited to | off is (IES) |
| cm. If maximum shear stress flur rigidity $G = 0.8 \times 10^6 \text{ kg/cm}^2$, the | en radius of the sha | 27 |
| π π | (c) 18 | (d) $\frac{1}{\pi}$ |
| rigidity G = 0.8 × 10° kg/cm ⁻ , the (a) $\frac{\pi}{18}$ (b) $\frac{\pi}{27}$ | π | ring is called |
| | Antinotion iii a si |) |
| | | rigidity. (d) Young's modulus. an axial load, it is said to be |
| | aina is sublected to | AH AMMI IVIII |

218. When a closely-coiled helical spring is subjected to an axial load, it is said to be under

(a) bending (b) shear (c) torsion (d) crushing

(a) bending (b) shear (c) torstood (219. A spring used to absorb shocks and vibrations is

A spring used to absorb snocks and vibrations is

(a) torsion spring (b) leaf spring (c) helical spring (d) conical spring

Shear stress

Maximum normal stress

(b)

(a) Principal stress

(c) Normal stress
Objective Civil Eng. \ 2016 \ 17

| 229. | A th | rust diagram | indic | ates | | | | | | |
|------|------|-----------------|---------|------------------------|--------|---|--|--|--|--|
| | (a) | shear force | (b) | Transverse force | (c) | axial force (d) none of these | | | | |
| 230. | A b | rittle material | exhib | oits | | • | | | | |
| | (a) | Large plastic | deforn | nation | (b) | Large elastic deformation | | | | |
| | (c) | Large yield pl | ateau | | (d) | no plastic deformation | | | | |
| 231. | A th | in walled cylir | ıdrica | l pressure pipe i | s mac | de by welding two semi circular parts | | | | |
| | alon | g the length. | Under | working condit | ions, | the weld shall be under | | | | |
| | (a) | Direct tension | . (b) | direct shear. | (c) | direct compression.(d) Torsional shear. | | | | |
| 232. | In c | ase of pure sh | ear c | ondition, princip | al pl | lanes lie at | | | | |
| | (a) | 45° to 135° | (b) | 90° to 135° | (c) | 0° to 90° (d) 90° to 180° | | | | |
| 233. | A ca | intilever beam | carry | ing UDL over it | s enti | ire span is to be replaced by a simply | | | | |
| | supp | ported beam o | f sam | e span. The max | kimur | m bending stress will be | | | | |
| | (a) | reduced by tw | o time | es. | (b) | reduced by four times. | | | | |
| | • / | increased by t | | | ` ' | decrease by four times. | | | | |
| 234. | A ci | rcular section | of be | am having dia l |) is 1 | replaced by square section of side D, | | | | |
| | | bending stress | | | | | | | | |
| | | _ | | increase | | | | | | |
| 235. | A m | | _ | shear ultimately | | | | | | |
| | (a) | uniaxial stress | cond | ition. | (b) | biaxial stress condition. | | | | |
| | (c) | Triaxial stress | condi | ition. | (d) | zero stress condition. | | | | |
| 236. | _ | | ver w | ill have <mark></mark> | reac | ctions. | | | | |
| | (a) | 2 | (b) | 3 | (c) | 4 (d) 1 | | | | |
| 237. | A si | mply supporte | d bea | m is replaced by | y a fi | ixed beam carrying point load at mid | | | | |
| | - | i. The maximu | | flection will | | | | | | |
| | ` ' | increase to 4 | | | ` ' | reduce to four times | | | | |
| | ` , | increase to 5 | | | . , | reduce to five times | | | | |
| 238. | | | _ | _ | | ire span is to be replaced by a simply | | | | |
| | | | f sam | e span. Nature | of str | ress at top and bottom fibre of cross | | | | |
| | | ion will | | | | | | | | |
| • | (a) | reverse | | | | no change | | | | |
| | (c) | depends on in | • | | (d) | Depend on span length | | | | |
| 239. | | | | - | _ | n a Mohr's circle represents | | | | |
| | (a) | Major principa | | | (b) | • • | | | | |
| | (c) | Maximum she | | | (d) | None of the above | | | | |
| 240. | | | | on, principal str | | | | | | |
| | (a) | at 45° and 13: | | - | (b) | applied stress itself | | | | |
| | (c) | average of app | plied s | stress | (d) | none of the above | | | | |

(b) Rankine's formula

(d) Straight line formula

(a) Euler's formula

(c) Perry's formula

| 252. If yy axis is tangential to Mohr's circle | it represents |
|---|---|
| (a) uniaxial stress condition | (b) biaxial stress condition |
| (c) Pure shear condition | (d) Shear and uniaxial stress condition |
| 253. A shaft carrying 3 kN.m bending m | oment and 4 kN.m torsional moment. |
| Equivalent torsional moment producing | same shear stress will be |
| (a) 3 kN.m (b) 7 kN.m | (c) 5 kN.m (d) 1 kN.m |
| 254. A long closed cylindrical pressure vesse | el is made by joining different cylindrical |
| parts using lap joint along the peripher | |
| (a) bending shear (b) direct compression | |
| 255. Shear centre of symmetrical I section w | vill lie |
| (a) at centroid | (b) outside the section |
| (c) At bottom most fibre | (d) at top most fibre |
| 256. In case of beam having channel section | placed as C, the direction of shear flow in |
| flanges shall be | |
| (a) in same direction | (b) in opposite direction |
| (c) dependent on thickness of flange | (d) dependent on width of flange |
| 257. The diameter of kern (core) section of | |
| (a) $d/2$ (b) $d/3$ | (c) d/4 (d) d/8 |
| 258. In thin spherical shell, the ratio of rad | |
| (a) $\frac{1}{2}$ (b) 1 | (c) 2 (d) $\frac{1}{4}$ |
| | 7 |
| 259. In case of two point loading test the be | (c) varying (d) data insufficient |
| (a) zero (b) constant | (*) |
| 260. Impact test measures of mater | |
| (a) Hardness (b) ductility 261. The S.F. diagram for a cantilever bear | (4) |
| | (b) zero |
| (a) horizontal straight line | (d) inclined straight line |
| (c) parabola | |
| 262. Lateral strains are longitudinal | (b) always greater than |
| (a) always equal | (d) sometimes less than |
| (c) always less than 263. When a simply supported beam with or | √ − <i>y</i> |
| 263. When a simply supported beam with or | ng, the nature of bending stress along top |
| fibres at the central span shall be | |
| (a) flexural compressive | (b) flexural tensile |
| (c) axial compressive | (d) axial tensile |
| (o) axiai compiossivo | |

| | **** | | 11 . | | 111 2 4 1 | | |
|------|------|----------------------------------|----------|--------------------|--------------|-----------------------------------|--|
| 264. | | ich of the to led by u.d.l. | | _ | - | ave the point | of contraflexure when |
| | (a) | Simply supp | orted b | eam | (b) | cantilever bea | nm |
| | (c) | overhanging | beam | | (d) | none of the a | bove |
| 265. | | a proped ca p will be | ntileve | r beam ca | rrying UDI | w kN/m on o | entire span, reaction a |
| | (a) | $\frac{5}{6}$ wl | (b) | $\frac{3}{8}$ wl | (c) | $\frac{1}{2}$ wl | (d) $\frac{3}{4}$ wl |
| 266. | A c | antilever bea | m of le | ength L ha | s flexural | rigidity EI up | to length L/2 from the |
| | | | | | t carries a | moment M at | the free end. The slope |
| | at t | he free end i | s giver | by | | | |
| | (a) | $\frac{\text{ML}^2}{2\text{EI}}$ | (b) | $\frac{ML^2}{3EI}$ | (c) | $\frac{3ML}{2EI}$ | (d) $\frac{2ML}{3EI}$ |
| 267. | The | rectangular | section | n with side | s 3 m and | 6 m has a cor | ·e |
| | (a) | Parallelogran | n of si | des 2 m, ar | nd 4 m | | |
| | (b) | square diago | nal of | sizes 1 m a | and 3 m | | |
| | (c) | circular of ra | adius 3 | m | | 777 | |
| | (d) | Rhombus dia | agonal | of sizes 1 i | m and 2 m | | |
| 268. | Prir | icipal stress | on the | plane of n | naximum s | hear stress is | |
| | (a) | Maximum | (b) | zero | (c) | Minimum (| d) None of the above |
| 69. | A ro | | d centr | ally in a t | ube and as | sembly is tight | tened by rigid washers |
| | (a) | rod is subject | ted to | compressiv | e load | | |
| | (b) | tube is subje | cted to | compressi | ve load | | |
| | (c) | both are sub | jected 1 | to compress | sive load | | |
| | (d) | rod is subject | ted to | compressiv | e, tube is s | ubjected to ten | sile |
| 270. | | | | | | y. Due to wind ing capacity of | l, a UDL acts along its column will |
| | (a) | increase | | | (b) | decrease | * - × |
| | (c) | Remain same | e | | (d) | Depends on the | he direction of wind |

Objective Civil Engineering

134

| : ANSWER | S : |
|----------|------------|
|----------|------------|

| | | ANDVIDIO | <u></u> | |
|----------|----------|----------|----------|--------------------|
| 1. (a) | 2. (c) | 3. (a) | 4. (b) | 5. (c) 10. (c) |
| 6. (d) | 7. (b) | 8. (c) | 9. (d) | 10. (c) 15. (b) |
| 11. (b) | 12. (c) | 13. (b) | 14. (c) | 20. (c) |
| 16. (c) | 17. (a) | 18. (c) | 19. (b) | 25. (d) |
| 21. (d) | 22. (a) | 23. (d) | 24. (a) | 30. (c) |
| 26. (b) | 27. (c) | 28. (a) | 29. (b) | 35. (d) |
| 31. (d) | 32. (a) | 33. (c) | 34. (b) | |
| 36. (a) | 37. (d) | 38. (a) | 39. (a) | 40. (c) |
| 41. (b) | 42. (c) | 43. (b) | 44. (a) | 45. (b) |
| 46. (a) | 47. (b) | 48. (c) | 49. (c) | 50. (d) |
| 51. (b) | 52. (c) | 53. (c) | 54. (a) | 55. (d) |
| 56. (b) | 57. (a) | 58. (b) | 59. (a) | 60. (b) |
| 61. (c) | 62. (a) | 63. (d) | 64. (a) | 65. (b) |
| 66. (b) | 67. (a) | 68. (c) | 69. (b) | 70. (d) |
| 71. (b) | 72. (c) | 73. (a) | 74. (b) | 75. (b) |
| 76. (c) | 77. (b) | 78. (a) | 79. (b) | . 80. (b) |
| 81. (b) | 82. (b) | 83. (b) | 84. (c) | 85. (c) |
| 86. (c) | 87. (d) | 88. (a) | 89. (a) | 90. (a) |
| 91. (d) | 92. (b) | 93. (b) | 94. (c) | 95. (a) |
| 96. (d) | 97. (c) | 98. (c) | 99. (c) | 100. (b) |
| 101. (a) | 102. (b) | 103. (c) | 104. (d) | 105. (c) |
| 106. (d) | 107. (c) | 108. (b) | 109. (a) | 110. (a) |
| 111. (b) | 112. (b) | 113. (c) | 114. (a) | 115. (a) |
| 116. (b) | 117. (a) | 118. (d) | 119. (c) | 120. (c) |
| 121. (b) | 122. (d) | 123. (a) | 124. (b) | 125. (d) |
| 126. (b) | 127. (b) | 128. (b) | 129. (c) | 130. (c) |
| 131. (b) | 132. (a) | 133. (c) | 134. (a) | 135. (d) |
| 136. (b) | 137. (a) | 138. (a) | 139. (c) | 140 (b) |
| 141. (d) | 142. (b) | 143. (d) | 144. (b) | 145. (c) |
| 146. (d) | 147. (d) | 148. (c) | 149. (b) | 150. (c) |
| 151. (c) | 152. (d) | 153. (d) | 154. (d) | 155. (c) |
| 156. (b) | 157. (a) | 158. (c) | 159. (c) | 160. (d) |
| 161. (a) | 162. (a) | 163. (a) | 164. (b) | 165. (d) |
| 166. (a) | 167. (c) | 168. (c) | 169. (d) | 170. (b) |
| 171. (b) | 172. (b) | 173. (d) | 174. (a) | 175. (c) |
| 176. (c) | 177. (d) | 178. (d) | 179. (c) | 180. (b) |

Strength of Materials

| 181. (b) | 182. (c) | 183. (c) | 184. (a) | 185. (b) |
|----------------------|----------------------|----------|----------|----------|
| 186. (b) | 187. (c) | 188. (a) | 189. (b) | 190. (d) |
| 191. (c) | 192. (d) | 193. (b) | 194. (a) | 195. (a) |
| 196. (c) | 197. (d) | 198. (c) | 199. (c) | 200. (c) |
| 201. (c) | 202. (a) | 203. (a) | 204. (b) | 205. (d) |
| 201. (c) 206. (c) | 207. (a) | 208. (c) | 209. (a) | 210. (a) |
| 200. (c) 211. (c) | 212. (c) | 213. (a) | 214. (a) | 215. (c) |
| 211. (c) 216. (d) | 217. (b) | 218. (c) | 219. (b) | 220. (b) |
| 216. (d) 221. (d) | 222. (b) | 223. (a) | 224. (c) | 225. (b) |
| 1 ' ' | 227. (b) | 228. (b) | 229. (c) | 230. (d) |
| 226. (d) | 232. (c) | 233. (b) | 234. (c) | 235. (c) |
| 231. (b) | 232. (b) 237. (b) | 238. (a) | 239. (a) | 240. (b) |
| 236. (c) | 242. (b) | 243. (b) | 244. (b) | 245. (b) |
| 241. (d) | | 248. (a) | 249. (d) | 250. (b) |
| 246. (c) | 247. (c) | 253. (c) | 254. (c) | 255. (a) |
| 251. (b) | 252. (a) | | 259. (b) | 260. (c) |
| 256. (a) | 257. (c) | 258. (b) | 264. (c) | 265. (b) |
| 261. (a) | 262. (c) | 263. (b) | 269. (c) | 270. (b) |
| 266. (c) | 267. (d) | 268. (b) | 209. (0) | 270. (0) |

EXPLANATIONS

39. (a)
$$\sigma = \left(\alpha t - \frac{\delta}{l}\right) E$$

$$= (2^{7} \times 10^{-6} \times 100 - \frac{0.01}{25}) \times 1 \times 10^{6}$$

$$= -200 \text{ kg.cm}^{2} \text{ (tensile)}$$

46. (a)
$$\delta l = l \alpha t$$

= 15000 × 12 × 10⁻⁶ × 25
= 4.5 mm (gap)

48. (c) Four elastic constants are required E, K, G and μ

E, K, G and
$$\mu$$

62. (a) $E = 2G (1 + \frac{1}{m})$ $\mu = 0.2 = \frac{1}{m}$
 $E = 2G (1 + 0.2)$ $\frac{E}{G} = 2.4 = \frac{12}{5}$

64. (a)
$$E = 2G (1 + \frac{1}{m})$$

$$2 \times 10^6 = 2G (1 + 0.25)$$

$$G = 0.8 \times 10^6 \text{ kg/cm}^2$$

65. (b)
$$E = 2G (1 + \frac{1}{m})$$

$$2.1 \times 10^6 = 2 \times 0.8 \times 10^6 (1 + \frac{1}{m})$$

$$\therefore 1 + \frac{1}{m} = 1.3125$$

$$\therefore \frac{1}{m} = \mu = 0.3125$$

66. (b)
$$\varepsilon = \frac{\delta l}{l} = \frac{0.03}{20} = 0.0015$$

$$\varepsilon' = \frac{\delta d}{d} = \frac{0.0018}{4} = 4.5 \times 10^{-4}$$

$$\therefore \quad \mu = \frac{\epsilon'}{\epsilon} = \frac{4.5 \times 10^{-4}}{0.0015} = 0.3$$

82. (b)

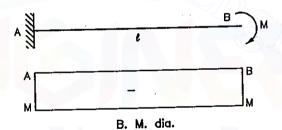
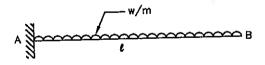
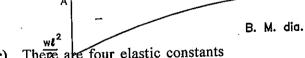


FIG. A-82





There are four elastic constants 84. (c)

E, K, G and μ

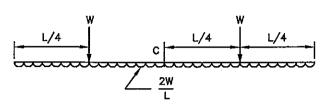
FIG. A-83

But only E and μ are independent

87. (d)

Strength of Materials

$$M_{c} = \frac{2W}{L} \times \frac{L}{2} \times \frac{L}{4} - W \times \frac{L}{4}$$
$$= \frac{WL}{4} - \frac{WL}{4}$$
$$= 0$$



88. (a) Taking moment @ A

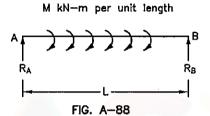
$$R_B \times L = M.L$$

$$\therefore$$
 $R_B = M$

B.M. at mid span =
$$R_B \times \frac{L}{2} - M \cdot \frac{L}{2}$$

= $M \cdot \frac{L}{2} - M \cdot \frac{L}{2}$
= 0

FIG. A-87



137

95. (a)

$$R_B \times l = \frac{1}{2} \times w \cdot l \times \frac{1}{3} l$$

$$\therefore R_{\rm B} = \frac{wl}{6} \dots SF \text{ at B}$$

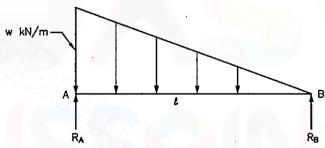


FIG. A-95

105. (c) For fixed beam, $\delta_1 = \frac{Wl^3}{192 EI}$

For simply supported beam, $\delta_2 = \frac{Wl^3}{48 EI}$

 $\therefore \qquad \delta_1 = \frac{1}{4} \, \delta_2$

106. (d) For fixed beam, $\delta_1 = \frac{wl^4}{384 EI}$

For simply supported beam, $\delta_2 = \frac{5}{384} \frac{wl^4}{EI}$

$$\therefore \quad \delta_2 = 5\delta_1$$

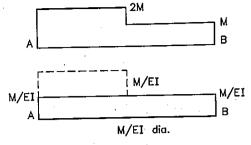
113. (c) Moment area Method:

$$y_B = moment of \frac{M}{EI} dia$$

between B and A about B

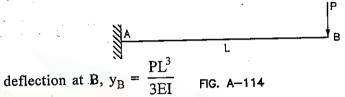
Objective Civil Eng. \ 2016 \ 18

$$= \frac{M}{EI} \times L \times \frac{L}{2} = \frac{ML^2}{2EI}$$



114. (c) Part AC is rigid, \therefore EI $\rightarrow \infty$

Hence for calculation of deflection at B beam can be repromised and



B.M. at $A = P \times 2L$ (no effect of rigidity of AC)

= 2PL

115. (a) QR is rigid.

Structure can be converted into a cantilever subject to act moment WL at Q, and oncentrated load W at Q.

$$\therefore \quad \text{def. at Q} = \frac{\text{WL}^3}{3\text{EI}} + \frac{(\text{WL})\text{L}^2}{2\text{EI}} \qquad \text{P} \qquad \qquad \text{L}$$

$$= \frac{2\text{WL}^3 + 3\text{L}^3}{6\text{EI}} = \frac{5\text{WL}^3}{6\text{EI}} \qquad \text{Fig. A-115}$$

126. (b) def. at B due to u.d.l. = def. due to point load

$$\frac{\text{w}l^4}{8\text{EI}} = \frac{\text{W}l^3}{3\text{EI}}$$

$$\frac{\text{w}l^4}{8} = \frac{\text{W}l^3}{3}$$

$$\therefore W = \frac{3}{8} \text{ w}l \quad \text{Fig. A-126}$$

150. (c)
$$r_{min} = \sqrt{\frac{I}{A}} = \sqrt{\frac{\frac{\pi}{64} \times d^4}{\frac{\pi}{4} \times d^2}} = \frac{d}{4}$$

Slenderness ratio, $\lambda = \frac{l_e}{r_{min}}$

$$\therefore 160 = \frac{l_{\rm e} \times 4}{\rm d}$$

$$\therefore \frac{l_e}{d} = 40$$

152. (d) End A allows only axial deformation which is negligible in case of buckling for long column. At end B, k_T approaches infinity, it will behave as fixed end. So, both ends behave as fixed ends.

$$\therefore \quad P_{cr} = \frac{\pi^2 EI}{\left(\frac{L}{2}\right)^2} = 4 \frac{\pi^2 EI}{L^2} \qquad \therefore \quad \alpha = 4$$

153. (d) One end is fixed, other free

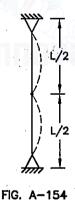
$$\therefore l_e = 2L$$

154. (d)
$$P_{cr} = \frac{\pi^2 EI}{l^2} = 200 \text{ kN}$$

For new condition

$$l_{\rm e} = \frac{l}{2}$$

$$P_{cr} = \frac{\pi^2 EI}{(l_e)^2} = \frac{\pi^2 EI}{\left(\frac{l}{2}\right)^2} = \frac{4\pi^2 EI}{l^2}$$
= 4 × 200
= 800 kN



164. (b)
$$\frac{f}{y} = \frac{E}{R}$$

$$f = y \times \frac{E}{R} = 1 \times \frac{2 \times 10^6}{10 \times 100} = 2 \times 10^3 \text{ kg/cm}^2$$

171. (b)
$$\sigma_t = \frac{\sigma_1 - \sigma_2}{2} \sin 2\theta - \tau \cos 2\theta$$
 $\tau = 0$

$$= \frac{(2000 - 1000)}{2} \sin 30^\circ - 0$$

$$= 250 \text{ kg./cm}^2$$
 $\sigma_2 = 100 \text{kg/cm}^2$

 $\theta=15^{\circ}$ $\sigma_1=2000 \text{kg/cm}^2$

172. (b)
$$\sigma_1 = 173 \text{ mPa}$$

 $\sigma_2 = 0$
 $\tau = 0$ For τ_{max} , $\theta = 45^\circ$

$$\tau_{\text{max}} = \sigma_{\text{t}} = \frac{\sigma_{\text{l}} - \sigma_{\text{2}}}{2} \sin 2\theta - \tau \cos 2\theta$$
$$= \frac{173}{2} \sin 90^{\circ} - 0$$

173. (d)
$$\sigma_{11} = 30$$
, $\sigma_{12} = 0$, $\sigma_{21} = 0$, $\sigma_{22} = 30$

:. Principal stresses are,

$$\sigma_{11} = 30, \ \sigma_{22} = 30$$

$$\therefore \text{ radius} = \frac{\sigma_{11} - \sigma_{12}}{2} = \frac{30 - 30}{2} = 0$$

centre of Mohr's circle will be (30, 0)

174. (a) Isotropic state means having same material properties in all directions. iso = same, tropic = direction

175. (c)
$$I_{AB} = I_g + ah^2$$

= $\frac{bd^3}{12} + bd \times \left(\frac{d}{4}\right)^2$

$$= \frac{bd^3}{12} + \frac{bd^3}{16}$$
$$= \frac{7bd^3}{48}$$

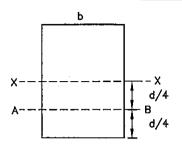


FIG. A-175

184. (a)
$$\sigma_A = \sigma_0 + \sigma_{bx} - \sigma_{by}$$

= 400 + 1200 - 800
= 800 t/m² (compressive)

185. (b)
$$M = 2.25 \text{ t.m}$$

= 2.25 × 1000 × 100 = 225000 kg.cm

$$I = \frac{15 \times 30^3}{12} = 33750 \text{ cm}^4$$

$$y = \frac{30}{2} = 15 \text{ cm}$$

$$\frac{M}{I} = \frac{f}{y} \qquad \qquad \therefore \qquad \frac{225000}{33750} = \frac{f}{15}$$

$$f = 100 \text{ kg/cm}^2$$

186. (b)
$$\tau_{ave} = \frac{F}{A} = \frac{F}{\frac{\pi}{2} \times d^2} = \frac{4F}{\pi d^2}$$

For circular section,

$$\tau_{\text{max}} = \frac{4}{3} \tau_{\text{ave}}$$
$$= \frac{4}{3} \times \frac{4F}{\pi d^2} = \frac{16F}{3\pi d^2}$$

190. (d) For gradual load,
$$u = \frac{\sigma^2}{2E} \times v$$

for sudden load,
$$u = \frac{(2\sigma)^2 \times v}{2E}$$
$$= \frac{4\sigma^2 \times v}{2E}$$

192. (d)
$$U_1 = \frac{1}{2} \times P_1 \times \text{elongation}$$

$$= \frac{1}{2} \times P_1 \times \frac{P_1 L}{AE} = \frac{P_1^2 L}{2AE}$$
Similarly, $U_2 = \frac{P_2^2 L}{2AE}$

$$\therefore U_1 + U_2 = \frac{L}{2AE} \left(P_1^2 + P_2^2 \right) \dots \text{(i)}$$
Now,
$$U = \frac{1}{2} \times \left(P_1 + P_2 \right) \times \frac{\left(P_1 + P_2 \right) L}{AE}$$

$$= \frac{1}{2} \frac{\left(P_1 + P_2 \right)^2 L}{AE} = \frac{L}{2AE} \times \left(P_1^2 + P_2^2 + 2P_1P_2 \right) \dots \text{(iii)}$$

$$\therefore U > U_1 + U_2$$

201. (c)
$$f = \frac{pd}{2t}$$

$$9000 = \frac{500 \times 0.6}{2t}$$

$$\therefore$$
 t = 0.0166 m = 16.7 mm \approx 17 mm

202. (a)
$$f_1 = \frac{pd}{2t} = \frac{(0.7) \times 1000}{2 \times 25}$$
 $p = 700 \text{ kPa}$
= 14 N/mm² (MPa) = 700 kN/m² = 0.7 N/mm²

212. (c) For a shaft P
$$\propto$$
 T

shaft A ,
$$T_1 = \frac{\pi}{16} \times \tau \times D^3$$

Shaft B,
$$T_2 = \frac{\pi}{16} \times \tau \times (2D)^3$$

$$\frac{P_1}{P_2} = \frac{T_1}{T_2} = \frac{16}{8} \times \tau \times D^3 \times 8$$

213. (a) For hollow shaft

$$\begin{split} T_1 &= \frac{\pi}{16} \times \tau \times \frac{\left(D^4 - d^4\right)}{D} = \frac{\pi}{16} \times \tau \frac{\left(\left(2d\right)^4 - d^4\right)}{2d} \\ &= \frac{\pi}{16} \times \tau \times \frac{15d^4}{2d} \\ &= \frac{\pi}{16} \times \tau \times d^3 \times \frac{15}{2} \quad ... \quad (i) \end{split}$$

For solid shaft,

$$T_{2} = \frac{\pi}{16} \times \tau \times D^{3}$$

$$= \frac{\pi}{16} \times \tau \times (2d)^{3} = \frac{\pi}{16} \times \tau \times d^{3} \times 8 \dots (ii)$$

$$\therefore \frac{T_{1}}{T_{2}} = \frac{15}{2 \times 8} = \frac{15}{16}$$

214. (a) bending stress
$$\sigma = \frac{32 \text{ M}}{\pi \text{D}^3}$$

shear stress
$$\tau = \frac{16T}{\pi D^3}$$

Since, $\sigma = \tau$

$$\frac{32M}{\pi D^3} = \frac{16T}{\pi D^3} \qquad \therefore \qquad M = \frac{T}{2}$$

$$M = \frac{T}{2}$$

216. (d)
$$\frac{\tau}{r} = \frac{G\theta}{l}$$

$$\frac{1000}{r} = \frac{0.8 \times 10^6 \times 1 \times \pi}{120 \times 180}$$

$$\therefore \quad \mathbf{r} = \frac{27}{\pi}$$

224. (c) Taking moment @ A
$$R_{B} \times L = w \times L \times L$$

$$\therefore R_B = wL$$

$$\therefore R_A = \text{Total load} - R_B$$
$$= wL - wL = 0$$

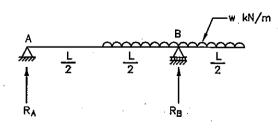


FIG. A-224

233. (b) For cantilever beam,
$$M = \frac{wl^2}{2}$$

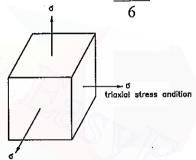
For simpli supported beam, $M = \frac{wl^2}{8}$

$$f = \frac{M}{Z}$$
 \therefore $f \propto M$

234. (c) For circular section,
$$f = \frac{M}{Z} = \frac{M}{\frac{\pi}{32} D^3} = \frac{19.18 M}{D^3}$$

For square section,
$$f = \frac{M}{Z} = \frac{M}{\frac{D}{D}^2} = \frac{6M}{D^3}$$

235. (c)



237. (b) For simply supported beam,
$$\delta = \frac{Wl^3}{48 EI}$$

For fixed beam,
$$\delta = \frac{Wl^3}{192 EI}$$

242. (b)

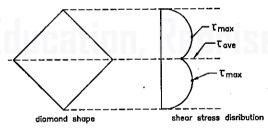
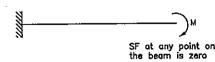


FIG. A-242

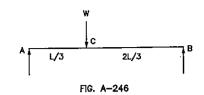
245. (b)



SF at any point on the beam is zero.

Strength of Materials

246. (c)
$$R_B \times L = W \times \frac{L}{3}$$
 $\therefore R_B = \frac{W}{3}$
 $\therefore M_c = \frac{W}{3} \times \frac{2L}{3} = \frac{2WL}{9}$



249. (d)
$$P_E = \frac{\pi^2 EI}{(l_e)^2}$$
 $I_1 = \frac{b.d^3}{12} = \frac{b^4}{12}$

$$I_1 = \frac{b \cdot d^3}{12} = \frac{b^4}{12}$$

252. (a)

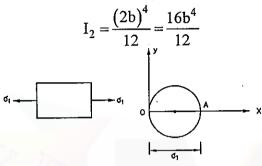
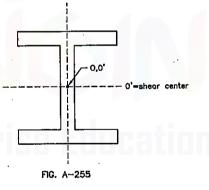


FIG. A-252

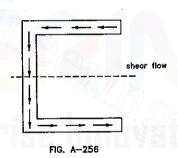
253. (c)
$$T_e = \sqrt{M^2 + T^2}$$

= $\sqrt{3^2 + 4^2}$
= 5 kN.m

255. (a)



256. (a)



257. (c) For circular section

$$e_{lim} = \frac{d}{8}$$

$$\therefore \text{ dia. of core} = 2 \times \frac{d}{8} = \frac{d}{4}$$

258. (b) For thin spherical shell

= circumferential stress radial stress

$$=\frac{pd}{4t}$$

Objective Civil Eng. \ 2016 \ 19

Objective Civil Engineering 146 261. (a) SFD BMD FIG. A-261 265. (b) 263. (b) Tension at top FIG. A-263 FIG. A-265 I/2L 2 Fig. A-266 267. (d) 6m

FIG. A-267

| 1. | The load | on a | spring | per | unit | deflection, | is | called |
|----|----------|------|--------|-----|------|-------------|----|--------|
|----|----------|------|--------|-----|------|-------------|----|--------|

- (a) stiffness
- (b) Proof resilience (c) Proof stress
- (d) Proof load

At yield point of a test specimen, the material 2.

(a) Obey's Hook's law

- (b) behaves in an elastic manner
- (c) regains its original shape
- (d) undergoes plastic deformation

Slenderness ratio of long column is 3.

- area of cross section divided by radius of gyration
- area of cross section divided by least radius of gyration
- (c) radius of gyration divided by area of cross section
- length of column divided by least radius of gyration.

The greatest eccentricity which a load W can have without producing tension on the cross section of a short column of external diameter D and internal diameter d, is

(a)
$$\frac{4W}{\pi \left(D^2 - d^2\right)}$$

$$\frac{4W}{\pi(D^2 - d^2)}$$
 (b) $\frac{\pi(D^4 - d^4)}{32 D}$ (c) $\frac{D^2 + d^2}{8D}$ (d) $\frac{D^2 - d^2}{8D}$

(c)
$$\frac{D^2 + d^2}{8D}$$

(d)
$$\frac{D^2 - d^2}{8D}$$

Explanation:

4. (c)
$$e_{max} = \frac{Z}{A}$$

4. (c)
$$e_{\text{max}} = \frac{Z}{A}$$
 $Z = \frac{I}{V_{\text{max}}} = \frac{\pi}{64} \frac{(D^4 - d^4)}{D/2}$

$$=\frac{\pi}{32\,\mathrm{D}}\left(\mathrm{D}^4-\mathrm{d}^4\right)$$

$$A = \frac{\pi}{4} \left(D^2 - d^2 \right)$$

$$\therefore \quad e_{\text{max}} = \frac{Z}{A} = \frac{\frac{\pi}{32D} \left(D^4 - d^4 \right)}{\frac{\pi}{4} \left(D^2 - d^2 \right)} = \frac{1}{8D} \frac{\left(D^2 + d^2 \right) \left(D^2 - d^2 \right)}{\left(D^2 - d^2 \right)}$$

$$=\frac{\left(D^2+d^2\right)}{8D}$$

Stress may be expresed in 5.

- (a) N/mm^2
- (b) N/cm²
- (c) N/m^2
- None of the above

For a beam of uniform strength if breadth is constant 6.

- (a) depth d $\alpha \sqrt{M}$
- (b) depth d α M (c) depth d α $3\sqrt{M}$
- (d) depth d $\alpha \frac{1}{M}$

7. A beam is said to be of uniform strength if

- The shear stress is constant throughout the beam
- The extreme fibre stress is the same at every section
- bending moment is constant throughout the length of the beam
- deflection is the same throughout the length of the beam.

The middle third rule applies to the retaining wall for its stability against

(a) 0.25%

8.

| | (a) Sliding (b) tension (c) Overturning (d) all of the above |
|-----|--|
| 9. | When a body is in equilibrium undergoes an infinitely small displacement, world |
| | imagined to be done is known as |
| | (a) imaginary work (b) negative work (c) Virtual work (d) none of these |
| 10. | The effect of number of loads applied simultaneously is the sum of the effect o |
| 20. | each load. This statement is the principle of |
| | (a) Superposition (b) complementary load |
| | (c) reciprocal theorem (b) transmissibility |
| 11. | The angle of obliquity ϕ , the normal stress σ_n and the tangential shear stress σ_t are |
| 11. | related to an oblique plane of an element. The resultant stress σ_r is expressed by : |
| | (a) $\sigma_r = \sigma n^2 + \sigma t^2$ (b) $\sigma_r = \sqrt{\sigma n^2 + \sigma t^2}$ (c) $\sigma_r = \sigma n + \sigma t$ (d) $\sigma_r^2 = \sqrt{\sigma n^2 + \sigma t^2}$ |
| 10 | For an element in pure shear, principle planes are oriented at |
| 12. | (1) 750 |
| - | (a) 43 |
| Exp | olanation: |
| | 12. (a) $\tan 2\alpha_1 = \frac{2\tau}{\sigma_1 - \sigma_2} = \frac{2\tau}{0} = \infty$ |
| | 12. (a) $\sigma_1 - \sigma_2 = 0$ |
| | $2\alpha_1 = \tan^{-1} \infty$ |
| | $= 90^{\circ} \qquad \therefore \alpha_1 = 45^{\circ}$ |
| 13. | The amount of energy absorbed by the material under impact load is known as |
| | (a) Durability (b) Toughness (c) Ductility (d) Hardness |
| 14. | Resistance of a material against reversel of stresses is |
| | (a) tensile strength (b) compressive strength |
| | (c) fatigue strength (d) damping |
| | INPISE FILICATION REPORTS INDIVISION |
| 15. | $P = \frac{\pi^2 EI}{4L^2}$ is the equation of Euler's crippling load, if |
| | (a) both ends are fixed |
| | (b) both ends are hinged. |
| | (c) one end is fixed and other end is free |
| | (d) one end is fixed and other end is hinged. |
| | Explanation: 15 (c) $P = \frac{\pi^2 EI}{le^2} = \frac{\pi^2 EI}{(2L)^2} = \frac{\pi^2 EI}{4L^2}$ |
| | For one end fixed, other free, $le = 2L$ |
| 17 | The state of the s |
| 16 | (a) 10% (d) 8 % |
| | (a) 0.25% (b) 12% (c) 10% (d) 5% |

(b) 12%

Structural Analysis

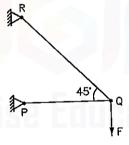
- For a perfect truss with j is the number of joints and m is the number of member,
 - (a) m = 2j 3
- (b) m > 2j 3
- (c) m < 2j 3
- (d) j = 2m 3

- A truss is said to be deficient if
 - (a) m = 2j 3
- (b) m > 2j 3
- m < 2j 3(c)
- (d) j = 2m 3

- A framed structure of a triangular shape is
 - (a) Perfect
- (b) imperfect
- (c) deficient
- (d) redundant

- A redundant frame/truss is also called
 - (a) Perfect
- (b) imperfect
- (c) deficient
- (d) none of these
- In the analysis of truss the incorrect assumption is,
 - all the joints are pinned joints.
 - (b) External forces are acting at the joint only.
 - Members are subjected to transverse loads.
 - Self weight of the member is neglected.
- In a cantilever truss it is very essential to find out the reactions before analyzing it.

- (b) disagree
- The force in member PQ of the truss PQR is



- (a) F (compression)
- (b) $F\sqrt{2}$ (Compression)
- (c) F (tension)
- (d) $F/\sqrt{2}$ (tension)

FIG. Q-7

In the truss shown in figure the forces in members AB and BC will be respectively (plus denotes tension)

- zero, zero (a)
- $-W/\cos 60^{\circ}$, + W cos 60°
- W/cos 60°, zero (c)
- zero, + W cos 60°

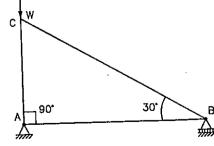
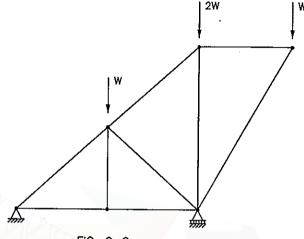


FIG. Q-8

| 17. | For a rectangular | foundation of wi | dth b, eccentricity | of load should note exceed | | | | | | | | |
|-----|--|---------------------------|--|---|--|--|--|--|--|--|--|--|
| | (a) b/ ₂ | (b) $b/_{3}$ | (c) b/ ₄ | (d) $b/_6$ | | | | | | | | |
| 18. | Two principal plan | es are located at | an angle of | | | | | | | | | |
| | (a) 180° | (b) 45° | (c) 90° | (d) 135° | | | | | | | | |
| 19. | When two equal of | pposite forces ap | plied to a body te | nd to elongate it, the stress | | | | | | | | |
| | so produced is call | ed | • | | | | | | | | | |
| | (a) shear stress | | (b) comp | ressive stress | | | | | | | | |
| | (c) tensile stress | | (d) trans | verse stress | | | | | | | | |
| 20. | The relationship be | tween bulk mod | ulus K, modulus of | elasicity E and modulus of | | | | | | | | |
| | rigidity G is | | | | | | | | | | | |
| | (a) $E = \frac{6KG}{(2K+G)}$ | (b) $E = \frac{9}{(2k)}$ | $\frac{KG}{(G-G)}$ (c) $E = \frac{1}{G}$ | $\frac{6KG}{3K-G}$ (d) $E = \frac{9KG}{(3K+G)}$ | | | | | | | | |
| 21 | | | (| () | | | | | | | | |
| 41. | 21. The centre of gravity of the solid hemisphere of radius r is located from the flat | | | | | | | | | | | |
| | base of hemisphere along the centreline at a distance of | | | | | | | | | | | |
| | (a) $\frac{r}{2}$ | (b) $\frac{3r}{g}$ | (c) $\frac{4r}{2-}$ | (d) $\frac{4r}{5}$ | | | | | | | | |
| 22. | 2 | O | , 311 | . 5 | | | | | | | | |
| 22. | area A at the neutr | | to shear force r | on a rectangular section of | | | | | | | | |
| | | | 775 an | 2.0 | | | | | | | | |
| | (a) $\frac{3F}{2A}$ | (b) $\frac{F}{2A}$ | (c) $\frac{2F}{3A}$ | (d) $\frac{2F}{\Delta}$ | | | | | | | | |
| 23. | ZA | 211 | JAX . | A | | | | | | | | |
| 20. | Moment of inertia $(d = 2r, mass = m)$ | | nai disc about any | of its diameter is | | | | | | | | |
| | | | | | | | | | | | | |
| | (a) Mr ² | (b) $\frac{Mr^2}{2}$ | (c) $\frac{Mr^2}{12}$ | (d) $\frac{Mr^2}{4}$ | | | | | | | | |
| | (a) Mr ² Explanation: 23. (| 2 | Mr^2 12 | 4 | | | | | | | | |
| | Explanation: 23. (| d) M.I. about dia | $meter = {4}$ | | | | | | | | | |
| | M.I. perpendicular to | o plane = $\frac{Mr^2}{}$ | | | | | | | | | | |
| | · · · · · · · · · · · · · · · · · · · | 2 | | | | | | | | | | |
| | | · : AN | ISWERS: | | | | | | | | | |
| | 1 (a) 2 | | | 5 (.) | | | | | | | | |
| | 1. (a) 2. | (d) 3. | (d) 4. (c) | | | | | | | | | |
| | 6. (b) 7. | (b) 8. | (b) 9. (c) | • • | | | | | | | | |
| | 11. (b) 12. 16. (a) 17. | (a) 13. | (b) 14. (c) | | | | | | | | | |
| | ` ' | (d) 18. | (c) 19. (c) | 20. (d) | | | | | | | | |
| | 21. (b) 22. | (a) 23. | (d) | | | | | | | | | |
| | | * | *** | | | | | | | | | |

The pin jointed frame shown in fig. is



- FIG. Q-9

- perfect frame (a)
- (b) redundant frame
- (c) deficient frame
- (d) none of these

10. The force in member BD is,

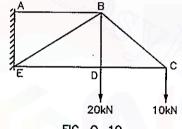


FIG. Q-10

- 20 kN (comp.)
- (b) 20 kN (Tension
- 30 kN (tension)
- (d) zero

11. A pin jointed cantilever truss is shown in fig. The force in member ED is,

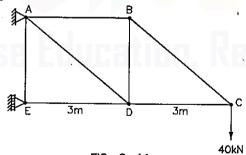


FIG. Q-11

- (a) 40 kN (C)
- (b) 80 kN (T)
- (c) 80 kN (C)
- (d) 120 kN (C)

(Civil / services)

- (a) 5 t, tensile
- (b) zero
- (c) 2.88 t, compressive
- (d) 5 t, compressive

12. Force in member BC of the truss shown in fig. is

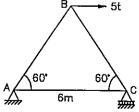
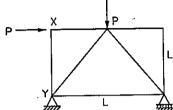


FIG. Q-12

13. A truss is shown in fig. below. The cross sectional area of each member is 'A' and modulus of elasticity of the material is 'E'. The strain energy in the member 'XY' is given by (IES)

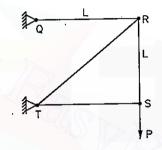


(a) $\frac{P^2L}{2AE}$ (b) $\frac{P^2L}{6AE}$

(c) $\frac{P^2L}{3AE}$ (d) zero

FIG. Q-13 14. A truss is shown in fig. The force in member QR is

(GATE 2010)

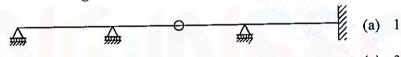


(a) zero

(b)
$$P/\sqrt{2}$$

(d)
$$\sqrt{2}$$
 P

FIG. Q-14
15. What is the degree of static indeterminacy of the beam given below?



(b)

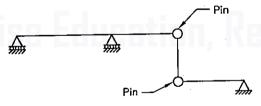
(c) 3

(d) 4

2

16. What is the degree of static indeterminacy of the beam given below?

FIG. Q-15



(a) 0

(b) 1

(c) 2

FIG. Q-16

(d) 3

17. What is SI for the truss shown below?

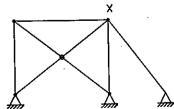


FIG. Q-17

(a) 1

(b) 2

(c) 3

(d) 0

18. Match list I with list II and select correct answer using the codes given below the lists:

(Civil Services)

List I (structure)

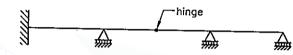
List II (S.I.)

A.



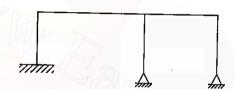
1. three

В.



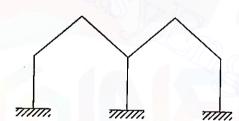
2. Six

C.



3. two

D.



4. Four

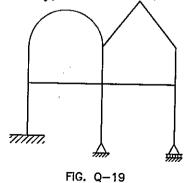
Codes:

A

В

- C D
- (a) 1
- 2
- (b) 3 (c) 3
- 2
- 3 1
- 4
- (d) 1
- 4 2

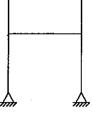
19. For a plane frame shown in figure below static and kinematic degrees of indeterminacy are respectively, (IES)



- (a) 12 and 27
- (b) 12 and 24
- (c) 9 and 24
- (d) 9 and 27

Objective Civil Eng. \ 2016 \ 20

20. What is degree of kinematic indeterminacy of the frame shown below if axial deformation is neglected?

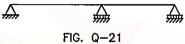


- (a) 6
- (b) 8
- (c) 10

FIG. Q-20

(d) 12

21. What is KI of the beam shown in fig.

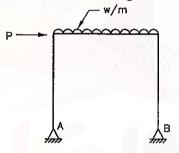


- (a) 2
- (b) 3

- (c) 4
- (d) 5

22. The frame shown in fig. has

(Civil Services) one unknown reaction component



- two unknown reaction components
- Three unknown reaction components
- Six unknown reaction components

23. The degree of static indeterminacy of the rigid frame having two internal hinges as shown in the figure below is (GATE 2008)

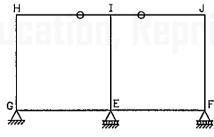
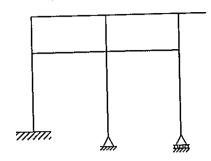


FIG. Q-23

- (b)
- (c)
- (d) 5

24. For the plane frame with an overhang as shown below, assuming negligible axial deformation, the degree of static indeterminacy d, and the degree of kinematic indeterminacy, k are (GATE 2004)

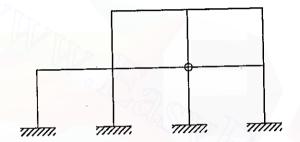
Structural Analysis



- (a) d = 3, d = 10
- (b) d = 3, k = 13
- (c) d = 9, k = 10
- (d) d = 9, k = 13

FIG. Q-24

25. Statical indeterminacy of the given 2D frame is



- (a) 10
- (b) 11
- (c) 12
- (d) 13

FIG. Q-25

- 26. The fixed support in a real beam becomes in the conjugate beam a
 - (a) fixed support
- (b) hinged support (c) roller support
- (d) free support
- 27. The theorem of three moments is applicable only when
 - (a) the beam is prismatic
 - (b) the spans are equal
 - (c) there is no discontinuity such as hinges within the span
 - (d) there are atleast 2 spans
- 28. The fixed end of a continuous beam in Clapeyron's equation is replaced by an addition of span of length
 - (a) zero length

- (b) infinite length
- (c) equal to other span lengths
- (d) none of the above
- 29. The conjugate beam method falls in the category of
 - (a) force method

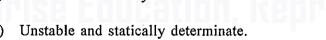
(b) displacement method

(c) Stiffness method

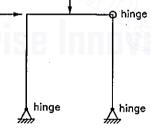
- (d) none of these
- 30. In the moment area method, the difference in slope between two sections of a beam is equal to the

- (a) area of $\frac{M}{EI}$ diagram between these two sections.
- (b) moment of $\frac{M}{EI}$ diagram between these two sections.
- (c) $\frac{1}{2}$ x area of $\frac{M}{EI}$ diagram between these two sections.
- (d) $\frac{1}{2}$ moment of $\frac{M}{EI}$ diagram between these two sections.
- 31. In the moment area method, the deflection of point A from a tangent at B is equal to
 - (a) area of $\frac{M}{EI}$ diagram between A and B.
 - (b) Moment of $\frac{M}{EI}$ diagram between A and B about A.
 - (c) Moment of $\frac{M}{EI}$ diagram between A and B about B.
 - (d) $\frac{1}{2}$ × area of $\frac{M}{EI}$ diagram between A and B.
- 32. Shear force at any section in a conjugate beam gives in the actual beam.
 - (a) slope
- (b) deflection
- (c) curvature
- (d) none of these
- 33. Bending moment at any section in a conjugate beam gives in the actual beam.
 - (a) slope
- (b) deflection
- (c) curvature
- (d) none of these

- 34. The plane frame shown in fig. is
 - (a) Stable and statically determinate.



- (c) Stable and statically indeterminate.
- (d) unstable and statically indeterminate.



- FIG. Q-34
- 35. The unit load method used in structural analysis is
- (GATE 2004)
- (a) applicable only to statically determinate structures.
- (b) another name of stiffness method.
- (c) an extension of Maxwell's reciprocal theorem.
- (d) derived from Castigliano's theorem.

43. For a propped cantilever beam with u.d.l. on entire span, B.M. at fixed and is

(a)
$$\frac{wl^2}{2}$$

(b)
$$\frac{wl^2}{4}$$

(c)
$$\frac{wl^2}{8}$$

(d)
$$\frac{wl^2}{12}$$

44. Strain energy stored in a bar due to axial load P is

(a)
$$\frac{P^2L}{AE}$$

(b)
$$\frac{P^2L}{2AE}$$

(c)
$$\frac{PL}{2AE}$$

(d)
$$\frac{P^2L}{4AE}$$

45. The equation for strain energy due to bending is given by

(a)
$$U = \int_{0}^{L} \frac{M^2L}{2EI} \cdot dx$$

(b)
$$U = \int_{0}^{L} \frac{M^2}{EI} dx$$

$$U = \int_{0}^{L} \frac{M^{2}L}{2EI} \cdot dx \quad (b) \quad U = \int_{0}^{L} \frac{M^{2}}{EI} dx \quad (e) \quad U = \int_{0}^{L} \frac{M^{2}}{2EI} dx \quad (d) \quad U = \int_{0}^{L} \frac{M}{2EI} dx$$

(d)
$$U = \int_{0}^{L} \frac{M}{2EI} dx$$

46. According to Castigliano's first theorem partial derivative of strain energy U w.r.t. force P will give

(a) slope

(b) deflection

(c) moment

(d) Shear force

47. According to Castigliano's first theorem partial derivative of strain energy U w.r.t. moment M will give

(a) slope

(b) deflection

(c) Shear force

(d) curvature

48. The fictitious method of finding slope and deflection of beam is

unit load method.

(b) Castigliano's first theorem.

castigliano's second theorem.

consistent deformation method.

49. The principal of least work is

Castigliano's first theorem

(b) Castigliano's second theorem

principle of superposition

(d) Muller Breslan principle

50. Fixed end moment for a beam with uniformly distributed load on entire span is

(a)
$$\frac{Wl^2}{8}$$

(b)
$$\frac{wl^2}{12}$$

(c)
$$\frac{wl^2}{16}$$

(d)
$$\frac{wl^2}{24}$$

51. For a beam loaded as shown in fig. fixed end moment at A is

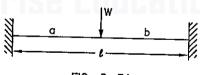


FIG. Q-51

(c) $\frac{\text{Wba}^2}{I^2}$

(d) $\frac{Wa^2b^2}{r^2}$

52. For a beam loaded as shown in fig. fixed end moment at B is

(b) $\frac{wl^2}{20}$

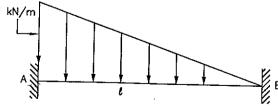


FIG. Q-52

Structural Analysis

| 53. | For a | fixed | beam | with | central | point | load | fixed | end | moment | at | each | end | will | be |
|-----|-------|-------|------|------|---------|-------|------|-------|-----|--------|----|------|-----|------|----|
|-----|-------|-------|------|------|---------|-------|------|-------|-----|--------|----|------|-----|------|----|

- (a) $\frac{Wl}{4}$
- (b) $\frac{Wl^2}{4}$
- (c) $\frac{Wl^2}{8}$
- (d) $\frac{Wl}{8}$

54. At the fixed end support slope will be

- (a) zero
- (b) maximum
- (c) minimum

55. Moment required to produce unit rotation is called

- (a) Flexibility
- (b) stiffness
- (c) rigidity
- (d) none of these

56. The stiffness value k, for a beam with far end fixed is

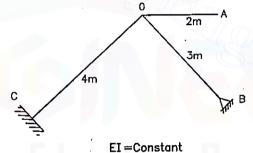
- (a) $\frac{2EI}{L}$
- (b) $\frac{3EI}{L}$
- (c) $\frac{4EI}{L}$
- (d) $\frac{6EI}{L}$

- (a) $\frac{2EI}{L}$
- (b) $\frac{3EI}{L}$
- (c) $\frac{4EI}{L}$
- (d) $\frac{6EI}{L}$

58. For a beam with far end fixed, carry over moment will be

- (a) $\frac{2EI}{L}$
- (b) $\frac{3EI}{L}$
- (c) $\frac{4EI}{L}$
- (d) $\frac{6EI}{L}$

59. The distribution factor for member OB shown in fig. is



- (a) 1
- (b) 0.33
- (c) 0.5
- (d) 0

FIG. Q-59

60. A moment of 6 kN-m is acting at joint O. The relative stiffness of member OA, OB,

OC are $\frac{1}{6}, \frac{1}{3}$ and $\frac{1}{2}$ respectively, the moment resisted by member OB is

- (a) 1.0 kN.m
- (b) 2 kN.m
- (c) 3 kN.m
- (d) 4 kN.m

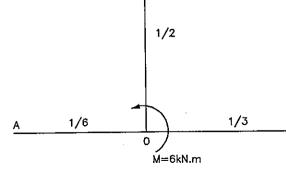


FIG. Q--60

61. Fixed end moment M_{fAB} for a given beam is

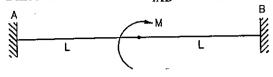


FIG. Q-61

- (a) zero
- (b) $\frac{M}{2}$

62. If a moment M is applied to the hinged end of a prismatic propped cantilever beam, then moment at the fixed end will be

- (a) M
- (b) $\frac{M}{2}$ (c)
- (d) $\frac{M}{4}$

63. The sum of distribution factors for moment at any joint is

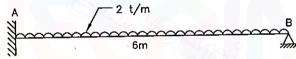
- (b) 0.5
- (c) 1.0
- (d) 1.5

64. The ratio of stiffness of a beam at the near end when far end is hinged to the stiffness when far end is fixed is

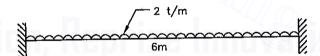
65. Consider the propped cantilever shown in fig. (a) and fixed beam shown in fig. (b). Consider the following statements:

Statement - I: Fixed end moment at A for propped cantilever is 12 t.m Statement - II: Fixed end moment at A of fixed beam is 6 t.m (Civil Services) of these statements:

both I and II are false



- I is correct but II is false
- both I and II are correct



I is false but II is correct

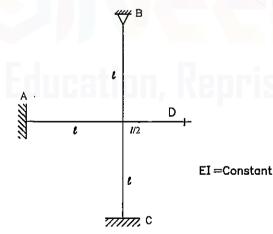
66. A fixed beam AB is subjected to triangular load varying from zero at A to w per unit length at end B. The ratio of fixed end moment at B to A will be

(Civil Services)

- (b) $\frac{1}{3}$

67. For a fixed beam AB, support B sink by δ downwards. The moment produced at end B will be

- (a) $\frac{6EI\delta}{L^2}$ anticlockwise
- (b) $\frac{6EI\delta}{I^2}$ clockwise
- (c) $\frac{12EI\delta}{L^3}$ anticlockwise
- (d) $\frac{12EI\delta}{I^3}$ clockwise
- 68. For a propped cantilever beam AB, with end B fixed, sink down by δ. The reaction produced at A will be
 - (a) $\frac{3EI\delta}{L^3}$ downward (b) $\frac{3EI\delta}{L^3}$ upward (c) $\frac{12EI\delta}{L^3}$ downward (d) $\frac{12EI\delta}{L^3}$ upward
- 69. For a fixed beam AB, if end B rotate through θ , anticlockwise, reaction at A will be
 - (a) $\frac{12EI\theta}{L^2}$ upward (b) $\frac{12EI\theta}{L^2}$ downward (c) $\frac{6EI\theta}{L^2}$ upward (d) $\frac{6EI\theta}{L^2}$ downward
- 70. When a beam is subjected to uniform rise of temperature (t), axial compressive force produced at the ends will be
 - (a) αtE
- (b) $\alpha t \to A$
- (c) $\frac{\alpha tE}{A}$
- (d) $\frac{\alpha tA}{E}$
- 71. The strain energy stored in a simply supported beam of span *l* and subjected to central point load W is
 - (a) $\frac{W^2l^3}{48FI}$
- (b) $\frac{W^2 l^2}{48EI}$
- (c) $\frac{W^2 l^2}{96EI}$
- (d) $\frac{W^2 l^3}{96EI}$
- 72. A steel frame is shown in fig. below. If joint O of the frame is rigid, the rotational stiffness of the frame at point O is given by (IES)



(a) $\frac{11E1}{I}$

(b) $\frac{10EI}{l}$

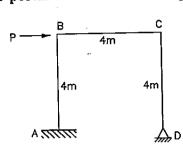
(c) $\frac{8EI}{l}$

(d) $\frac{6EI}{I}$

FIG. Q-72

Objective Civil Eng. \ 2016 \ 21

73. For the portal frame shown in fig. below the shear equation is (IES)



(a)
$$\frac{M_{BC} + M_{CB}}{4} + P = 0$$

(b)
$$\frac{M_{BA} + M_{BC}}{4} + P = 0$$

(b)
$$\frac{M_{BA} + M_{BC}}{4} + P = 0$$
(c)
$$\frac{M_{BA} + M_{BC}}{4} + \frac{M_{CD}}{4} + P = 0$$

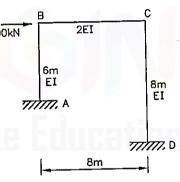
(d)
$$\frac{M_{CD}}{4} + P = 0$$

- 74. A fixed beam of uniform section is carrying a point load at its mid-span. If moment of inertia of the middle half length is now reduced to half its previous value, then (IES) fixed end moments will
 - increase (a)

(b) decrease

remain constant (c)

- (d) change their direction
- 75. Due to some point load any where on a fixed beam the maximum free bending (IES) moment is M. The sum of fixed end moments is
 - M (a)
- (b) 1.5 M
- (c) 2.0 M
- (d) 3.0 M
- 76. The slope deflection equation at the end B of member BC for the frame shown in fig. below will be



- (a) $M_{BC} = \frac{4EI}{g} (2\theta c \theta_B)$
- (b) $M_{BC} = \frac{4EI}{8} (2\theta_B \theta_C)$
- (c) $M_{BC} = \frac{4EI}{8} (2\theta_B + \theta_C)$

(d)
$$M_{BC} = \frac{4EI}{8} (\theta_B + \theta_C)$$

77. Where the concentrated load 'W' should be kept on simply supported beam AB so that $R_A = 2R_B$?

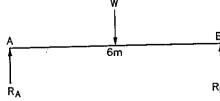


FIG. Q-77

- (a) 2 m from A
- (b) 1.5 m from A
- (c) 1.5 m from B
- (d) 2 m from B

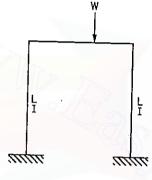
78. The moment distribution method is

- (a) an iterative method
- (b) an exact method
- (c) an approximate method
- (d) none of these

79. If M is the external moment which rotates the near end of a prismatic beam without translation, the far end being fixed, then the moment induced at the far end is

(a) zero

- (b) $\frac{M}{2}$ in the same direction
- (c) $\frac{M}{2}$ in the opposite direction of M (d) None of the above
- 80. The portal frame shown below will



- (a) not sway
- (b) sway towards left
- (c) sway towards right
- (d) sway either to left or right

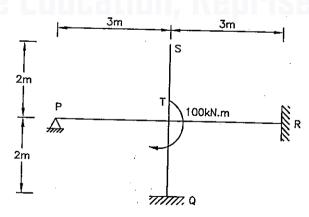
FIG. Q-80

81. The carry over factor for prismatic member with far end fixed is

- (a) $\frac{-1}{2}$
- (b) $\frac{1}{2}$
- (c) $\frac{-1}{4}$
- (d) $\frac{1}{4}$

82. All members in the rigid jointed frame are prismatic and have the same flexural stiffness EI. Find the magnitude of the bending moment at Q (in kN-m) due to given loading

(GATE 2013)



- (a) 100 kN.m
- (b) 50 kN.m
- (c) 25 kN.m
- (d) 20 kN.m

FIG. Q-82

83. A three hinged parabolic arch having span l, and central rise r, subjected to u.d.l. on entire span,

horizontal thrust is $\frac{wl^2}{8r}$

(b) S.F. will be zero throughout

B.M. will be zero throughout

(d) all the above

84. A three hinged arch is generally hinged at its supports and

at one quarter span

(b) at the crown

any where in the rib (c)

(d) none of these

85. The equation of parabolic arch of span l and rise r, is given by

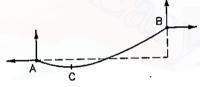
(a)
$$y = \frac{2r}{L^2} x (L - x)$$

(b)
$$y = \frac{4r}{L^2} x (L - x)$$

(c)
$$y = \frac{3r}{L^2} x (L - x)$$

(d)
$$y = \frac{r}{L^2} x (L - x)$$

86. In the cable shown in fig. The minimum tension occur at



(a) A

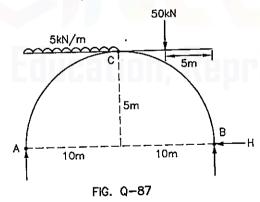
(b) B

C (c)

FIG. Q-86

(d) between A and C

87. A three hinged parabolic arch of span 20 m and rise 5 m is loaded as shown in fig. The horizontal thrust H, is



(a) 50 kN

(b) 75 kN

(c) 100 kN

(d) 150 kN

88. A cable of span l and central dip d is subjected to uniform load w per unit horizontal length. The horizontal component of tension in the cable is

(b) $\frac{wl^2}{8d}$ (c) $\frac{wl^2}{12d}$

(d) $\frac{wl^2}{16d}$

| 89. | Length of parabolic cable of span l ar | | | |
|------|--|--------|------------------------|----------------------------|
| | (a) $l + \frac{d^2}{3l}$ (b) $l + \frac{4d^2}{3l}$ | (c) | $l + \frac{8d^2}{3l}$ | (d) $l + \frac{16d^2}{3l}$ |
| 90. | Shape of cable suspended between two | poi | nts is | 31 |
| | (a) parabolic (b) Catenary | (c) | Circular (d) de | epends upon loads |
| 91. | The cable resists external loads by | | | |
| | (a) tension | (b) | compression | |
| | (c) bending | (d) | compression and b | |
| 92. | The shape of cable under horizontal u | nifor | mly distributed lo | ad is |
| | (a) parabolic (b) catenary | (c) | circular | (d) triangular |
| 93. | An arch resist the external load by | | | _ |
| | (a) normal thrust | (b) | normal thrust and | B.M. |
| | (c) radial shear and B.M. | (d) | normal thrust, radi | al shear and B.M. |
| 94. | A line of thrust of a parabolic arch is | | | |
| | (a) Parabolic (b) Funicular poly | gon | (c) triangular | (d) circular |
| 95. | In case of a simply supported rectang | | | |
| | central load W, the length of elasto-pl | astic | zone of the plastic | hinge is |
| | (a) $\frac{L}{2}$ (b) $\frac{L}{3}$ | | | |
| | _ | | / T | (d) $\frac{L}{5}$ |
| 96. | In plastic analysis, the shape factor for | rect | tangular section is | |
| | (a) 1.7 (b) 1.6 | (c) | | (d) 1.4 |
| 97. | In plastic analysis, the shape factor for | circ | ular section is | |
| | (a) 1.5 (b) 1.6 | (c) | 1.7 | (d) 1.8 |
| 98. | In plastic analysis, the shape factor for | · tria | ngular section is | |
| | (a) 1.5 (b) 2.34 | (c) | | (d) 2.5 |
| 99. | The shape factor of standard rolled be | am s | ection varies from | nnnisätir |
| | (a) 1.10 to 1.20 (b) 1.20 to 1.30 | (c) | 1.30 to 1.40 | (d) 1.40 to 1.50 |
| 100. | Pick up the correct statement from the | | | • |
| | (a) In a loaded beam the moment at whi | | | called vield moment |
| | (b) In a loaded beam the moment at wh | nich t | he entire section of | f the beam become |
| | fully plastic, is called plastic momen | ıt. | | |
| | (c) In a fully plastic stage of beam, the neut | | is divides the section | in to two equal areas |
| | (d) all the above. | | | , and the order areas |
| 101. | The shape factor is the ratio of | | | |
| | (a) Mp and My (b) My and Mp | (c) | Ze and Zn | (d) None of these |
| 102. | If Q is load factor, S is shape factor and | l F is | the factor of safe | ty in electic design |
| | the following is correct. | ^_ | | ij in clastic design, |
| | (a) $Q = S + F$ (b) $Q = S - F$ | (c) | O = F - S | (d) $Q = F \times S$ |
| | (-) 2 2 | (-) | × 1 0 | (u) (v - r ^ 5 |

| | | | | Opjectiv | e Civil Engineering |
|------|------|---|-----------------|-----------------------|---------------------------|
| 103. | In t | he approximate analysis of frame | s under | lateral loads, the pe | oint of contraflexur |
| | | he beams and columns are assur | | , , | |
| | (a) | $\frac{L}{2}$ for beam and $\frac{2}{3}$ H from bas | e for co | olumn | |
| | | $\frac{L}{2}$ for beam and $\frac{H}{3}$ from base | | | |
| | (c) | $\frac{L}{10}$ for beam and $\frac{H}{2}$ from base | for colu | umn | |
| | (d) | $\frac{L}{2}$ for beam and $\frac{H}{2}$ from base | for colu | mn | |
| 104. | Moi | ment distribution method is best | suited | for | |
| | (a) | Indeterminate pin jointed truss | | (b) Rigid frames | |
| | (c) | Space frame | | (d) Composite stru | ıcture |
| 105. | Tota | al reactions in 3D space frame a | t fixed | | |
| | (a) | | | zero | (d) 6 |
| 106. | For | a simply supported beam, of sp | an <i>l</i> the | ordinate for B.M. | ` ' |
| | | support is | | | |
| | (a) | $\frac{x}{L}$ (b) $\frac{L-x}{L}$ | (c) | $\frac{x(L-x)}{L}$ | (d) x (L – x) |
| 107. | B.M | . and slope at the end of fixed | suppor | t of a fixed beam | loaded by u.d.l. is |
| | | and respectively. | | | |
| | (a) | Minimum, zero (b) Maximum, z | zero (c) | zero, maximum | (d) zero, minimum |
| 108. | STA | AD uses method of anal | ysis for | plate elements. | |
| | (a) | Finite element method | (b) | Finite difference m | ethod |
| | (c) | Stiffness method | (d) | Flexibility method | |
| 109. | In a | two span continuous beam load | ed by I | UDL, point of conti | raflexure exist |
| | (a) | At mid support | (b) | In both spans near | middle support |
| | (c) | near end support | (d) | At the end support | |
| 110. | For | a fixed beam with span 'L' havi | ng plas | tic moment capacit | y Mp, the ultimate |
| | cent | ral concentrated load will be | | • | |
| (| (a) | $\frac{4M_{\rm P}}{L}$ (b) $\frac{6M_{\rm P}}{L}$ | (c) | $\frac{8M_{P}}{L}$ | (d) $\frac{11.7M_{P}}{L}$ |
| | | a fixed beam with span 'L' ha | | | - |
| | | ipse u.d.l. will be | J . | | ¥ , |
| | | 6M ₂ 8M ₋ | | 121/1 | 16M |

112. For a propped cantilever beam with u.d.l. on entire span, plastic moment capacity will be

(a)
$$\frac{wl^2}{8}$$

(b)
$$\frac{wl^2}{11.656}$$
 (c) $\frac{wl^2}{12}$

(c)
$$\frac{wl^2}{12}$$

(d)
$$\frac{wl^2}{16}$$

113. For a simply supported beam of span 16 m, the maximum B.M. at section 4 m from left support, when u.d.l. 2 kN/m longer than span crosses the girder from left to right is

- (a) 12 kN.m
- (b) 24 kN.m
- (c) 48 kN.m
- (d) 96 kN.m

: ANSWERS:

| 1. (a) | 2. (c) | 3. (a) | 4. (b) | 5. (c) |
|----------|----------|----------|----------|----------|
| 6. (b) | 7. (a) | 8. (a) | 9. (a) | 10. (b) |
| 11. (c) | 12. (d) | 13. (d) | 14. (c) | 15. (b) |
| 16. (a) | 17. (b) | 18. (d) | 19. (c) | 20. (b) |
| 21. (d) | 22. (d) | 23. (d) | 24. (d) | 25. (c) |
| 26. (d) | 27. (c) | 28. (a) | 29. (a) | 30. (a) |
| 31. (b) | 32. (a) | 33. (b) | 34. (a) | 35. (d) |
| 36. (a) | 37. (c) | 38. (c) | 39. (c) | 40. (d) |
| 41. (d) | 42. (b) | 43. (c) | 44. (b) | 45. (c) |
| 46. (b) | 47. (a) | 48. (b) | 49. (b) | 50. (b) |
| 51. (b) | 52. (c) | 53. (d) | 54. (a) | 55. (b) |
| 56. (c) | 57. (b) | 58. (a) | 59. (c) | 60. (b) |
| 61. (c) | 62. (b) | 63. (c) | 64. (b) | 65. (d) |
| 66. (d) | 67. (a) | 68. (b) | 69. (c) | 70. (b) |
| 71. (d) | 72. (a) | 73. (c) | 74. (a) | 75. (a) |
| 76. (c) | 77. (a) | 78. (a) | 79. (b) | 80. (c) |
| 81. (b) | 82. (c) | 83. (d) | 84. (b) | 85. (b) |
| 86. (c) | 87. (a) | 88. (b) | 89. (c) | 90. (d) |
| 91. (a) | 92. (a) | 93. (d) | 94. (b) | 95. (b) |
| 96. (c) | 97. (c) | 98. (b) | 99. (a) | 100. (d) |
| 101. (a) | 102. (d) | 103. (d) | 104. (b) | 105. (d) |
| 106. (c) | 107. (b) | 108. (a) | 109. (b) | 110. (c) |
| 111. (d) | 112. (b) | 113. (c) | | |
| | | | | |

EXPLANATIONS

7. (a) At joint Q, $\Sigma V = 0,$ $\therefore F = RQ \sin 45^{\circ} = \frac{RQ}{\sqrt{2}} \qquad \therefore RQ = \sqrt{2} F$ $\Sigma H = 0,$

RQ cos 45° = PQ

$$\sqrt{2} \text{ F} \times \frac{1}{\sqrt{2}} = \text{PQ}$$
 \therefore PQ = F (comp.)

8. (a) $V_B = 0$ and $V_A = W$ Joint A: $\Sigma H = 0$, \therefore $F_{AB} = 0$ $\Sigma V = 0$, \therefore $F_{AC} = W$ (comp.) Joint B: $\Sigma H = 0$, $F_{BC} \cos 30^\circ = F_{AB}$ $F_{BC} \cos 30^\circ = 0$ \therefore $F_{BC} = 0$

9. (a)
$$m = 9$$

 $j = 6$ $m = 2j - r$
 $r = 3$ $= 2 \times 6 - 3$
 $= 9$ \therefore Perfect frame

10. (b) At joint D,

$$\Sigma V = 0$$
, $\therefore F_{BD} = 20 \text{ kN (T)}$

11. (c) Taking moment @ A $H_E \times 3 = 40 \times 6$ $\therefore H_E = 80 \text{ kN}$ At joint E, $\Sigma H = 0$ $\therefore F_{ED} = 80 \text{ kN (C)}$

12. (d)
$$\tan 60^{\circ} \frac{BD}{3}$$

 \therefore BD = 5.196 m ... vertical height

taking moment @ A, $R_C \times 6 = 5 \times 5.196$ $R_C = 4.33 \text{ t}$ at joint C, $\Sigma V = 0$,

$$F_{BC} \sin 60^{\circ} = R_{C}$$

$$F_{BC} = \frac{4.33}{\sin 60^{\circ}} = 5 \text{ t (compressive)}$$

13. (d) Consider Joint x,

$$\Sigma V = 0$$
, $\therefore Fxy = 0$

Since force in member XY is zero,

strain energy is also zero

14. (c) Taking moment @ T

$$R_O \times L = P \times L$$

$$\therefore R_O = P$$

Force in member QR = P (Tensile)

15. (b)
$$D_S = SI = (1 + 1 + 1 + 3) - 4$$

= 2

Internal hinge provide one extra condition

16. (a)
$$D_S = SI = (2 + 1 + 2) - 5 = 0$$

two extra conditions available at each pin

17. (b)
$$D_{se} = R - r$$
 $R = 6$
= 6 - 4 $r = 3 + 1 = 4$... one extra condition at X
= 2

$$D_{si} = (m + r) - 2j$$

= $(8 + 4) - 2 \times 6$
= 0

$$D_{s} = D_{se} + D_{si}$$
= 2 + 0 = 2

$$SI = (3 + 3) - 3 = 3$$

$$SI = (3 + 1 + 1 + 1) - 4 = 2$$

$$D_{se} = R - r$$

$$= 7 - 3 = 4$$
 : $D_s = 4 + 0 = 4$

$$D_{si} = 3C = 0$$

For D:

$$D_{se} = R - r$$
$$= 9 - 3$$

$$D_s = 6 + 0 = 6$$

$$D_{si} = 3C = 0$$

$$D_{se} = R - r$$

= $(3 + 2 + 1) - 3$

$$D_{si} = 3 C$$
$$= 3 \times 2$$

$$= 3$$

$$D_s = 3 + 6 = 9$$

$$D_k = 3j - R$$

$$= 3 \times 10 - 6$$

Objective Civil Eng. \ 2016 \ 22

Objective Civil Engineering

170

20. (b)
$$D_{k} = 3j - R$$

$$= 3 \times 6 - 4$$

$$= 14$$

$$= 14$$

$$= 8$$
21. (d)
$$D_{k} = 3j - R$$

$$= 3 \times 3 - 4$$

$$= 5$$

22. (d) Six unknown reaction components

$$R_A$$
, H_A , M_A and R_B , H_B , M_B

23. (d)
$$D_{se} = R - r$$
 $D_{si} = 3C - rr$ $D_{se} = 4 - 3 = 1$ $rr = (2 - 1) + (2 - 1) = 2$ $D_{si} = 3 \times 2 - 2 = 4$

$$D_s = D_{se} + D_{si}$$

= 1 + 4 = 5

24. (d)
$$D_{se} = R - r$$
 $D_{si} = 3 C$
 $= 6 - 3 = 3$ $= 3 \times 2 = 6$
 $\therefore D_s = 3 + 6 = 9$
 $D_{knad} = (3j - R) - m$ axial deformation is neglected
 $= (3 \times 10 - 6) - 11$
 $= 13$

25. (c)
$$D_{se} = R - r$$

 $= (12) - 3 = 9$
 $D_{si} = 3C - rr$
 $= 3 \times 2 - 3$
 $= 3$

$$D_{s} = 9 + 3 = 12$$
34. (a) $D_{se} = R - r$ $D_{si} = 3C = 0$

$$= 4 - 4$$

$$= 0$$

: Stable and determinate

| 59. (c) | Member | k | Σk | $DF = \frac{k}{\Sigma k}$ |
|---------|--------|----------------------|----------|---------------------------|
| | OA | 0 | | 0. |
| | ОВ | $\frac{3EI}{3} = EI$ | 2EI | 0.5 |
| | ОС | $\frac{4EI}{4} = EI$ | <i>,</i> | 0.5 |

Structural Analysis

| - | _ | - |
|---|---|---|
| - | • | 7 |
| - | | _ |
| | | |

| 60. | (b) | Member | k | Σk | $DF = \frac{\bar{k}}{\Sigma k}$ |
|-----|-----|--------|-----|------------|---------------------------------|
| | · | OA | 1/6 | | 0.166 |
| | | OB | 1/3 | 1.0 | 0.333 |
| | | OC | 1/2 | | 0.50 |

$$M_{OB} = M \times D.F.$$

= 6 × 0.333
= 2.0 kN.m

65. (d) For propped cantilever beam

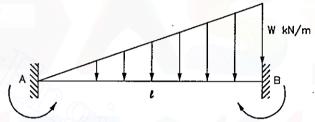
$$M_{fAB} = \frac{wl^2}{8} = \frac{2 \times 6^2}{8} = 9 \text{ kN.m}$$

For fixed beam

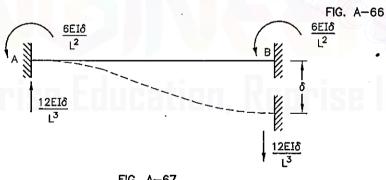
$$M_{fAB} = \frac{wl^2}{12} = \frac{2 \times 6^2}{12} = 6 \text{ kN.m}$$

66. (d)

$$\frac{M_{\rm fBA}}{M_{\rm fAB}} = \frac{wl^2/20}{wl^2/30} = \frac{3}{2}$$



67. (a)



68. (b)

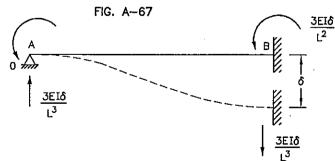


FIG. A-68

172

69. (c)

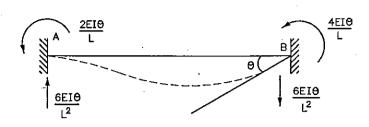


FIG. A-69

1/2

FIG. A-71

71. (d) For portion AC

$$M_{x} = \frac{W}{2} \cdot x$$

Strain energy stored in AC,

$$= \int_{0}^{1/2} \frac{M^{2}}{2EI} dx = \int_{0}^{1/2} \left(\frac{W}{2}x\right)^{2} 2EI dx$$

$$= \int_{0}^{1/2} \frac{W^{2}x^{2}}{8EI} dx = \frac{W^{2}}{8EI} \left[\frac{x^{3}}{3} \right]_{0}^{1/2} = \frac{W^{2}}{8EI} \left[\frac{l^{3}}{24} \right] = \frac{W^{2}l^{3}}{192EI}$$

.. Total energy stored in entire beam

$$= 2 \times \frac{W^2 l^3}{192EI} = \frac{W^2 l^3}{96EI}$$

72. (a) Total stiffness =
$$\frac{4EI}{l} + \frac{3EI}{l} + 0 + \frac{4EI}{l}$$

$$= \frac{11EI}{l}$$

75. (a) Maxi. free B.M. =
$$M = \frac{Wab}{l}$$

Sum of fixed end moments =
$$\frac{\text{Wab}^2}{l^2} + \frac{\text{Wba}^2}{l^2}$$

= $\frac{\text{Wab}}{l}$
= M

76. (c)
$$M_{BC} = M_{fBC} + \frac{2EI}{l} (2\theta_B + \theta_C - \frac{3\delta}{l})$$

 $M_{fBC} = 0, \quad \delta = 0$
 $\therefore M_{BC} = \frac{2E(2I)}{8} (2\theta_B + \theta_C) = \frac{4EI}{8} (2\theta_B + \theta_C)$

Structural Analysis

173

77. (a)
$$R_B \times 6 = W.x$$
 ... (i) $R_A + R_B = W$ $2R_B \times R_B = W$ $3R_B = W$... (ii) From equation (i) $R_B \times 6 = 3R_B x$

 \therefore x = 2 m from A

82. (c) Member k
$$\Sigma k$$
 DF = $\frac{k}{\Sigma k}$

TP $\frac{3EI}{3} = EI$ 0.25

TQ $\frac{4EI}{2} = 2EI$ 4EI 0.50

TR $\frac{4EI}{4} = EI$ 0.25

 \therefore M_{TQ} = D.F. × M
= 0.5 × 100 = 50 kN.m

 $M_{QT} = \frac{50}{2} = 25 \text{ kN.m (half moment is carried over to Q)}$

87. (a) Taking moment @ A $V_{B} \times 20 = (5 \times 10 \times 5) + (50 \times 15)$ $\therefore V_{B} = 50 \text{ kN}$ Taking moment @ C $V_{B} \times 10 = 50 \times 5 + H \times 5$ $50 \times 10 = 50 \times 5 + H \times 5$

 $\therefore H = 50 \text{ kN}$

FIG. A-106

113. (c) \therefore B.M. maxi = u.d.l. \times area of ILD

$$= 2 \times \frac{1}{2} \times 16 \times 3$$

$$= 48 \text{ kN.m}$$

$$= 4m \text{ log}$$

$$= 12m$$

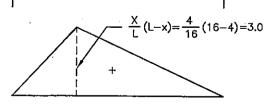


FIG. A-113 .

Concrete Technology

Short Answer Questions

(1) What are the raw materials of OPC?

A.

Raw Materials

1

Calcareous Materials

Argillaceous Materials

Lime Stone

Clay

- Chalk

Shale

(2) What are the main elements of oxide composition of OPC?

A. As per IS: 269 - 1989 elements of OPC are:

CaO (Lime)

60 - 70%

SiO₂ (Silica)

16 - 25%

Al₂O₃ (Alumina) -

3 - 8%

(3) Give OPC Bogue's compounds and their properties.

A. C₃S - It produces more heat of hydration.

- It is responsible for early strength.

C₂S - It produces less heat of hydration.

- It is responsible for later strength of concrete

C₁A - It causes flash set.

- It does not contribute to the strength of concrete.

C₄AF - It provides resistance to sulphate attack.

(4) What are the different grades of concrete as per IS -456 - 2000?

A. In IS: 456 - 2000, M-10 to M-80 grades of concrete are suggested.

 $M-7.5 \rightarrow 1:4:8$

 $M-10 \rightarrow 1:3:6$

 $M-15 \rightarrow 1:2:4$

 $M-20 \rightarrow 1:1.5:3$

 $M-25 \rightarrow 1:1:2$

(5) What is M 20?

A. M means Mix.

Concrete Technology

20 means \rightarrow 15 cm \times 15 cm \times 15 cm cubes tested after 28 days of curing, compressive strength should not be less than 20 N/mm² (MPa).

- (6) Why Gypsum is added to cement?
- A. To retard the setting time of cement 2 to 3% Gypsum is added to cement.
- (7) Suggest suitable type of cement.
- A. Concrete gravity dam, Retaining wall

Marine structures

Road repair, Culverts

Under water construction

Waterproofing of dam

- Low heat cement

- Sulphate resisting cement

- Rapid hardening cement

- Portland pozzolana cement

- Quick setting cement

- (8) How much water is required for full hydration of cement?
- 23% water for chemical reaction A.
 - + 15% to fill up the gel pores

38% water for full hydration of cement.

If less than 38% water is used, hydration will be incomplete. Some of the cement particles will remain dry and strength will be reduced.

If more the 38% water is added, then the excess water will cause undesirable cavities and concrete becomes porous.

- (9) What is heat of hydration?
- A. The reaction of cement with water is exothermic. The reaction liberates a considerable quantity of heat.

The quantity of heat (in joules) per gram of unhydrated cement, evolved upon complete hydration at a given temperature is defined as heat of hydration.

- (10) How cement is tested in the field?
- A. (1) Open the bag of cement and take a good look at the cement. There should not be any visible lumps.
 - (2) The colour of the cement should be greenish grey.
 - (3) When hand is inserted in cement bag it should give cool feeling.
 - (4) Take a pinch of cement and feel between the fingers. It should give a smooth feeling and not a gritty feeling.
 - (5) Take a handful of cement and throw it on a bucket full of water, the particles should float on water for some time before they sink.
- (11) Give IS criteria for fineness of cement.
- A. Sieve analysis test
- Cement retained on 90 µ sieve

For OPC ₹ 10%

For RHC ≯ 5%

Air permeability tests → Specific surface area

For OPC ≮ 2250 cm²/gm

For RHC ≮ 3250 cm²/gm

- (12) What is consistency of cement paste?
- A. Consistency means degree of fluidity or degree of stiffness or degree of softness.
- (13) Explain 'initial and final setting time' of cement.
- A. The period elapsing between the time when water is added to the cement and the time at which the cement paste starts losing plasticity is termed as initial setting time. For OPC initial setting time should not be less than 30 minutes. During this time period cement paste remains in plastic condition. Various operations in making concrete like mixing, transporting, placing and finishing must be completed within the initial setting time period.

The period elapsing between the time when water is added to the cement and the time at which the cement paste completely loose its plasticity is termed as **final setting** time. For OPC final setting time should not be more than 600 minutes (10 hours).

(14) Give IS criteria for compressive strength of OPC.

| | | 3 days | 7 days | 28 days (all N/mm ²) |
|----|----------------|--------|--------|----------------------------------|
| A. | OPC - 43 grade | 23 | 33 | 43 |
| | OPC - 53 grade | 27 | 37 | 53 |

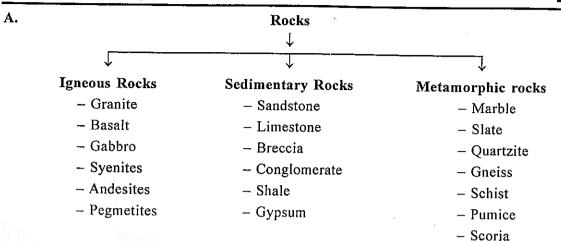
- (15) What is 'unsoundness' of cement?
- A. Undesirable expansion of some of the constituents of cement after setting is known as unsoundness.

Unsoundness of cement is due to excess lime, Magnesia or sulphates.

By Le-chatellier apparatus, expansion of cement should not be more than 10 mm.

- (16) Give requirement of water for different tests on cement.
- A. Initial and final setting time -0.85 P %Soundness test -0.78 P %P = of water for standard consistency

 Compressive strength of cement $-\left(\frac{P}{4} + 3.0\right)\%$
- (17) Give list of natural aggregates from different rocks.



(18) What is 'plum' concrete?

A. The original idea of the use of aggregate as an inert filler can be extended to the inclusion of large stones up to 300 mm size in a normal concrete; thus the apparent yield of concrete for a given amount of cement is increased. The resulting concrete is called 'plum concrete' or 'cyclopean concrete'.

(19) What is maximum size of aggregate in RCC?

- A. (i) Clear cover 5 mm
 - (ii) Spacing between Mainbars 5 mm
 - (iii) 20 mm

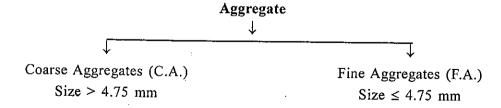
A.

Not more than smaller of three values.

(20) Give different tests on aggregate and their IS criteria.

| A. | lests Name | | I. S. Criteria |
|------|----------------------------|-------------|------------------------------------|
| | Aggregate Impact Value | > | 30% concrete for Wearing surfaces. |
| | (Passing 2.36 mm Sieve) | > | 45% concrete for Buildings. |
| | Aggregate Crushing Value | * | 30% concrete for Roads. |
| | (Passing 2.36 mm Sieve) | > | 45% concrete for Buildings. |
| | Los Angeles Abrasion Value | > | 30% concrete for Roads. |
| | (Passing 1.70 mm Sieve) | > | 45% concrete for Buildings. |
| (21) | O' T '0" '1 A | | · · |

(21) Give classification of aggregate based on size.



Objective Civil Eng. \ 2016 \ 23

(22) What is bulking of aggregates?

A. Bulking is the phenomenon of increase in the volume of fine aggregates caused by the presence of free moisture.

Free moisture forms a film around each particle. This film of moisture exerts what is known as surface tension which keeps the neighbouring particles away from it. This causes increase in volume of the mass of fine aggregates.

After addition of certain amount of water in FA, the further addition of water breaks the film around the particles and hence, volume gradually decreases. In CA bulking is negligible.

(23) What is 'alkali-aggregate reaction'?

A. Normally, aggregates used in concrete are considered as inert materials. But some of the aggregates contains reactive type of silica, which reacts with alkalies present in cement i.e. sodium oxide (Na2O) and potassium oxide (K2O). As a result, the alkali silicate gels of unlimited swelling type are formed. This reaction is known as 'alkali aggregate reaction'.

Factors affecting alkali-aggregate reaction are:

- reactive type of aggregate
- High alkali content (more than 0.6%)
- 10 to 38°C temperature
- Availability of moisture
- Fineness of cement particles.

(24) Distinguish between 'good grading' and 'gap grading'.

Good grading: Aggregate comprising particles of various sizes will give a mass of lesser voids. It is called 'good grading'.

Gap grading: When one or more intermediate size fractions are absent from a particular grading, it is termed as 'gap grading'.

(25) What is fineness modulus?

- 'Fineness modulus' is a single factor computed from the sieve analysis, and is defined as the sum of the cumulative percentages of aggregate retained on each of the standard sieves ranging from 80 mm to 150 micron divided by 100.
 - Fineness modulus (F.M.) =

Sum of cumulative percentages of Aggregate retained on sieve from 80 mm to 150 micron

Fineness modulus gives an idea of the mean size of the particles present in the entire body of the aggregate.

F.M. is a measure of coarseness or fineness of the aggregate. The smaller the F.M. value, the finer is the material. The following limits may be taken as guidance.

Concrete Technology

| Sand | F.M. |
|-------------|-----------|
| Fine sand | 2.2 - 2.6 |
| Medium sand | 2.6 - 2.9 |
| Coarse sand | 2.9 - 3.2 |

(26) Define 'Flakiness index' and 'elongation index'.

A. The flakiness index of aggregate is the percentage by weight of particles in it whose least dimension (thickness) is less than 3/5th (0.6) of their mean dimension. The test is not applicable to sizes smaller than 6.3 mm.

The elongation index of aggregate is the percentage by weight of particles in it whose greatest dimension (length) is greater than 1.8 times their mean dimension. The test is not applicable to size smaller than 6.3 mm.

(27) Give maximum limit of impurities in construction water.

A.

| Maximum limit |
|------------------------|
| 200 mg/lit. |
| 3000 mg/lit. |
| 400 mg/lit. |
| 2000 mg/lit. (For PCC) |
| 500 mg/lit. (For RCC) |
| 2000 mg/lit. |
| |

(28) What is admixtures?

A. Admixture is defined as a material other than the basic ingredients of concrete cement, aggregates and water, added to the concrete mix immediately before or during mixing to modify some properties of concrete in the fresh or hardened state. They should not adversely affect any property of concrete. Admixtures are no substitute for good concreting practice.

(29) Give list of accelerators and retarders.

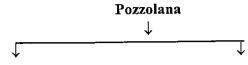
A. Accelerators - Calcium chloride - Sodium nitrate - Sodium silicate - Sodium aluminate - Sodium silicate - Sodium aluminate - Salts, etc.

(30) Give list of air-entraining agents.

A. - Aluminium Powder - Vegetable Oils, Fats
- Hydrogen Peroxide - Alkali Salts

(31) What is Pozzolana? What are its effects on properties of concrete?

The pozzolanic materials are essentially a silicious or aluminous materials which itself possessing no cementitious properties, which will, in finely divided form and in the presence of water, react with calcium hydroxide [Ca(OH)₂] liberated in the hydration process to form compounds possessing cementitious properties.



Natural Pozzolana

- Clay
- Shale
- Diatomaceous earth
- Volcanic tuff
- Opaline cherts

Artificial Pozzolana

- Fly ash
- Surkhi
- Blast furnace slag
- Silica fume
- Rice husk ash
- Metakaoline

Effects of Pozzolana:

- Decrease in Permeability
- Increase in workability
- Less heat of hydration
- Less alkali-aggregate reaction
- Decrease in cost

(32) Give importance of w/c ratio.

A. w/c ratio is kept between 0.40 to 0.60.

If w/c ratio is less than 0.40, hydration of cement will be incomplete and strength will be reduced.

If w/c ratio is more than 0.40, excess water will create undesirable capillary cavities and concrete will become porous.

(33) Explain 'Gel space ratio'.

Volume of Gel Gel space Ratio = Space available

Volume of hydrated cement paste

Volume of hydrated cement + Volume of capillary pores

(34) What is wokability?

The diverse requirements of partial properties of concrete like mixability, stability, transportability, placeability, mobility, compactibility and finishability are collectiviely referred to as wokability.

Signs of good workability are:

- easy flow
- free from bleeding

- Free from segregation
 - homogeneous mix

(35) List factors affecting workability.

- A. (1) Water content
 - (2) Mix proportions
 - (3) Size of aggregates
 - (4) Shape of aggregates
 - (5) Surface texture of aggregate

- (6) Grading of aggregate
- (7) Use of admixtures
- Time
- (9) Temperature

(36) List tests for workability of concrete.

A. - Slump test

Kelly-ball test

- Compacting factor test

Vee-Bee consistometer test

Flow test

(37) Give types of slump:

- A. 1. True Slump
 - 2. Shear Slump
 - 3. Collapse Slump

(38) Give relation between compacting factor and workability of concrete.

A. Compacting factors = $\frac{\text{weight of partially compacted concrete}}{\text{weight of fully compacted concrete}}$

| Workability | Compacting factor |
|-------------|-------------------|
| Very Low | 0.78 |
| Low | 0.85 |
| Medium | 0.92 |
| High | 0.95 |

(39) What is segregation? What are its causes?

A. Segregation can be defined as separating out of the ingredients of concrete mix, so that the mix is no longer in a homogenous and stable condition.

Causes:

- 1. Badly proportioned mix
- 2. Excess water content
- 3. Insufficient mixing of concrete
- 4. Dropping concrete from heights
- 5. Discharging concrete against an obstacle like formwork
- 6. Conveyance of concrete by wheel borrow, conveyor belts, etc.

(40) What is 'laitance'?

A. Bleeding water while coming from bottom to top, brings certain quantity of cement to the surface. When the surface is worked up with the trowel and floats, the aggregates goes down and the cement and water come up to the top surface. This formation of cement paste at the surface is known as 'laitance' or 'scum'.

(41) Give methods of transporting concrete.

A. - Mortar Pan

Chute

- Wheel Borrow

Skip & Hoist

Belt Conveyors

- Pumps & Pipeline

- Bucket and Ropeway

(42) Give various methods of curing of concrete.

- A. (i) Water curing Immersion Ponding, Spraying, Wet covering
 - (ii) Membrane curing
 - (iii) Application of heat
 - (iv) Miscellaneous surface coatings.

(43) What is modular ratio?

A. Modular ratio =
$$\frac{E \text{ for steel}}{E \text{ for concrete}}$$

$$\therefore m = \frac{E_s}{E_c}$$
where, $E_s = 2 \times 10^5 \text{ N/nm}^2$

$$E_c = 5000 \sqrt{f_{ck}}$$

$$f_{ck} = \text{grade of concrete in N/mm}^2$$

(44) What is creep?

A. The increase of strain in concrete with time under sustained (stable) load is termed as creep.

Factors affecting creep:

(1) Aggregate

- (2) Water/cement ratio
- (3) Age at the time of loading
- (4) Moisture content of the concrete
- (5) Humidity of the ambient air
- (6) Type of cement
- (7) Intensity and duration of stress
- (8) Size of the specimen

(9) Temperature.

(45) Explain Shrinkage of concrete.

A. With decrease in water content, there is reduction in volume of concrete. It is called Shrinkage.

Types of Shrinkage:

- (i) Plastic Shrinkage
- (ii) Drying Shrinkage
- (iii) Autogenous Shrinkage
- (iv) Carbonation Shrinkage.

(46) What is sulphate attack?

A. Solid salts (sulphates) do not attack concrete, but when present in solution they can react with hardened cement paste. In the hardened concrete, sulphates react with the free calcium hydroxide [Ca (OH)₂] to form gypsum (calcium sulphate). Similarly, sulphates react with calcium aluminate hydrate (C-A-H) to form calcium sulphoaluminate, the volume of which is approximately 117% of the volume of the original aluminates. The produce of the reactions, gypsum and calcium sulphoaluminate have a considerably greater volume than the compounds they replace, so that the reactions with the sulphates lead to expansion and disruption of the concrete. Of all

Concrete Technology

the sulphates, magnesium sulphate causes maximum damage to concrete. A characteristic whitish appearance is the indication of sulphate attack.

- (1) Use of sulphate resisting cement
- (2) Addition of Pozzolana

(3) Quality of concrete

- (4) Use of air-entrainment
- (5) High-Pressure steam curing
- (6) Use of high-alumina cement
- (7) Lining of polyethylene sheet

(47) Compare strength obtained by different size of specimens.

- A. $15 \text{ cm} \times 15 \text{ cm} \times 15 \text{ cm}$ cube
- its strength is considered as standard strength.

 $10 \text{ cm} \times 10 \text{ cm} \times 10 \text{ cm}$ cube

- strength is reduced by 10%

15 cm dia., 30 cm height cylinder

- strength is increased by 20%

(48) Give different methods of testing of hardened concrete.

A. Destructive Tests

Non-destructive Tests - Rebound hammer test

- Compressive Strength test
- Rebound hammer test
- Flexural strength test
- Penetration & Pull out testUltrasonic Pulse velocity test
- Tensile Strength test
- Radioactive tests
- Nuclear tests
- Acoustic emission tests.

(49) Define the terms.

A. (1) Mean strength (\bar{x}) :

This is the average strength obtained by dividing the sum of strength of all the cubes by the number of cubes.

$$\frac{1}{x} = \frac{\sum x_i}{n}$$

where, $\bar{\chi}$ = mean strength

 $\sum x_i$ = sum of strength of all cubes

n = number of cubes.

Mean is a measure of central tendency or tendency of getting grouped about a central value.

(2) Variance:

This is the measure of variability or difference between any single observed data from the mean strength.

$$\therefore \quad \text{variance} = x_i - \overline{x}.$$

(3) Range:

The range is the difference between the largest and the smallest values in a set of observations.

(4) Standard deviation (s):

The standard deviation or root mean square deviation of a set of observations (Population) is defined as:

$$S = \sqrt{\frac{\sum (x_i - \overline{x})^2}{n - 1}}$$

where, S = standard deviation

x = particular value of observation

 \bar{x} = mean strength

n = no. of cubes.

(50) What is concrete mix design?

A. Concrete mix design may be defined as the art of selecting suitable ingredients of concrete and determining their relative proportions with the object of producing concrete of certain minimum strength and durability as economically as possible.

(51) What are the objectives (purposes) of concrete mix design?

- The purpose of concrete mix design is to ensure the most optimum proportions of the constituent materials to fulfill the requirements of the structure being built. Mix design should ensure the following objectives.
 - (1) To achieve the designed/desired workability in the plastic stage.
 - (2) To achieve the desired minimum strength in the hardened stage.
 - (3) To achieve the desired durability in the given environmental conditions.
 - (4) To produce concrete as economically as possible.

(52) List various methods of concrete mix design.

- IS Method
 - (ii) ACI Method
 - (iii) Road Note No. 4 Method (U.K. Method)
 - (iv) IRC 44 Method
 - (v) F. M. (Fineness Modulus) Method
 - (vi) Surface Area Method
 - (vii) Maximum Density Method
 - (viii) Arbitrary Proportion Method

(53) What is special concrete?

Conventional ordinary concrete has many drawbacks, like,

Poor tensile strength

Corrosion of reinforcement

Permeability

Less durability

Porosity

To overcome these drawbacks, concrete with some special characteristics is prepared. It is called special concrete.

(2) High Density concrete

(4) Plum concrete

(6) Aerated concrete

(8) Polymer concrete

(10) Fly ash concrete

Concrete Technology

(54) List different types of special concrete.

- (1) Light weight concrete
- (3) Ready Mixed Concrete (RMC)
- (5) No fines concrete
- (7) Fibre Reinforced Concrete (FRC)
- (9) Ferrocement
- (11) Pumped concrete

(55) Give types of light weight concrete.

- Light Weight Aggregate Concrete
 - Aerated Concrete (Foam Concrete)
 - No Fine Concrete
- (56) Give list of light weight aggregates.

Light Weight Aggregate

Artificial Aggregates

- Bloated Clay
- Expanded Shale
- Siutered Flyash
- Foamed Slag

Natural Aggregates

- Pumice
- Scoria
- Rice Husk
- Saw Dust
- Diatomite
- Volcanic Cinders

(57) Give advantages of light weight concrete.

- Reduction of dead load.
 - Smaller sections of structural members can be adopted.
 - Lower haulage and handling costs.
 - Increase in the progress of work.
 - Reduction in foundation costs, particularly in the case of weak soil and tall structures.

(58) How Cellular Concrete or (Foam Concrete) is prepared?

- Producing gas in concrete by chemical reaction
 - Adding foam in concrete
 - Adding metal powder (Aluminium powder or hydrogen peroxide)

(59) What do you mean by fibre reinforced concrete?

Fibre reinforced concrete (FRC) can be defined as a composite material consisting of concrete and discontinuous, discrete, uniform dispersed fine fibres. The continuous meshes, woven fabrics and long wires or rods are not considered to be discrete fibres.

Objective Civil Eng. \ 2016 \ 24

| (60) Give list of fibres used in fibre reinforce |
|--|
|--|

A. - Steel

- Carbon

Glass

Asbestos

Polypropelene

Nylon, etc.

(61) What is polymer concrete? Give its types.

A. Polymer impregnated concrete is produced by impregnating or infiltrating a hardened Portland cement concrete with a monomer and subsequently polymerizing the monomer in situ. It is one of the widely used polymer composite.

Types of polymer concrete:

- (i) Polymer Impregnated Concrete (PIC)
- (ii) Polymer Cement Concrete (PCC)
- (iii) Polymer Concrete (PC)
- (iv) Partially Impregnated and Surface Coated Polymer Concrete.

(62) What are the aggregates used in high density concrete?

A. Aggregates used in high density concrete:

Magnetite

Limonite

- Hematite

- Barytes, etc.

Density of high density concrete ranges from 3400 kg/m³ to 5500 kg/m³.

This concrete is used for radian shielding.

(63) What is ferrocement?

A. Ferro cement is a relatively new material consisting of wire meshes and cement mortar. It consists of closely spaced wire meshes which are impregnated with rich cement mortar mix. While the mortar provides the mass, the wire mesh imparts tensile strength and ductility to the material. The ferro cement possess high resistance against cracking, high fatigue resistance higher toughness and higher impermeability.

(64) What are the applications of ferrocement?

A. (i) Mobile Homes

(ii) Water tight Structure

(iii) Silos and bins

(iv) Boat hulls

(v) Pipes

(vi) Folded Plates

(vii) Shell roofs, etc.

(65) What is nominal mix?

A. A concrete mix prepared by using fix proportions, without taking into consideration the properties of constituents of concrete, without any criteria of workability and durability is called nominal mix.

e.g. M 20 - 1 : 1.5 : 3 (nominal mix)

It is used for ordinary concrete.

(66) What is design mix?

A. The concrete mix produced under quality control, keeping in view the strength, durability and workability is called the design mix. For all important works, concrete mix is designed.

(67) List causes of corrosion of reinforcement steel.

presence of cracks in concrete

(ii) presence of moisture

(iii) permeability of concrete

(iv) carbonation

(v) chlorides

(vi) sulphate attack

(vii) alkali aggregate reaction

(viii) inadequate cover to reinforcement.

(68) List remedial measures to prevent corrosion of reinforcement steel.

- Application of protection coating to steel.
 - (ii) using concrete of low permeability.
 - (iii) proper mixing, compaction and curing of concrete.
 - (iv) providing minimum 20 mm clear cover.
 - (v) using minimum cement 350 kg/m³.
 - (vi) using maximum w/c ratio equal to 0.45
 - (vii) using PPC cement.

(69) List causes of cracks in concrete.

A. (i) Moisture movent

(ii) Temperature variation

(iii) Plastic Shrinkage

(iv) Creep

(v) Chemical reaction

(vi) Drying Shrinkage

(vii) Weathering

(70) What is 'Shrinkage Crack'?

When moisture evaporates from the surface of freshly placed concrete faster than it is replaced by bleed water, the surface of concrete shrinks. Due to shrinking tensile stress produced in the concrete which causes cracks in the concrete. It is called Shrinkage cracks.

(71) What are the causes of Shrinkage cracks in concrete?

A. (i) Cement content

- Higher the cement content more will be Shrinkage

(ii) Aggregate

More the aggregate content, less will be the Shrinkage

(iii) Temperature

- At higher temperature Shrinkage will be more

(iv) Moisture in atmosphere - Lesser the air moisture more will be Shrinkage

(v) Curing

- Inadequate curing may increase Shrinkage

(72) List materials used for repair of cracks.

A. (i) Cement Slurry

(ii) Cement mortar

(iii) Epoxy resin

(iv) Polymer resin

- (73) List materials used in grouting.
- A. For grouting, a mixture of cement + water + Sand is used. Sometimes bentonite clay is also used.
- (74) List the methods of application of repair materials.
- A. (i) Dry packing
 - (ii) Concrete replacement
 - (iii) Mortar replacement
 - (iv) Grouting
 - (v) Guniting or Shotcreting
- (75) What is 'Guniting' or 'Shotcreting'?
- A. 'Shotcrete' or 'Guniting' is a mortar or a fine concrete that is pneumatically transported through a hose and projected on to a surface at a high velocity. The thickness of layer is small in guiniting while it is thick in shotcreting. In shotcreting small size course aggregate are also used in mortar.
- (76) What is full form of PPC, RHC, PSC?
- A. PPC Portland Pozzolana Cement
 - RHC Rapid Hardening Cement
 - PSC Portland Slag Cement
- (77) What is code number for concrete mix design?
- A. IS: 10262 1982
- (78) What is standard size of concrete slump mould?
- A. Top diameter 10 cm
 - Bottom diameter 20 cm
 - Height 30 cm

MCQ'S

SET-1

| | | <u>5.</u> | | |
|----|-----------------------------------|----------------------|-----------------------------|---------------------|
| 1. | The maximum per | centage of chemic | al ingredient of cement is | s that of |
| | (a) Alumina | (b) Silica | (c) Magnesium oxide | (d) lime |
| 2. | The following mate | erial does not belo | ong to calcareous rocks | , |
| | (a) lime stone | (b) marl | (c) chalk | (d) laterite. |
| 3. | Pick up correct pro | oportions of chem | ical ingredients of ordina | ry Portland cement. |
| | (a) lime: silica: A | lumina : Iron oxid | e :: 63 : 22 : 6 : 3 | |
| | (b) Silica: lime: A | lumina : Iron oxid | e :: 63 : 22 : 6 : 3 | |
| | (c) Alumina: silica | : lime : Iron oxid | e :: 63 : 22 : 6 : 3 | |
| | (d) Iron oxide: Alu | mina : silica : lim | e :: 63 : 22 : 6 : 3. | |
| 4. | Pick up the correc | t statement from | the following. | |
| | (a) Lime in excess, | causes the cement | to expand and disintegrate | e. |
| | (b) Silica in excess, | causes the cemen | t to set slowly. | |
| | (c) Alumina in exce | ess, reduce the stre | ngth of cement. | |
| | (d) Magnesium oxic | le in access, remai | ns in free state and causes | unsoundness. |
| | (e) all of the above | | | |
| 5. | Pick up incorrect | statement from th | e following. | |
| | (a) C ₃ S hydrates rap | pidly. | | |
| | (b) C ₃ S generates m | nore heat of hydrat | ion. | |
| | (c) C ₃ S develops ea | arly strength. | | |
| | (d) C ₃ S has more re | esistance to sulpha | te attack. | |
| 6. | Pick up incorrect | statement from th | e following. | |
| | Tricalcium alumina | te (C_3A) | | . (|
| | (a) reacts fast with | water | | |
| | (b) generates less h | eat of hydration | | |
| | (c) causes flash set | ting of cement | | |
| | (d) does not contrib | oute to develop ult | imate strength. | |
| 7. | Efflorescence in co | ement is caused d | ue to an excess of | |
| | (a) Alumina | (b) Iron oxide | (c) Alkalis | (d) Silica. |
| 8. | Pick up incorrect | statement from tl | ie following. | |
| | Dicalcium silicate (| (C,S), | • | |

| | (a) hydrates slowly. | | | | | | |
|-------------------------------------|---|--|------------------------|---------------------|--|--|--|
| (b) responsible for early strength. | | | | | | | |
| | (c) provide more resistance to chemical attack. | | | | | | |
| | (d) generates less heat of | hydration. | | | | | |
| 9. | Portland pozzolana cem | Portland pozzolana cement is used with confidence for, | | | | | |
| | (a) Dams | (b) Abutments | (c) Massive found | lations | | | |
| | (d) R.C.C. structures | (e) All the abo | ove. | | | | |
| 10. | The standard sand now | adays used in I | India is obtained from | , | | | |
| | (a) Jaipur (Rajasthan) | (b) Jullandhar | (Punjab) | | | | |
| | (c) Hyderabad (A.P.) | (d) Ennore (M | (adras) | | | | |
| 11. | Sand normaly requires | a _, higher w/c ra | ntio belongs to | | | | |
| | (a) zone I (b) | zone II | (c) zone III | (d) zone IV | | | |
| 12. | Sands of zone I are, | | | | | | |
| | (a) coarse (b) | medium | (c) fine | (d) medium to fine | | | |
| 13. | The types of aggregate | e of same non | ninal size which cont | ain less voids when | | | |
| | compacted are, | | | | | | |
| | (a) Flaky (b) | Rounded spher | ical (c) Irregular | (d) Elongated | | | |
| 14. | The bulk density of agg | | | | | | |
| | (a) shape (b) | grading | (c) compaction | (d) all of above | | | |
| 15. | The bulk density of agg | regate is gener | ally expressed as, | | | | |
| | (a) tonne/cubic metre (b) | kg/cubic metre | (c) kg/litre | (d) gm/cubic cm. | | | |
| 16. | The risk of segregation | is more for, | | | | | |
| | (a) wetter mix | | (b) coarser grading | | | | |
| | (c) large proportion of m | aximum size of | aggregate | | | | |
| | (d) all the above. | | | | | | |
| 17. | The most useless aggreg | gate is one who | se surface texture is | | | | |
| | (a) smooth (b) | granular | (c) glassy | (d) honey combed | | | |
| 18. | On a grading curve, ga | | | | | | |
| | (a) a horizontal line | | | | | | |
| | (c) N-W inclined line | | | | | | |
| 19. | To obtain very high str | ength concrete, | | | | | |
| | (a) Magnetite (b) | granite | (c) Barite | (d) volcanic scoria | | | |
| 20. | Natural light weight ag | gregates are ob | otained from, | | | | |
| | (a) Igneous rocks | , | (b) sedimentary rocks | | | | |
| | (c) metamorphia rocks | | (d) volcanic source | | | | |
| | | | | | | | |

| 21. | Saw dust ca | n be rendered ch | emically inert by | y boiling it in water | r containing |
|----------|---|----------------------|---------------------|------------------------|-------------------|
| | (a) Ammonia | a (b) Potas | ssium chloride (c) | Ferrous sulphate | (d) Nitric acid |
| 22. | For a given v | water content work | ability of concrete | e is good if aggregate | s used are |
| | (a) rounded | (b) irreg | ular (c) an | gular (d | i) flaky |
| 23. | Pick up the | correct statement | t: An excess of f | laky particles in co | ncrete aggregates |
| | (a) Increases | the quantity of w | ater and sand. (l | o) decreases the wor | kability. |
| | (c) affects th | ne durability of con | ncrete. (| d) more than 15% as | e not desirable. |
| | (e) all the al | bove | | | |
| 24. | An aggrega | te which passes tl | hrough 25 mm IS | S sieve and is retain | ned on 20 mm IS |
| | | l to be flaky if its | | | |
| | (a) 22.5 mm | (b) 18.5 | mm (c) 16 | .5 mm (| d) 13.5 mm |
| 25. | A flaky agg | regate is said to | be elongated if it | is length is, | |
| | (a) equal to | the mean size | (b) 1.5 | 8 times the mean siz | e |
| | (c) thrice th | e mean size | (d) fo | ur times the mean si | ze |
| 26. | | ial of a cement c | oncrete mix is | | |
| | (a) water | (b) aggre | egate (c) ce | ment (| d) none of these |
| 27. | An aggrega | te is known as 'c | yclopean aggrega | ite' if its size is mo | re than |
| | (a) 20 mm | (b) 30 m | | | d) 75 mm |
| 28. | The aggrega | te containing mois | ture in pores and | having its surface dr | y, is known as |
| | (a) Dry aggi | | | oist aggregates | |
| | (c) saturated | l surface dry aggre | egate (d) ve | ry dry aggregate | |
| 29. | Setting time | e of cement incre | ases by adding | | |
| | (a) Gypsum. | (b) Calc | ium chloride. (c) | Hydrogen peroxide. | (d) Sodium oxide. |
| 30. | Which is be | est accelerator fro | om followings? | | |
| | (a) Calcium | chloride. | (b) Ca | alcium sulphate (Gyj | osum). |
| | (c) Alumini | um powder. | (d) St | igars. | |
| 31. | Commercia | l brand name Im | permo is a | nico Inn | |
| | (a) Accelera | ting admixture. | (b) Pl | asticizer. | |
| | (c) Waterproofing admixture. (d) Bonding admixture. | | | | |
| | | | : ANSWERS | S: | . ' |
| <u> </u> | (1) (d); | (2) (d); | (3) (a); | (4) (e); | (5) (d); |
| | (6) (b); | (7) (c); | (8) (b); | (9) (e); | (10) (d); |
| | (11) (a); | (12) (a); | (13) (b); | (14) (d); | (15) (c); |
| | (16) (d); | (17) (c); | (18) (a); | (19) (b); | (20) (d); |
| | (21) (c); | (22) (a); | (23) (e); | (24) (d); | (25) (b); |
| | (26) (b); | (27) (d); | (28) (c); | (29) (a); | (30) (a); |
| | (31) (c). | | | | |

SET-2

| 1. | Pick up the correct statement. | | • | | | |
|----|--|---------------------------|--------------------------|--|--|--|
| | (a) Sogregation is necessary for a workable concrete. | | | | | |
| | (b) Consistency does not affect the workability of concrete. | | | | | |
| | | | | | | |
| | (d) If the concrete mix is dry, the slun | np is maximum. | | | | |
| | (a) None of the above. | | | | | |
| 2. | If the slump of concrete mix is 60 n | am, its workability is | (4) high | | | |
| | (b) low | (c) meature | (d) high | | | |
| 3. | (a) very low (b) low If the compacting factor of concrete | mix is 0.88, its workat | ollity is | | | |
| ٠. | | | | | | |
| 4. | (a) very low (b) low Workability of concrete mix havi | ng very low water cer | nent ratio should be | | | |
| 71 | ascertained by | | | | | |
| | (a) slump test | (b) tensile strength t | | | | |
| | | (d) flexural strength | test | | | |
| 5. | - Lover of concrete | e is compacted by a stee | rod 600 mm long and | | | |
| ٠. | 16 mm diameter for | | (d) 50 times | | | |
| | (a) 20 times (b) 25 times | (c) 15 times | (u) 50 times | | | |
| 6. | | YZ | orata | | | |
| | (a) honey combed concrete | (b) porous layer in | Concrete | | | |
| | (c) surface scaling in concrete | (d) sand streaks in | concrete | | | |
| | (e) all the above | | | | | |
| 7 | t t - t | | ament content | | | |
| | | be accompanied by an in | crease in cement content | | | |
| | as my town of concrete mix incre | eases with an increase in | | | | |
| | . I a component of the | duce the workauting or | | | | |
| A | (d) Large size aggregates increase the | ne workability due to 143 | ser surface area. | | | |
| | : elections of a cement bay | g for storage, are | | | | |
| , | (a) weight 50 kg (b) height 18 | cm (c) plan area 3000 | sq. cm | | | |
| | as 1:the above (e) all the above | ove | 4 | | | |
| | 9. To prevent segregation, the maxim | num height for placing | concrete is | | | |
| | (a) 100 cm (b) 125 cm | (c) 130 cm | (d) 200 cm | | | |
| | 10. Slump test of concrete is a measu | are of its | | | | |
| | (a) compressive strength | (b) consistency | | | | |
| • | (c) tensile strength | (d) impact value | | | | |
| | (0) 10 | | | | | |

| 11. | Workability of con- | crete may be impro | ved by adding | | |
|-----|--|--------------------------|--------------------------|--------------------------|--|
| | (a) fly ash | | (c) calcium chloride | | |
| | (d) bentonite | (e) all the above | | | |
| 12. | The cement become | es useless if its abso | orbed moisture exceeds | | |
| | (a) 1% | (b) 2% | (c) 3% | (d) 5% | |
| 13. | Minimum water/ce | ment ratio required | for a workable concre | te is, | |
| | (a) 0.30 | (b) 0.40 | (c) 0.50 | (d) 0.60 | |
| 14. | Grading of aggrega | ite in a concrete mi | x is necessary to achiev | re · | |
| | (a) adequate workab | ility | (b) higher density | | |
| | (c) reduction of voi | ds | (d) better durability | | |
| 15. | Reduction in A/C | atio while keeping | w/C ratio constant caus | | |
| | (a) decrease in work | cability | (b) workability is not a | ffected | |
| | (c) increase in work | ability(d) none of the | ne above | | |
| 16. | The reasonable slu | mp for mass concre | ete should be | | |
| | (a) 90 - 180 mm | (b) $10 - 30 \text{ mm}$ | (c) $50 - 75$ mm | (d) $25 - 50 \text{ mm}$ | |
| 17. | Workability test m | ost suitable for con | crete of very low works | ability is | |
| | (a) vee-bee test | (b) slump test (c) | compaction factor test | (d) kelly ball test | |
| 18. | Addition of pozzol | anas in to concrete | | | |
| | (a) workability | | (b) resistance to chemi | cal attack | |
| | (c) both of the above | | (d) none of the above | | |
| 19. | For mass concrete | works, the degree | of workability should b | e | |
| | (a) low | (b) average | (c) high | (d) very high | |
| 20. | Specific gravity of | fly ash falls in the | | | |
| | (a) 1.0 to 1.6 | (b) 1.6 to 1.9 | (c) 1.9 to 2.4 | (d) 2.4 to 2.8 | |
| 21. | Portland cement i | s heavier than wate | | | |
| | (a) 1.15 times | (b) 2.30 times | (c) 3.85 times | (d) 3.15 times | |
| 22. | The phenomenon | | in concrete mix which is | | |
| | (a) lean and wet | | et (c) coarse and dry | (d) lean and dry | |
| 23. | | | expressed in terms of: | | |
| | | concrete produced p | | | |
| | (b) total volume of concrete produced in 8 hours | | | | |
| | (c) total volume of | concrete produced p | per hour | | |
| | | erete mix handled pe | | | |
| 24 | | | gredients should be mea | sured to a tolerance | |
| | (as a percentage of | of batch quantity) o | | (1) L C O | |
| | $(a) \pm 1.0$ | (b) ± 2.0 | $(c) \pm 3.0$ | $(d) \pm 5.0$ | |
| | | | | | |

25. A mixer designated 400 NT indicates that

| | (a) It is a non-tilting | type mixer | | |
|-----|-------------------------|------------------------------------|--------------------------|---------------------------|
| | (b) Its nominal mix | batch capacity is 4 | 00 litres | |
| | (c) both of the above | e . | | |
| | (d) It is a non-tilting | type mixer requir | ing 400 revolutions for | r proper mixing. |
| 26. | | | | ie for mixers up to 750 |
| | | • | _ | als have been added is |
| | (in minutes) | | | |
| | (a) 1.0 | (b) 1.5 | (c) 2.0 | (d) 5.0 |
| 27. | While pumping con- | crete, | | |
| | (a) care should be ta | ken to reduce the | number of bends in the | e delivery pipe |
| | (b) the pipe should b | e cleaned immedi | ately after use | |
| | (c) initially a 1:3 cen | nent sand mortar sho | ould be pumped to lubric | ate the pipe line |
| | (d) all of the above. | | | |
| 28. | When concrete is pu | umped by a pump | of 60 h.p. the maxim | um horizontal distance |
| | that can be covered | would be, | | |
| | (a) 150 m | (b) 200 m | (c) 300 m | (d) 400 m. |
| 29. | A 90° bend in the pi | peline reduces the | effective pumping dist | tance by approximately, |
| | (a) 10 m | (b) 5 m | (c) 3 m | (d) 2 m |
| 30. | Ready mixed concre | ete (RMC) | , 75 | |
| | (a) is weigh batche | ed and mixed in a | centrally located plant | t, transported in a truck |
| | mixer or agitato | or and deliver <mark>ed</mark> in | a condition ready to u | ise |
| | (b) is produced und | ler site condi <mark>tio</mark> ns | | |
| | (c) does not require | e control of all of | perations of manufactu | re and transportation of |
| | fresh concrete | | | |
| | (d) all of the above | | | |
| 31. | | | | n of concrete by high |
| | frequency vibration | | | |
| | (a) g to 2g | | (c) 7g to 9g (d | • |
| 32. | | ' | sed for compaction o | |
| | (a) form vibrator | • • | or (c) surface vibrator | (d) screen vibrator |
| 33. | | | · - | prator is recommended. |
| | (a) Form vibrator | (b) Needle vibrat | or (c) Surface vibrator | (d)Vibrating table. |
| 34. | | effectived only w | hen the thickness of | concrete member does |
| | not exceed, | | | |
| | (a) 100 mm | (b) 150 mm | (c) 200 mm | (d) 500 mm. |
| | | | | |
| | | | | |

| 35. | While using vibrators for compacting concrete mixes | | | | |
|-----|--|--|--|--|--|
| | (a) vibrations are used for spreading concrete in the form | | | | |
| | (b) vibrations reduce entrained air | | | | |
| | (c) prolonged vibrations reduces chances of segregation. | | | | |
| | (d) all of the above. | | | | |
| 36. | Maturity of concrete is the, | | | | |
| | (a) 28 day strength of concrete (b) 7 day strength of concrete | | | | |
| | (c) Product of period of curing and temperature of curing | | | | |
| | (d) 365 day strength of concrete. | | | | |
| 37. | The following conditions of concrete placement are termed as extreme | | | | |
| | environmental conditions. | | | | |
| | (a) under water concreting | | | | |
| | (b) when concreting operations are carried out at temperature beyond 40°C | | | | |
| | (c) when concreting operations are done at temperature below 5°C. | | | | |
| | (d) any of the above | | | | |
| 38. | For a concrete slab for a 3.75×4.75 m room the stripping time of form should be | | | | |
| | (a) 3 days (b) 7 days (c) 14 days (d) 21 days | | | | |
| 39. | To take care of any sag in the beams, the forms are given a camber of, | | | | |
| | (a) 1:200 (b) 1:300 (c) 1:500 (d) 1:650 | | | | |
| 40. | The timber formwork for concrete should be made of | | | | |
| | (a) teak wood (b) shisham wood (c) soft wood planks (d) hard wood | | | | |
| 41. | Membrane curing of the concrete is the | | | | |
| | (a) process of providing plastic sheeting as a protective cover for curing concrete. | | | | |
| | (b) process of applying a membrane forming compound on the concrete surface. | | | | |
| | (c) process of spraying the sodium silicate on the concrete surface. | | | | |
| | (d) all of the above. | | | | |
| 42. | In cold weather curing of concrete should be continued for | | | | |
| , | (a) 7 days (b) 14 days (c) 21 days (d) 28 days. | | | | |
| 43. | In hot weather concreting it is recommended for | | | | |
| | (a) use of cold mixing water | | | | |
| | (b) use of low heat cement | | | | |
| | (c) have minimum cement content | | | | |
| | (d) reduce period between mixing and placement to a absolute minimum | | | | |
| | (e) all of the above. | | | | |
| 44 | . The standard moist curing of concrete for the first 7 to 14 days may result in a | | | | |
| | compressive strength of per cent of 28 day moist curing | | | | |
| | (a) 60 to 70 (b) 70 to 80 (c) 80 to 90 (d) 90 to 95. | | | | |
| | | | | | |

| 45. | In | steam | curing | concrete |
|-----|----|-------|--------|----------|
|-----|----|-------|--------|----------|

- (a) mixes of high w/C ratio respond more favourably than mixes of low w/C ratio.
- (b) the heating of concrete products is caused by steam at low pressure or at high pressure
- (c) steam curing is followed by water curing for a period of at least 21 days
- (d) all of the above.

46. A mix for under water concreting uses should have,

- (a) very low slump
- (b) low slump
- (c) high slump.
- (d) none of the above.

47. The water/cement ratio is expressed by

- (a) volume
- (b) weight
- (c) density
- (d) none of the above.

48. The freshly prepared concrete mix should be consumed normally within

- (a) 24 hours
- (b) 8 hours
- (c) $1\frac{1}{2}$ hours
- (d) 15 minutes
- 49. The stripping time in case of rapid hardening cement is x times that in ordinary cement, where x is,
 - (a) $\frac{1}{2}$
- (b) $\frac{3}{7}$
- (c) $\frac{3}{4}$

(d) $\frac{4}{7}$

50. Tremie is a

- (a) bucket
- (b) water-tight pipe (c) bag
- (d) prepackd concrete
- 51. The diameter of tremie pipe for under water concreting should not be less than
 - (a) 100 mm
- (b) 150 mm
- (c) 200 mm
- (d) 300 mm.

: ANSWERS :

| (2) (c); | (3) (c); | (4) (c); | (5) (b); |
|-----------|---|--|--|
| (7) (d), | (8) (e); | (9) (a); | (10) (b); |
| (12) (d); | (13) (b); | (14) (c); | (15) (c); |
| (17) (a); | (18) (c); | (19) (a); | (20) (c); |
| (22) (a); | (23) (d); | (24) (c); | (25) (c); |
| (27) (d); | (28) (d); | (29) (d); | (30) (a); |
| (32) (b); | (33) (c); | (34) (c); | (35) (b); |
| (37) (d); | (38) (c); | (39) (c); | (40) (c); |
| (42) (d); | (43) (e); | (44) (b); | (45) (b); |
| (47) (b); | (48) (c); | (49) (b); | (50) (b);(51) (c). |
| | (7) (d); (12) (d); (17) (a); (22) (a); (27) (d); (32) (b); (37) (d); (42) (d); | (7) (d); (8) (e); (12) (d); (13) (b); (17) (a); (18) (c); (22) (a); (23) (d); (27) (d); (28) (d); (32) (b); (33) (c); (37) (d); (38) (c); (42) (d); (43) (e); | (7) (d); (8) (e); (9) (a); (12) (d); (13) (b); (14) (c); (17) (a); (18) (c); (19) (a); (22) (a); (23) (d); (24) (c); (27) (d); (28) (d); (29) (d); (32) (b); (33) (c); (34) (c); (37) (d); (38) (c); (39) (c); (42) (d); (43) (e); (44) (b); |

SET-3

| 1. | The strength of concrete is decreased by |
|----|---|
| _, | (a) vibration (b) impact (c) fatigue (d) all of the above |
| 2. | The concrete may attain its 100 per cent strength after, |
| | (a) 7 days (b) 28 days (c) 1 year (d) 3 years |
| 3. | The tensile strength of concrete is approximately what per cent of compressive |
| | strength of concrete? |
| | (a) 50% (b) 20% (c) 10% (d) 5% |
| 4. | The knowledge of flexural strength is useful in design of, |
| | (a) reinforced concrete members (b) pavement slabs and air field runways |
| | (c) prestressed concrete structures (d) water retaining structures |
| 5. | The increase in strength of concrete with time is |
| | (a) linear (b) non-linear (c) asymptotic (d) all of the above |
| 6. | The split tensile strength of M15 grade concrete when expressed as a fraction of |
| | its compressive strength is, |
| | (a) 0.10 to 0.15 (b) 0.15 to 0.20 (c) 0.20 to 0.25 (d) 0.25 to 0.30 |
| 7. | Hardened concrete is, |
| | (a) linearly elastic material till the fracture |
| | (b) non-linearly elastic material till the fracture |
| | (c) linearly elastic up to the level where stress is less than 0.5 times the maximum |
| | stress in compression. |
| | (d) non of the above |
| 8. | Rheology of concrete deals with |
| | (a) deformation (b) compatibility (c) flow properties (d) all of the above |
| 9. | As per IS: 456-2000, the relationship between flexural strength (f_{cr}) and |
| | characteristic strength of concrete (f_{ck}) is (a) $0.5 \sqrt{f_{ck}}$ (b) $0.12 \sqrt{f_{ck}}$ (c) $0.7 \sqrt{f_{ck}}$ (d) $1.0 \sqrt{f_{ck}}$ |
| | (A) (J, J, J, J, J, L, |
| 10 | . The approximate ratio of concrete strength at 7 days to its strength at 28 days is |
| | (a) 3/4 (b) 2/3 (c) 1/2 (d) 1/3 |
| 11 | . The subject of 'Rheology' is more closely related to |
| | (a) strength of materials (b) Fluid mechanics |
| | (c) Engineering mechanics(d) none of the above |
| 12 | . The shrinkage in concrete is due to |
| | (a) hydration of cement |
| ٠, | (b) loss of water by evaporation from the surface |
| | (c) withdrawal of water stored in unsaturated air voids of concrete |
| | (d) all of the above. |

| 13. | The shrinkage of concrete can be r | educed by | |
|-----|---|---|-------------------|
| | (a) low water/cement ratio(b) | water-tight and non-absorbe | nt formwork |
| | (c) presaturated aggregates | (d) all of the above. | |
| 14. | Creep in concrete is undesirable par | rticularly in | |
| | (a) continuous beams (b) reinforce | ced concrete columns | |
| | (c) prestressed concrete structures | (d) all of the above | |
| 15. | Shrinkage increases with | | |
| | (a) increase in the water-cement ratio |) | |
| | (b) increase in cement content | | |
| | (c) decrease in humidity | | |
| | (d) decrease in the maximum size of | the aggregate | |
| | (e) all the above | | |
| 16. | For cement concrete the stress-stra | in curve is linear approxim | nately up to, |
| | (a) 1/4 of ultimate stress | (b) 1/3 of ultimate stre | SS |
| | (c) 1/2 of ultimate stress | (d) 3/4 of ultimate stre | SS . |
| 17. | The modulus used for all design pu | urposes is | |
| | (a) Tangent modulus | (b) Secant modulus | |
| | (c) Initial tangent modulus | (d) Chord modulus | |
| 18. | The modulus of elasticity can be m | reasured in | |
| | (a) tension (b) compression | n (c) shear | (d) all the above |
| | | | |
| 19. | Pick out the incorrect statement. | | |
| 19. | (a) Dense aggregates have a high ela | astic modulus. | |
| 19. | (a) Dense aggregates have a high ela (b) Wet concrete will show higher m | astic modulus. aodulus of elasticity. | |
| 19. | (a) Dense aggregates have a high ela(b) Wet concrete will show higher m(c) Modulus of elasticity decreases v | astic modulus. nodulus of elasticity. with age. | |
| | (a) Dense aggregates have a high ela (b) Wet concrete will show higher m (c) Modulus of elasticity decreases v (d) Richer mixes shows higher modulus | astic modulus. nodulus of elasticity. with age. | |
| | (a) Dense aggregates have a high ela (b) Wet concrete will show higher m (c) Modulus of elasticity decreases v (d) Richer mixes shows higher modu Pick out the incorrect statement. | nstic modulus. nodulus of elasticity. with age. nulus of elasticity. | |
| | (a) Dense aggregates have a high ela (b) Wet concrete will show higher m (c) Modulus of elasticity decreases v (d) Richer mixes shows higher modulus Pick out the incorrect statement. (a) Increase in water increases shrink | astic modulus. nodulus of elasticity. with age. nulus of elasticity. kage. | |
| | (a) Dense aggregates have a high ela (b) Wet concrete will show higher m (c) Modulus of elasticity decreases v (d) Richer mixes shows higher modu Pick out the incorrect statement. (a) Increase in water increases shrink (b) High cement content will reduce | astic modulus. nodulus of elasticity. with age. ulus of elasticity. kage. shrinkage. | |
| | (a) Dense aggregates have a high ela (b) Wet concrete will show higher m (c) Modulus of elasticity decreases v (d) Richer mixes shows higher modulated the incorrect statement. (a) Increase in water increases shring (b) High cement content will reduce (c) The harder aggregates gives lower | astic modulus. nodulus of elasticity. with age. nlus of elasticity. kage. shrinkage. er shrinkage. | imen. |
| 20. | (a) Dense aggregates have a high ela (b) Wet concrete will show higher m (c) Modulus of elasticity decreases v (d) Richer mixes shows higher modu Pick out the incorrect statement. (a) Increase in water increases shrink (b) High cement content will reduce (c) The harder aggregates gives lowed (d) Shrinkage decreases with an increase | astic modulus. nodulus of elasticity. with age. nlus of elasticity. kage. shrinkage. er shrinkage. | imen. |
| 20. | (a) Dense aggregates have a high elasticity with the concrete will show higher methods. (b) Wet concrete will show higher methods. (c) Modulus of elasticity decreases with an increase in water increases shrink the incorrect statement. (a) Increase in water increases shrink the high cement content will reduce the content will reduce the content will reduce the content with an increase with an increase with an increase with the incorrect statement. | astic modulus. nodulus of elasticity. with age. nlus of elasticity. kage. shrinkage. er shrinkage. rease in the size of the spec | imen. |
| 20. | (a) Dense aggregates have a high elate (b) Wet concrete will show higher m (c) Modulus of elasticity decreases w (d) Richer mixes shows higher module Pick out the incorrect statement. (a) Increase in water increases shrink (b) High cement content will reduce (c) The harder aggregates gives lowed (d) Shrinkage decreases with an increase. Pick out the incorrect statement. (a) Light weight aggregate undergoe | astic modulus. nodulus of elasticity. with age. ulus of elasticity. kage. shrinkage. er shrinkage. rease in the size of the spec | imen. |
| 20. | (a) Dense aggregates have a high elasticity with the concrete will show higher methods. (b) Wet concrete will show higher methods. (c) Modulus of elasticity decreases with an increase in water increases shrink the incorrect statement. (a) Increase in water increases shrink the high cement content will reduce the content will reduce the content will reduce the content with an increase with an increase with an increase with the incorrect statement. | astic modulus. nodulus of elasticity. with age. ulus of elasticity. kage. shrinkage. er shrinkage. rease in the size of the spec s lower creep. water/cement ratio. | imen. |
| 20. | (a) Dense aggregates have a high ela (b) Wet concrete will show higher m (c) Modulus of elasticity decreases w (d) Richer mixes shows higher modulated the incorrect statement. (a) Increase in water increases shrind (b) High cement content will reduce (c) The harder aggregates gives lowed (d) Shrinkage decreases with an increase of the incorrect statement. (a) Light weight aggregate undergoe (b) Creep increases with increase in (c) Portland blast stag cement result (d) Creep decreases with increase in | astic modulus. nodulus of elasticity. with age. ulus of elasticity. kage. shrinkage. er shrinkage. rease in the size of the spec s lower creep. water/cement ratio. s in a higher creep. the size of the specimen. | imen. |
| 20. | (a) Dense aggregates have a high elasticity decreases with an increase with a higher materials. (b) Wet concrete will show higher mode of the content will reduce the content will reduce the content of the content | astic modulus. nodulus of elasticity. with age. ulus of elasticity. kage. shrinkage. er shrinkage. rease in the size of the spec s lower creep. water/cement ratio. s in a higher creep. the size of the specimen. | imen. (d) 1.1 |

| 23 | The permanent deferment | -4. 6 | | |
|-----|-------------------------------|-------------------|-----------------------------|------------------------------|
| 2. | (a) cross | ation of concrete | e with time, un | der sustained load is called |
| | (a) creeh (b |) relaxation | (c) viscosity | (d) visco-elasticity |
| 24 | . The shrinkage of conc | | | |
| | (a) length (b |) cross-sectional | l area (c) volur | ne (d) surface area |
| 23 | . Concrete shrinkage is | | | • |
| | (a) rich mix (b |) very lean mix | (c) lean mix | (d) normal mix |
| 26 | . Concrete with aggregat | te of higher mo | dulus of elastic | city will shrink |
| | (a) by same amount (b) |) less | (c) more | (d) more or less the same |
| 27 | . The time dependent ph | enomenon in co | oncrete is | |
| | (a) gain of strength (b) |) shrinkage | (c) creen | (d) all of the above |
| 28 | . Creep of the concrete i | s influenced by | | |
| | (a) strength of concrete | | (b) age of con | crete |
| | (c) water cement ratio | | (d) all of the | above |
| 29. | The ratio of ultimate co | reep strain to e | lastic strain is | known as |
| | (a) creep modulus | | (b) creep coeff | |
| | (c) creep strain ratio | | (d) tertiary cre | |
| 30. | Shrinkage strain in con | crete is approxi | imately | |
| | (a) 0.0035 (b) | 0.0350 | (c) 0.0003 | (d) 0.3500 |
| 31. | The approximate value | of the thermal | coefficient of e | expansion of concrete is |
| | (a) 9×10^{-5} per °C | | (b) 10×10^{-6} | ner °C |
| | (c) 9×10^{-7} per °C | | (d) 8×10^{-6} pc | |
| 32. | Modulus of elasticity of | concrete increa | ases with | |
| | (a) the age | | (b) the increase | e in W/C ratio |
| | (c) the decrease in curing | period | (d) all of the a | |
| 33. | The most appropriate m | ethod to specif | v the concrete | mix is by |
| | (a) the nominal mix ratio | (b) the designed | d mix ratio | |
| | (c) the degree of control | | (d) the grade of | f concrete |
| 34. | The strength of concrete | is influenced b | ov | a concrete |
| | (a) size of test specimen | (b) moisture co | nditions | |
| | (c) rate of loading (d) | type of testing r | nachine (e) a | Il of the above |
| 35. | The concrete may attain | its 100 per cen | it compressive | strongth often |
| | (a) 7 days (b) | | | l) 1 year (e) 3 years. |
| 36. | The tensile strength of co | oncrete is annro | (v) 1. days (d (ximately | of compressive strength |
| | of concrete. | | | or compressive strength |
| | (a) 50% (b) 3 | 20% | (c) 10% | (4) 50/ |
| 37. | The standard size of a co | | r compressive : | (d) 5% |
| | (a) 50 mm (b) j | 100 mm (| (c) 150 mm | (d) 200 |

| 38. | For compressive strength determination the minimum number of cubes required | | | | | |
|------|--|-----------------------|-------------|--|--|--|
| | in a sample are | | • | | | |
| | (a) 2 (b) 3 | (c) 5(d) 6. | | | | |
| 39. | Split tensile strength tests are better than the direct tensile strength tests because | | | | | |
| | (a) the test gives move uniform results | | | | | |
| | (b) the results give values closer to the actual tensile strength values | | | | | |
| • | (c) same moulds can be used for both tension tests and compression tests | | | | | |
| | (d) all of the above. | | | | | |
| 40. | With increase in the rate of loading during testing of concrete specimens, the | | | | | |
| | compressive strength of concrete | | | | | |
| | (a) increases | (b) decreases | | | | |
| | (c) does not change | (d) none of the above | e. | | | |
| 41. | ISI has specified the full strength of concrete after | | | | | |
| | (a) 7 days (b) 14 days | (c) 28 days | (d) 1 year. | | | |
| 42. | The 100 mm cubes may be provided for the work test of concrete if the maximum | | | | | |
| | nominal size of aggregate does not exceed, | | | | | |
| | (a) 10 mm (b) 15 mm | (c) 20 mm | (d) 25 mm. | | | |
| .43. | The individual variation in compressive strength results should not exceed | | | | | |
| | of the average. | | | | | |
| | (a) 5% (b) 10% | (c) 15% | (d) 20%. | | | |
| | | | | | | |

: ANSWERS :

| (1) (d); | (2) (d); | (3) (c); | (4) (b); | (5) (b); |
|----------|--------------|-----------|-----------|-----------|
| (6) (a); | (7) (c); | (8) (d); | (9) (c); | (10) (b); |
| (11) (b) | ; (12) (d); | (13) (d); | (14) (c); | (15) (e); |
| (16) (b) | ; (17) (b); | (18) (d); | (19) (c); | (20) (b); |
| (21) (a) | ; (22) (d); | (23) (a); | (24) (c); | (25) (a); |
| (26) (b) |); (27) (d); | (28) (d); | (29) (b); | (30) (a); |
| (31) (b) |); (32) (a); | (33) (d); | (34) (e); | (35) (e); |
| (36) (c) |); (37) (c); | (38) (b); | (39) (d); | (40) (a); |
| (41) (c) |); (42) (d); | (43) (c). | | |

SET-4

| 1. | 1. Efflorescence in cement is caused due to an excess of | | | | |
|----|---|----------------------|------------------------|------------------------|----|
| | (a) alkalis | (b) iron oxide | (c) silica | (d) alumina | a |
| 2. | Fly ash can be use | ed as | | | |
| | (i) Partial replace | ement of fine aggreg | gate | • | |
| | (ii) Partial replace | ement of cement | | ÷ | |
| | (iii) as an admixtu | ire. | | | |
| | The correct answer | ·is | | | |
| | (a) only (i) | (b) only (ii) | (c) both (i) and (ii) | (d) both (i) and (iii) | |
| 3. | The presence of c | ommon salt in sand | d result in | | |
| | (a) corrosion of re | inforcement | (b) scaling | | |
| | (c) pitting | | (d) all of the above | | |
| 4. | Carbonation of co | oncrete result into | | | |
| | (a) increased shrin | kage | (b) increased streng | th | |
| | (c) both of the abo | ove | (d) increased perme | ability C | |
| 5. | Permeability of co | oncrete reduces | | | |
| | (a) with the carbon | nation of concrete | (b) with the strengt | h of cement paste | |
| | (c) with the decrea | ase in the porosity | (d) all of the above | | d |
| 6. | The durability of | concrete is due to | its resistance to | | |
| | (a) deterioration fr | om environmental c | onditions | | |
| | (b) internal disrup | tive forces | (c) chemical attack | (d) all of the above | ٠. |
| 7. | The thermal cond | luctivity of concret | e decreases with the | | d |
| | (a) light weight co | oncrete (b) increase | in the water/cement ra | atio | |
| | (c) decrease in the | e cement content | (d) all of the above | e d | |
| 8. | The concrete for | sea water applicati | on should not be lean | ier than | |
| | (a) 1:2:6 | (b) 1:2:4 | (c) 1:2:3 | (d) 1:1:2 | С |
| 9. | 9. In case of plain concrete exposed to sea waves, the grade of concrete should not be lower than | | | | |
| | (a) M 15 | (b) M 20 | (c) M 25 | (d) M 30 | |
| 10 | . For high frost re | sistance, the concre | ete should be | | |
| | (a) dense | | (b) free from Crac | ks | |
| | (c) air-entrained | | (d) all of the abov | e | d |
| Ob | iactiva Civil Eng \ 20 | u16 \ 26 | | | |

| ~~ | _ |
|-----|---|
| 711 | • |
| | |

| 11 The total water | er soluble sulphat | es as SO, in the | e concrete mix | should not exceed |
|--------------------|-----------------------|------------------|-------------------|------------------------|
| (a) 2% | (b) 4% | (c) 6% | | (d) 8% |
| hy tha mass | of the cement. | | - | |
| 12 Which of the | following is the co | orrect ascending | order of the ag | gregates with respect |
| to thermal c | onductivity of con | crete ? | | |
| (a) Expanded | shale, quartzite, g | ranite, basalt | | |
| (h) Expanded | shale, granite, qua | artzite, basalt | | |
| (c) Expanded | shale, basalt, gran | ite, quartzite | | |
| (d) Quartzite | , basalt, granite, ex | panded shale | | |
| 13. Expansion jo | oints are provided | when length o | f building excee | eds |
| (a) 20 m | (b) 45 m | (c) 60 | m | (d) 100 m |
| ` ' | ters the concrete f | from | • | |
| (a) cement | (b) mixin | | mixtures | (d) aggregates |
| (e) all of the | above | | | |
| 15. The permea | bility of cement p | aste is mainly | due to | |
| (a) Gel pore | | (b) ca | apillary pores | |
| (a) finer cen | ent grains | (d) al | ll of the above | |
| 16. The length | of time over whic | h the structura | l concrete prese | rves structural action |
| is called | | | | |
| (a) fire ratin | g (b) therm | nal conductivity | (c) specific heat | (d) diffusivity |
| | 4.0 | : ANSWERS | | |
| | | : ANSWERS | | |
| (1) (a); | (2) (d); | (3) (a); | (4) (c); | (5) (d); |
| | | 123 () | (0) (1-) | (10) (d); |
| (6) (d); | (7) (d); | (8) (c); | (9) (b); | (10) (a), |
| nnien Fr | unotini | (12) (1) | (14) (e); | (15) (b); |
| (11) (b); | (12) (c); | (13) (b); | (14) (6), | (10) (0), |
| (16) (0) | | | | |
| (16) (a). | | | | |

<u>SET-5</u>

| 1. | The light-weight concrete may be produced by |
|----|---|
| | (a) Using light weight aggregate (b) Incorporating air bubbles in concrete |
| | (c) Omitting sand fraction (d) all of the above |
| 2. | Light-weight concrete is used |
| | (a) For reducing the dead weight of structures |
| | (b) In filler wall panels in highrise buildings |
| | (c) For improving thermal insulation |
| | (d) Any of the above |
| 3. | Light weight concrete has all the following beneficial characteristic except, |
| | (a) High thermal insulation (b) High sound insulation |
| | (c) Excellent fire resistance (d) Reduced drying shrinkage |
| 4. | Aerated concrete is produced by addition of |
| | (a) Sodium silicate (b) Copper sulphate |
| | (c) Aluminium powder (d) zinc |
| 5. | Vacuum concrete |
| | (a) Is the normally cured hardened concrete involving removal of air from the voids |
| | of the concrete by suction. |
| | (b) Is obtained by vacuum treatment of fresh concrete involving the removal of excess |
| | water and air by suction. |
| | (c) Is no fine concrete. |
| | (d) Has a low wear and abrasion resistance |
| 6. | |
| | (a) A large size aggregate and low slump is adopted |
| | (b) Heat of hydration is more. |
| | (c) There is early high strength but lower later strength. |
| | (d) Mix being harsh and dry requires immersion type power vibrators. |
| | (e) All of the above |
| 7. | |
| | fibre reinforced concrete? |
| | (a) Glass fibres (b) Polypropylene, nylon and other organic fibres |
| | (c) Carbon fibres (d) Asbestos fibres |
| 8. | Polymer impregnated concrete is obtained by |
| | (a) Impregnating low viscosity prepolymer or monomers into the pore structure of |
| | hardened concrete and polymerizing it by heating. |
| | (b) Replacing the cement-water matrix by pre-polymer and polymerizing it. |

| | (c) | | concrete during the mixing state. |
|-----|-----------------|---|---|
| | (d) | Any of the above processes. | testion can be achieved by |
| 9. | In | polymer concrete (Resin concrete) the | polymerization can be achieved by |
| | (a) | |) catalyst-promoter reaction |
| | (c) | radiation |) all of the above |
| 10. | Sho | tcrete | the letting it with high velocity on to the |
| | (a) | | ed by jetting it with high velocity on to the |
| | | prepared surface | rad structures |
| | (b) | | rete |
| | (c) | | |
| | (d) | all of the above otcrete differs from conventional conc | rete with regard to |
| 11. | Sh | materials, proportions and void system | (b) compaction |
| | | application procedure | (d) all of the above |
| 10 | ~ | | |
| 14. | · (0) | is the technique of depositing very thin I | ayers of mortar in each pass of nozzle |
| | (a _j |) mix is 1:3 to 1:4.5 with a water-ce | ment ratio of about 0.30 |
| | (0 | requires careful and skilful handling of | nozzle |
| | /3 | 1) -11 of the chore | |
| 13 | T. | he cement-sand ratio in the ferroceme | nt matrix should not be leaner than |
| | (2 |) 1·1 5 (b) 1:2.0 | (c) 1:3.0 (d) 1.4.0 |
| 14 | . T | he volume of reinforcement in ferroce | ment varies between |
| | (a | (b) 2-5% | (c) 5-8% (d) 8-1070 |
| 15 | . T | he sand recommended for ferrocement | mixes is |
| | (8 | | d 1.18 mm with optimum grading zones II |
| | | and III respectively. | utus maio I |
| | (1 | b) with maximum size 4.75 mm and gr | ading zone I. |
| | (| c) with maximum size 0.6 mm and gra | ding zone iv. |
| | (| d) none of the above. | t miv should be |
| 1 | | The water-cement ratio for ferrocemen | (b) between 0.35 to 0.40 |
| | (| a) less than 0.35 | • • |
| | (| c) between 0.40 to 0.50 (d) between 0. | |
| 1 | 7. J | Light weight concrete can used in | (b) non-load bearing walls |
| | (| (a) air-conditioned buildings | |
| | (| (c) providing reduced thickness of struct | • • |
| 1 | | Concreting in hot weather will result i | (b) increased cracking |
| | | (a) increased strength | (d) unevenness |
| | (| (c) retard setting | (-) |
| | | | |

19. Shotcrete is used in the application of (b) Waterproofing (a) Soil stabilization (d) None of the above (c) Stabilization of rock slopes 20. Concreting in cold weather results in cracking due to the temperature differential within concrete mass. (ii) increase the rate of strength development (iii) delay the removal of formwork (c) Both (ii) and (iii) (d) Both (i) and (iii) (b) Only (ii) (a) Only (iii) 21. Concreting in hot weather increases tendency to cracking (ii) makes air-content control difficult (iii) increases strength of hardened concrete. (b) both (i) and (ii) (c) both (ii) and (iii) (d) (i), (ii) and (iii) (a) only (i) 22. Roller compacted concrete is a (b) zero slump concrete (a) stiff concrete mix (c) mix having consistency of damp gravel (d) all of the above 23. Ready mixed concrete (RMC) is specified in terms of performance parameters (ii) produced under factory conditions (iii) produced and supplied by weight The correct answer is (b) both (i) and (iii) (c) both (ii) and (iii) (d) (i), (ii), (iii) (a) both (i) and (ii) 24. The RMC producer guarantees the desired performance (i) (ii) receives instructions through job specifications (iii) receives instructions in terms of prescriptive specifications The correct answer is (d) none of these (b) both (i) and (ii) (c) both (i) and (iii) (a) only (i)

: ANSWERS:

| (1) (d); (6) (e); | (2) (d); (7) (a); | (3) (b); (8) (a); | (4) (c); (9) (d); | (5) (b); (10) (d); |
|----------------------|----------------------|----------------------|----------------------|-----------------------|
| (1) (d); | (2) (d); | (3) (b); | (4) (c); | (5) (6); |
| (6) (e); | (7) (a); | (8) (a); | (9) (d); | (10) (d); |
| (11) (d); | (12) (d); | (13) (c); | (14) (c); | (15) (a); |
| (16) (b); | (17) (d); | (18) (b); | (19) (c); | (20) (d); |
| (21) (b); | (22) (d); | (23) (a); | (24) (b). | |

<u>SET-6</u>

| 1. | The approximate strength of concrete | at 28 days as a percen | itage of strength at | | | |
|----|--|-----------------------------|-----------------------|--|--|--|
| | one year is | | | | | |
| | (a) 98 (b) 90 | (c) 80 | (d) 75 | | | |
| 2. | The ratio of tensile strength of concret | e to its compressive str | ength is, | | | |
| | . 1 | | (d) $\frac{1}{33}$ | | | |
| | (a) $\frac{1}{10}$ (b) $\frac{1}{20}$ | (c) $\frac{1}{25}$ | $\frac{(a)}{33}$ | | | |
| 3. | Identify the Incorrect statements. | | _ | | | |
| | (a) Nominal mix is a mix of fixed prop | ortions which ensure ad | lequate strength. | | | |
| | (b) Nominal or standard mixes may be | used for high performar | nce concrete. | | | |
| | (c) Nominal mixes may result in under | or over rich mixes. | | | | |
| | (d) Standard mixes are useful as off-the self sets of proportions that allow the desired | | | | | |
| | concrete to be produced. | | | | | |
| 4. | The choice of mix proportions of a con | icrete is independent o | f | | | |
| | (a) grade designation | | | | | |
| | (b) maximum nominal size of aggregate | | | | | |
| | (c) minimum w/C ratio | | | | | |
| | (d) batching, mixing, placing, compactio | n techniques. | | | | |
| 5. | The maximum nominal size of coarse | aggregate is determin | ed by sieve analysis | | | |
| | and is designated by the sieve size higher | er than the largest size of | on which the material | | | |
| | retained is more than | | | | | |
| | (a) 5% (b) 15% | (c) 25% | (d) 50% | | | |
| 6. | | | | | | |
| | (a) Volume of water to volume of cemen | | | | | |
| | (b) Volume of water to the weight of ce | ment | | | | |
| | (c) Weight of water to the weight of cer | | | | | |
| | (d) Weight of water to the volume of ce | ement. | | | | |
| 7. | 7. The common mix design method for r | nedium strength concr | ete is | | | |
| | (a) The trial and adjustment method | (b) IS method | (c) ACI method | | | |
| | (d) DOE method (e) all of the above | /e. | | | | |
| 8. | 3. In a trial concrete mix, if the desired | slump is not obtained, | the adjustment in the | | | |
| | water content for each 10 mm differe | nce in slump (in per c | ent) is | | | |
| | (a) 0.5 (b) 1.0 | (c) 2.0 | (d) 5.0 | | | |
| 9. | 9. If the trial mix gives a higher 28 days | compressive strength | value than the design | | | |
| | value, then for the next trial, | | • | | | |
| | (a) cement content is reduced. | (b) water content is re | educed. | | | |
| | (c) w/C ratio is increased. | (d) proportion of sand | l is increased | | | |
| | | | | | | |

| 10. | amount e, the quantity of water is reduced by w then the reduction in the solid volume of sand is given by | | | | |
|-----|--|------------------------|---------------------------|------------------------------|--|
| | _ | | (c) $e - \frac{w}{2}$ | (d) $\frac{e}{2} - w$ | |
| 11. | When water is adde | d in an increasing | amount to a fixed m | ass of dry mortar mix, | |
| | the volume of morta | ar | | | |
| | (a) Initially increases | | | | |
| | (b) does not change | as water simply fills | s the voids | | |
| | (c) decreases | | (d) Increases | | |
| 12. | | | | of mortar is termed as, | |
| | (a) Saturation water | content | (b) hygroscopic water | er content | |
| | (c) basic water conte | | (d) highest water co | ntent | |
| 13. | The nominal mix co | orresponding to M2 | 20 grade concrete is | | |
| | (a) 1:1:2 | | (c) 1:2:3 | (d) 1:2:4 | |
| 14. | The grade of concre | ete corresponding t | to nominal mix prop | | |
| | (a) M 20 | (b) M 15 | | (d) M 25 | |
| 15. | The total number o | f grades of ordina | ry concrete stipulated | d in IS:456 - 2000 are | |
| | (a) 10 | (b) 8 | (c) 3(d) 5 | • | |
| 16. | For slabs and beam | is the concrete of n | iominal mix generall | y used is | |
| | (a) 1:1:2 | (b) 1:1.5:3 | (c) 1:2:4 | (d) 1:3:6 | |
| 17. | For water retaining | strenctures the no | minal mix generally | used is | |
| | (a) 1:1:2 | (b) 1:1.5:3 | | (d) 1:2:4 | |
| 18. | For a water-cement | ratio of 0.6 the w | ater content per bag | of cement is | |
| | (a) 10 kg | (b) 20 kg | (c) 30 kg | (d) 40 kg | |
| 19. | To ensure proper q | uality control the | number of cube spec | cimens to be cast for 5 | |
| | m ³ of concrete are, | | | | |
| | (a) 3 | \ <i>\</i> | (c) 9(d) 12 | | |
| 20. | In which method of | mix design there is | a provision of design | of mix using fly ash? | |
| | (a) Is method | (b) ACI method | (c) DOE method | (d) IRC -44 method | |
| 21. | Pumpable concrete | | | | |
| | (a) is transported the | rough completely fil | led dilivery pipes | | |
| | (b) Should have a sl | lump between 50 m | m and 100 mm | | |
| | | | n 270 kg/m³ to 460 kg | | |
| | (d) Maximum size of | crushed aggregate is l | imited to one-third of th | ne in side diameter of hose. | |
| | (e) all of above | | | | |
| ٠. | | | | | |
| | | | | | |

(1) (c);

(6) (c);

(2) (a);

(7) (e);

| | : ANSWERS | | |
|---|-----------|-----------|---------------------|
| 1 | (3) (b); | (4) (d); | (5) (b); |
| | (8) (b); | (9) (c); | (10) (a); (11) (a); |
| | (14) (c); | (15) (c); | (16) (b);(17) (a); |
| | (20) (c); | (21) (e). | |

| | (12) (c); (18) (c); | (13) (b); (19) (a); | (14) (c); (20) (c); | (15) (c); (21) (e). | (16) (b);(17) (a); | |
|--|---|------------------------------------|---------------------------------------|------------------------|----------------------|--|
| <u>L</u> _ | | | <u>SET-7</u> | | | |
| 1. | | | ncrete structures | by using epoxy, | the minimum width | |
| | of routing 1 | | | • | (d) 15 mm | |
| | (a) 3 mm | (b) 6 m | nm (c) 1 | 0 mm | (d) 15 mm | |
| 2. | | are caused by | | | C C | |
| | (a) Lack of | compaction | · · · · · · · · · · · · · · · · · · · | mproper design of | | |
| | (c) Poor wo | | • • • | excess water conte | ent | |
| 3. | | s in concrete str | | ue to | | |
| | (a) Errors is | n design and deta | 1 / | Corrosion of reinfe | orcement | |
| (c) Poor construction practices (d) All of the above | | | | | | |
| 4. | The tool av | vailable for evalu | nation of concrete | structures is | | |
| | (a) Visual inspection (b) Scrutiny of field date (c) NDT (d) All of the above | | | | | |
| 5. | | cracks can be co | ontrolled by | | | |
| | (a) Erecting | wind breaks | (b) S | | from direct sunlight | |
| | (c) Coverin | ng the concrete sur | rface by plastics s | sheet (d) All of t | he above | |
| 6. | The cracki | ing due to corros | sion of reinforcen | nent is character | rised by | |
| ٧. | | d reinforcement | (b) | Splitting and spa | alling of concrete | |
| | (c) Longitu | id <mark>inal cracks par</mark> al | llel to the bar (d |) All of the above |) | |
| 7. | The most | common symptor | m of distress in a | concrete structi | are is | |
| | | g of concrete | | (b) Surface crazi | ing // | |
| | · · · · · | ng of concrete | | (d) Scaling of co | oncrete | |
| 8. | The corro | sion of steel rei | nforcement emb | edded in concre | te is rapid when the | |
| 0. | | mmersed in | | | | |
| | (a) Acidic | * | (b) | Water with dissol | ved oxygen | |
| | (c) Alkalin | | ` ' | Sea-water | | |
| 0 | (c) Aikaiiii | deep cracks in c | ` ' | | d by | |
| 9. | (a) Groutin | | ovy injection (c) | Shotcreting (d) M | Nortar replacement | |
| | (a) Grouin | ig (0) Dp | joky injection (c) | | • | |
| | | | : ANSWERS | S : | | |
| 1 | | | (2) (1) | (4) (4)- | (5) (d): | |

| | | • 12210 | | | |
|----------------------|----------------------|----------------------|----------------------|----------|--|
| (1) (b); (6) (d); | (2) (c); (7) (b); | (3) (d); (8) (a); | (4) (d); (9) (a). | (5) (d); | |

SET-8

| 1. | The | reaction of cem | ent | with water is | called | İ | | • |
|------|-------|--------------------------|--------|--------------------------------|--------|------------------------------------|--------|-------------------------------|
| | (a) | segregation | (b) | bleeding | (c) | hydration | (d) | carbonation |
| 2. | Wh | y the name 'por | tlanc | d cement' was | giver | l. | | |
| | (a) | It was named at | fter t | he inventor. | | | | |
| | (b) | It is resembling | to a | type of limest | one o | ccuring at portland | in E | ngland. |
| | (c) | In olden days, i | t wa | s used at ports. | | | | |
| | (d) | It contains portl | and | as part of its in | igred: | ients. | | |
| 3. | The | amount of gyps | um, | usually added | in m | anufacture of cem | ent i | s |
| | (a) | 0.1 to 0.5 % | (b) | 0.5 to 1.0 % | (c) | 1 to 2 % | (d) | 2 to 3 % |
| 4. | Clin | kers are formed | at | a temperature | of | | | |
| | (a) | 500°C | (b) | 1000°C | (c) | 1200°C | (d) | 1500°C |
| 5. | The | residue on I.S. | sieve | No. 9 of ordi | nary | portland cement s | houl | d not exceed |
| | (a) | 5% | (b) | 10% | (c) | 15% | (d) | 20% |
| 6. | The | residue on I.S. | sievo | No. 9 of rapi | d ha | rdening cement sho | uld | not exceed |
| | (a) | 5% | (b) | 10% | (c) | 15% | (d) | 20% |
| 7. | The | minimum specif | fic s | urface area fo | OP | C is | | |
| | (a) | 1250 cm ² /gm | (b) | $2250 \text{ cm}^2/\text{gm}$ | (c) | $3250 \text{ cm}^2/\text{gm}$ | (d) | $4000 \text{ cm}^2/\text{gm}$ |
| 8. | If P | is the % of wa | ter | required for s | tanda | ard consistency of | ceme | ent, then % of |
| | wate | er required for i | nitia | al settin <mark>g t</mark> ime | test i | s | | |
| | (a) | | (b) | | ` ' | 0.85 P | | 0.75 P |
| 9. | For | | | | | str <mark>ength req</mark> uired a | fter ' | 7 days is |
| | (a) | 23 N/mm ² | (b) | 33 N/mm ² | (c) | 43 N/mm ² | (d) | 50 N/mm ² |
| 10. | In q | uick setting cem | | the compound | adde | ed is called | | |
| | (a) | aluminium sulph | ate | ation, | (b) | gypsúm | | |
| | (c) | Calcium sulphat | | | (d) | aluminium silicate | | |
| 11. | Effl | orescence in cem | ent | is caused due | to an | excess of | | |
| | (a) | | • / | iron oxide | ` ' | silica | (d) | alkalis |
| 12. | For | road pavements | | _ | ally t | ised is | | |
| | (a) | ordinary portlan | d cei | ment | (b) | rapid hardening cer | nent | • |
| | (c) | low heat cement | • | (d) | blast | furnace slag cemen | nt | |
| 13. | Sepa | ration of coarse a | ggre | egates from mor | tar d | uring transportation | ı is k | nown as |
| | (a) | bleeding | (b) | creeping | (c) | segregation | (d) | shrinkage |
| 14. | Sepa | ration of water, o | r wa | iter sand cemen | t froi | n a freshly mixed co | ncret | te is known as |
| | (a) | bleeding | (b) | segregation | (c) | creeping | (d) | flooding |
| Obje | ctive | Civil Eng. \ 2016 | 27 | | | | | |

| 210 | Objective of the state of the s |
|---|--|
| 15 For the construction of thin R.C.C | c. structures the type of cement to be avoided is |
| (a) ordinary portland cement | (b) Diast Inflace Bing Comment |
| (c) rapid hardening cement | (d) low heat cement |
| 16. Inert material of a cement concre | te mix, is |
| (b) cement | (c) aggregate |
| (a) water (b) the color of the | nfidence for construction of |
| (a) dams (b) abutment | s (c) massive foundations (d) all the above |
| - ata Irnovun | as a superior of the state of t |
| (a) Newton's law (b) Abram's | law (c) Hook s law (d) law of |
| 19. Which is the most desirable prop | perty of concrete: |
| (b) laitance | (c) hydration (d) 120115 |
| (a) bleeding (b) landales 20. When the aggregates are of single | e sized, the grading is termed as |
| (a) well graded | (b) gap graded |
| to td | (d) Continuous graded |
| 21 'Green concrete' can also be ref | erred to as which of the concrete below? |
| (a) hardened concrete | (b) Eco-Honary |
| | (d) Concrete of green colour |
| 22 By what percentage the strength | of concrete shall get reduced due to the presence |
| of 5 % voids? | |
| (b) 45 % | (c) 25 % (d) 5 % |
| 22 Which of the ingredients below | has the least specific gravity? |
| (1) Conrac | aggregate (C) watci |
| 24 Who developed the first portlar | nd cement by burning limestone and clay at hig |
| tomperature? | |
| (h) Duff A | bram (c) Aspdin (d) Le-chatellier |
| 25. Which instrument is used to de | termine soundness of cement? |
| (a) Vicat apparatus | (b) Le-chatelier apparatus |
| (c) Briquette apparatus | (d) cubes |
| ac (Cnowaam) is | . ~ _ |
| (a) Dawdered lime (b) Chalk | powder (c) Coloured lime |
| 27. A compacting factor of 0.85 fo | r a concrete sample indicates |
| 11.:1:4** | (b) Low workability |
| () 35-45 workshility | (d) high workability |
| 28. According to IS, the full stren | gth of concrete is achieved after |
| - 4 (/l-) 1/ do: | vs (c) 21 days . (d) 28 days |
| (a) 7 days (b) 14 days 29. How can you make concrete s | et faster? |
| | (b) Adding calcium chloride |
| - | (d) Adding extra cement |
| (c) Adding sugars | ` ' |

(a) 1:3:6

(b) 1:2:4

(c) 1:1.5:3

(d) 1:1:2

| 42. | The | proportions cer | nent : Sand | : aggregate | s for M 10 non | inal mix | c is |
|-----|---|-----------------------------|--------------|-----------------|----------------------------|-----------|----------------|
| | (a) | 1:3:6 | (b) 1:2: | 4 (c) | 1:1.5:3 | . (q) | 1:1:2 |
| 43. | The | number of cem | ent bags req | uired for 1 | m ³ of M 20 con | crete wo | uld be roughly |
| | (a) | 4 | (b) 6 | (c) | 8 | (d) | 10 |
| 44. | Hea | t of hydration o | f cement is | determined | by an apparati | ıs called | |
| | (a) | Hydrometer | (b) Calori | | Pycnometer | (d) | Hygrometer |
| 45. | The | maximum % o | f ingredient | in cement i | s that of | | |
| | (a) | lime | (b) silica | (c) | Alumina | (d) | Iron oxide |
| 46. | The | type of cement | preferred i | n dam const | truction is | | |
| | (a) | Ordinary portla | | (b) | Rapid hardenir | ng cemen | t |
| | (c) | low heat cemer | nt | • • | blast furnace s | | nt |
| 47. | The standard sand now a days used in India is obtained from | | | | | | |
| | (a) | Jaipur (Rajasth | | (b) | Jallandhar (Pi | ınjab) | |
| | (c) | Hyderabad (A.) | 2.) | • | Ennore (Madra | ıs) | |
| 48. | For | tunnel lining, 1 | ransportati | on of concre | te is done by | | |
| | (a) | Pans | (b) Whee | l barrows (c) | Pumps | (d) Bel | t conveyors |
| 49. | An | aggregate is sai | d to be flak | cy if its least | dimension (thi | ickness) | is less than |
| | | $\frac{1}{5}$ th of mean | | | $\frac{2}{5}$ th of mean | | |
| | (c) | $\frac{3}{5}$ th of mean of | limension | | $\frac{4}{5}$ th of mean | | n |
| 50. | An aggregate is said to be elongated if its length is more than | | | | | | |
| | (a) | 1.5 times their | mean dime | nsion (b) | 1.8 times their | r mean d | imension |
| | (c) | 2.0 times their | mean dime | nsion (d) | 2.5 times their | r mean d | imension |
| | | | | · ANSWER | !S : | | |

| 1. (c) | 2. (b) | 3. (d) | 4. (d) | 5. (b) |
|---------------------|---------|---------|----------|----------|
| 6. (a) | 7. (b) | 8. (c) | 9. (b) | 10. (a) |
| 11. (d) | 12. (b) | 13. (c) | 14. (a) | 15. (b) |
| 16. (c) | 17. (d) | 18. (b) | 19. (c) | 20. (c) |
| 21. (b) | 22. (c) | 23. (c) | 24. (c) | 25. (b) |
| 26. (c) | 27. (b) | 28. (d) | 29. (b) | 30. (c) |
| 31. (c) | 32. (a) | 33. (d) | 34. (c) | 35. (b) |
| 36. (a) | 37. (c) | 38. (d) | 39. (c) | 40. (d) |
| 41. (c) | 42. (a) | 43. (c) | 44. (b) | 45. (a) |
| 46. (c) | 47. (d) | 48. (c) | 49. (c) | 50. (b) |
| 4 0. (0) | 1 (4) | l | <u> </u> | <u> </u> |

Design of RCC Structures

Short Questions with Answers

1. What is limit state?

The acceptable limit for the safety and serviceability requirements before failure occurs is called a 'limit state.'

(IS: 456 - 2000, P.67)

2. List different types of limit states.

Limit state

(IS: 456, P. 67)

Limit state of collapse

- · Flexure (bending)
- · Shear
- · Compression
- · torsion

Limit state of serviceability

- Deflection
- Cracking
- Vibration

3. What is characteristic strength?

Characteristic strength means that value of strength of the material below which not more than 5 % of test results are expected to fall.

4. What is characteristic load?

Characteristic load means that value of load which has a 95 % probability of not being exceeded during the life of the structure.

5. What is design strength?

Design strength, $f_d = \frac{f_{ck}}{\gamma_m}$... for concrete

where,

 f_{ck} = characteristic strength of concrete

 $\gamma_{\rm m}$ = Partial safety factor

= 1.5 for concrete

Design strength, $f_d = \frac{f_y}{\gamma_m}$... for steel

where,

 f_y = characteristic strength of steel

 $\gamma_{\rm m} = 1.15$ for steel

6. What is design load?

design load, $F_d = F \times \gamma_f$

where,

F = Characteristic load

 γ_f = partial safety factor for loads

7. Characteristic strength for different grades of concrete and steel.

• For concrete, $f_{ck} = cha$. strength

| Grade | $\mathbf{f}_{\mathbf{ck}}$ |
|-------|----------------------------|
| M 20 | 20 N/mm ² |
| M 25 | 25 N/mm ² |
| M 30 | 30 N/mm ² |

• For steel, $f_v = \text{characteristic strength}$

| Grade | $\mathbf{f_y}$ |
|---------------------------|-----------------------|
| Mild steel, Fe - 250 | 250 N/mm ² |
| HYSD bars, Fe - 415 | 415 N/mm ² |
| Cold twisted bars, Fe 500 | 500 N/mm ² |

8. What is partial safety factor?

Due to some reasons, the material strength in actual structure gets reduced. Hence, the design strength of material is obtained by applying suitable partial safety factor, to the characteristic strength.

For concrete,
$$\gamma_m - 1.5$$

For steel, $\gamma_m = 1.15$

9. What is design compressive strength for M-20 Concrete?

$$f_d = \frac{f_{ck}}{\gamma_m} = \frac{20}{1.5} = 13.33 \text{ N/mm}^2$$

10. What is tensile strength (flexural strength) for M 20 grade of concrete? (IS: 456 - 2000, P. 16)

Flexural strength,
$$f_{cr} = 0.7 \sqrt{f_{ck}}$$

= 0.7 $\sqrt{20}$
= 3.13 N/mm²

11. What is modulus of elasticity for steel and M 20 grade concrete?

For mild steel,
$$E_s = 2 \times 10^5 \text{ N/mm}^2$$

For concrete, $E_C = 5000 \sqrt{f_{ck}}$
= 5000 $\sqrt{20}$

$$= 22360.7 \text{ N/mm}^2$$

Design of RCC Structures

215

12. What is M - 20?

M 20 is the grade of concrete

M = Mix

20 = 20 N/mm² compressive strength at 28 days

 $150 \times 150 \times 150$ mm size concrete cubes are tested after 28 days of curing. If compressive strength of concrete is more than 20 N/mm², the grade of concrete is referred to as M 20.

13. What is neutral axis (N.A.)?

The axis which separate compression zone and tension zone in the cross section of a beam is known as neutral axis.

14. What is under reinforced section?

- Steel content is less as compared to balanced section
- tensile strain in steel reaches limiting value, earlier to compressive strain in concrete reaching the limiting value
- Steel fail before concrete
- $x_{\rm u} < x_{\rm umax}$
- M_u , calculated w.r.t steel $(M_u = T \times Z)$

15. What is balanced section?

- Compressive strain in concrete and tensile strain in steel reach their limiting value simultaneously
- Both the materials fail at the same time
- $x_{\rm u} = x_{\rm umax}$
- M_u , can be calculated w.r.t. steel or concrete. $M_u = T \times Z$ or $M_u = C \times Z$

16. What is over reinforced section?

- Compressive strain in concrete reaches limiting value earlier to tensile strain in steel
- reinforcement is more as compared to balanced section
- concrete fail before steel
- $x_{\rm u} > x_{\rm umax}$
- M_u can be calculated w.r.t. concrete

$$M_{ij} = C \times Z$$

17. Give equation for x_{ii} .

$$x_{\rm u} = \frac{0.87 \, \rm f_y \, Ast}{0.36 \, \rm f_{ck} \, b}$$
 (IS : 456 – 2000, P. 96)

18. What are the values of x_{umax} for different grades of steel?

(IS: 456 - 2000, p. 70)

| f _v , N/mm ² | x _{umax} |
|------------------------------------|-------------------|
| 250 | 0.53 d |
| 415 | 0.48 d |
| 500 | 0.46 d |

19. Write equations for Mulim for different grades of steel.

| fy, N/mm ² | M _{ulim} |
|-----------------------|---|
| 250 | $M_{ulim} = 0.148 f_{ck}bd^2$ |
| 415 | $M_{ulim} = 0.138 f_{ck}bd^2$ |
| 500 | $M_{\text{ulim}} = 0.133 \text{ f}_{\text{ck}} \text{bd}^2$ |

20. Give equation for total compression force in concrete and total tensile force in steel for singly R.C. beam.

$$T = 0.87 f_y A_{st}$$

$$C = 0.36 f_{ck}.b.x_{u}$$

21. What is lever arm?

The distance between force C and force T is called lever arm.

$$Z = d - 0.42 x_{\rm u}$$

22. What is doubly R.C. beam?

When a beam is provided with tensile steel (A_{st}) in tension zone and compression steel (A_{sc}) in compression zone, it is called doubly R.C. beam.

23. When doubly R.C. beam is provided?

- When $M_u > M_{ulim}$
- When it is not possible to increase the depth of beam due to architectural reason
- · to get more headway in the rooms
- · When a beam is subjected to reversal of stresses (during earthquake)
- · For precast beams

24. Differentiate between singly R.C. beam and doubly R.C. beam

| | Singly R.C. beam | | Doubly R.C. beam |
|---------------------|-----------------------------------|---|--|
| · A _{st} i | s provided in tension zone | | A_{st} is provided in tension zone and A_{sc} is provided in compression zone $M_u > M_{ulim}$ |
| · M _u · | < M _{ulim} | • | M.R. is more |
| | . is less | • | Ductility of beam is more |
| · Duct | tility of beam is less | • | Suitable for reversal of stresses. (e.g. |
| · Not | suitable for reversal of stresses | | earthquake) |

25. What is minimum and maximum % of tension reinforcement in beams ? (IS: 456-2000, P. 46, 47)

Mini. reinforcement,
$$\frac{A_{st}}{bd} = \frac{0.85}{f_y}$$

For $f_y = 250 \text{ N/mm}^2$

$$\frac{A_{st}}{bd} \times 100 = p_t$$
 = $\frac{0.85}{f_y} \times 100$ = $\frac{0.85}{250} \times 100 = 0.34 \%$

For $f_y = 415 \text{ N/mm}^2$,

$$p_t = \frac{0.85}{415} \times 100 = 0.205 \%$$

Maximum reinforcement,

$$A_s = 0.04 \text{ bD}$$
 i.e. 4 % of gross area

26. What is nominal cover requirements?

27. What is maximum spacing of bars in beams?

Objective Civil Eng. \ 2016 \ 28

(IS: 456, P. 46, 47)

28. Write equation for width of flange for T-beam.

$$b_f = \frac{l_o}{6} + b_w + 6D_f$$
 (IS: 456, P. 37)

29. Write equation for development length of bars.

$$L_{d} = \frac{\phi \cdot \sigma_{s}}{4 \cdot \tau_{bd}}, \qquad \sigma_{s} = 0.87 \text{ f}_{y}$$

30: What is development length?

The length of reinforcement bar embedded in concrete, which develop bond stress, is termed as development length (Ld).

31. What is diagonal tension and shear cracks?

At the simply supported end of a beam, BM = 0

$$SF = \frac{wl}{2}$$
 (maximum)

direct tensile stress σ = 0
 Major principle stress

$$\sigma_{n_1} = \frac{\sigma}{2} + \sqrt{\left(\frac{\sigma}{2}\right)^2 + \tau^2}$$

= τ ... diagonal tension

Since concrete is weak in tension, cracks are developed at 45° near supports. These cracks are called shear cracks. Shear reinforcement is provided perpendicular to the direction of cracks

- 32. What are different types of stirrups in beams?
 - (i) vertical stirrups: 2-legged, 4-legged, 6-legged
 - (ii) inclined stirrups: Normally provided at 45° near the end of a beam
- 33. Write equation for minimum shear reinforcement.

$$\frac{A_{sv}}{b.S_{v}} \ge \frac{0.4}{0.87 f_{y}}$$
 (IS: 456, P. 48)

34. What are IS criteria for maximum spacing of shear reinforcement?

(IS: 456, P. 48, 73)

(i) 0.75 d

(ii) 300 mm

(iii)
$$S_v = \frac{0.87 f_y \cdot A_{sv} \cdot d}{V_{us}}$$

(iv)
$$S_v = \frac{0.87 f_y \cdot A_{sv}}{0.4 b}$$

whichever is smaller

35. What is nominal shear stress?

$$\tau_{\rm u} = \frac{V_{\rm u}}{h_{\rm o}d}$$
, $V_{\rm u} = \text{shear force}$ (IS: 456, P. 72)

- 36. Why shear reinforcement is provided in beams?
 - · to resist shear force
 - · To keep longitudinal bars in position
- 37. Why deformed bars are used in RCC?
 - To increase bond with concrete
- 38. List factors affecting shear strength of concrete
 - (i) Shear strength increases with compressive arength
 - (ii) increase in % of tensile steel, increases shear strength of concrete
- 39. Why design bond stress (τ_{bd}) is increased for deformed bars and compression? (IS: 456, P. 43)
 - Deformed bars: τ_{bd} is increased by 60 %, because deformed surface of bars increase bond between steel and concrete
 - Compression: τ_{bd} is increased by 25 %, because end bearing increases resistance to compression.

40. Define creep. List two negative effects of creep.

Creep is that property of concrete by which it continues to deform with time under sustained load

Negative effects of creep are:

- deformation of concrete is 2 to 3 times its initial deformation.
- · In RCC column, initial stress produces due to creep
- 41. What steps to be taken to reduce creep?
 - use high strength concrete
 - adding reinforcement
 - delaying finishes, partition walls, etc.
- 42. Define effective depth and effective cover.
 - d = effective depth
 - = distance between extreme compression fibre to the centre of tensile reinforcement
 - e = effective cover
 - = distance between extreme tensile fibre (bottom) to the centre of tensile reinforcement

43. What is balanced design?

In the balanced design, limiting stresses occur in steel and concrete at the same time and both the materials fail simultaneously.

44. Why over reinforced designed is not preferred?

In the over reinforced section, concrete fails before steel. Concrete is a brittle material, and fails suddenly without giving any warning signs.

45. Write equation to find load carrying capacity of axially loaded short column.

$$P_{u} = 0.4 f_{ck} \cdot A_{c} + 0.67 f_{y} A_{sc}$$

(IS: 456, P. 71)

46. What is minimum eccentricity for column?

(IS: 456, P. 42)

$$e_{\min} = \frac{l}{500} + \frac{D}{30}$$

Subjected to minimum of 20 mm

l = unsupported length of column

D = Lateral dimension of column

47. What are IS criteria for pitch and diameter of lateral ties? (IS: 456, P. 49)

Maxi. Pitch:

- (i) Least lateral dimension
- (ii) 16 × smaller dia
- (iii) 300 mm, which ever is less

diameter:

(i)
$$\frac{1}{4}$$
 × dia. of larger bar

(ii) 6 mm, whichever is more.

(IS: 456, P. 48)

- (i) Maxi. $p_t = 6\%$, Mini. $p_t = 0.8 \%$
- (ii) Mini. no. of bars = 4 in square column

= 6 in circular column

- (iii) Mini, dia. of bars = 12 mm
- (iv) Spacing of bars along periphery shall not exceed 300 mm

48. What are the IS criteria for longitudinal bars in columns?

49. Give location of critical section for B.M., One way shear and two way shear in footing.

For B.M - at the face of column

For oneway shear - at d distance from the face of column

For two way shear – at $\frac{d}{2}$ distance from the face of column

Design of RCC Structures

221

50. Distinguish between one way and two way slab

| | One way slab | Two way slab | | |
|-------|--|--------------|---|--|
| (i) | $\frac{l_{y}}{l_{x}} \ge 2$ | (i) | $\frac{l_{y}}{l_{x}} < 2$ | |
| (ii) | Main steel is provided along short span (l_x) . Distribution steel is provided along l_y | (ii) | In both the directions main steel is provided | |
| (iii) | Slab deflect along short span | (iii) | Slab deflect along l_x and ly | |

51. Give IS criteria for maximum spacing between bars in slab.

For main steel:

(IS: 456, P.46)

- (i) 3d
- (ii) 300 mm, whichever is smaller

For distribution steel:

- (i) 5d
- (ii) 450 mm, whichever is smaller
- 52. Give IS criteria for mini. reinforcement in slab.

For Fe 250,

 $p_t min = 0.15 \%$ of gross area

For Fe 415, 500 p_t min = 0.12 % of gross area

(IS: 456, P. 48)

53. What is maxi. dia. of bar for slabs?

Maxi. dia. = $\frac{1}{8}$ × D, where, D = total thickness of slab

54. What is minimum dia. of bars for slabs?

For main bars:

Plain bars 10 mmф

deformed bars 8 mm b

For distribution bars:

Plain bars 6 mm ϕ

Deformed bars..... 6 mm\$\phi\$

55. How effective span for simply supported slab/beam is calculated?

(IS: 456, P.34)

- (i) clear span +d whichever is smaller
- (ii) c/c of supports

56. Give /d ratios for slab and beam for span up to 10 m, for deflection control.

Cantilever
$$\frac{l}{d} = 7$$
Simply supported $\frac{l}{d} = 20$

Continuous $\frac{l}{d} = 26$

57. Give $\frac{l}{d}$ ratio for two way slab for short span up to 3.5 m and live load up to 3 kN/m².

(IS: 456, P. 49)

(IS: 456, P. 37)

These values are for mild steel

For HYSD bars, these values are multiplied by 0.8

58. In two way slab where torsion reinforcement is provided?

At the corner where both edges are discontinuous, full torsion steel is provided At the corner where one edge is discontinuous, 50 % torsion steel is provided

59. How much steel is provided at corners for torsion?

75 % of the maximum mid span steel, in each of the 4 layers extending up to $\frac{l_x}{5}$ from the edge

60. How much steel is provided in edge strips of two way slab?

Mini. 0.12 % (For Fe 415)

Mini. 0.15 % (For Fe 250)

MCQ'S

| 1. | Minimum amount of area. | HYSD bar reinfor | rcem | ent in slab shall | be | of gross |
|-----|--|--------------------------------|-------|------------------------|-------|--------------------------------|
| | (a) 0.12 % (b) |) 0.15 % | (c) | 0.20 % | (d) | 0.08 % |
| 2. | All the axially loaded | | | | n ecc | entricity of |
| | |) 20 mm | | | | 40 mm |
| 3. | Basic value of span/d | epth ratio for limit | of d | eflection for a si | mply | supported slab |
| | having span up to 10 | | | | | |
| | (a) 7 (b) |) 10 | (c) | 20 | (d) | 26 |
| 4. | The partial safety fac | ctor for material for | r cor | icrete is | | |
| | (a) 1.5 (b) |) 1.15 | (c) | 2.0 | (d) | 2.5 |
| 5. | If the minimum dime | ension of element to | be c | east in PCC is 75 | mn | i, the maximum |
| | size of aggregate can | be | | | | |
| | |) 10 mm | | | | |
| 6. | A long column is one exceeds | whose ratio of effe | ctive | length to its lea | st la | teral dimension |
| | (a) 5 (b) | 10 | (c) | 12 | (d) | 20 |
| 7. | Tensile strength of co | oncrete is given by | | | | |
| | (a) $5000 \sqrt{f_{ck}}$ (b) | $0.7 \sqrt{f_{ck}}$ | (c) | $0.7 \times f_{ck}$ | (d) | $0.1 \times f_{ck}$ |
| 8. | Props for slab for minimum of | rmwork (<mark>s</mark> pan up | o to | 4.5 m) can | be r | emoved after |
| | (a) 3 days (b) | 7 days | (c) | 14 days | (d) | 21 days |
| 9. | The minimum eccentr | ricity in columns ca | n be | calculated as | | |
| | l = D | l D | | l D | | |
| | (a) $\frac{l}{500} + \frac{D}{20}$ (b) | $\frac{1}{300} + \frac{1}{20}$ | (c) | 500 + 30 | (d) | $\frac{1}{300} + \frac{1}{30}$ |
| 10. | The test result of indi | ividual concrete spe | ecime | en should not va | ry n | ore than |
| | (a) $\pm 5 \%$ (b) | | | | | |
| 11. | In case of doubt of co | | | | | |
| | (a) DL + LL | (| (b) | $1.5 \times (DL + LL)$ | ı | · |
| | (c) $DL + 1.5 LL$ | : | (d) | DL + 1.25 LL | | |
| 12. | Moist curing of concre | ete in case of concre | ete m | ade by OPC and | l PP | C shall be |
| | days and days re | | | - | | |
| | (a) 7, 14 (b) | 10, 14 | (c) | 7, 10 | (d) | 14, 21 |

| 13. | Two way slabs can be analysed using | | |
|-----|---|------------|---------------------------------------|
| | (a) Pigand's theory | (b) | Westergaurd's theory |
| | (c) Johnson's theory | (d) | All the above |
| 14. | Deflection of beam is usually limited to | | |
| | (a) span/350 | (b) | 20 mm |
| | (c) a or b whichever is less | (d) | a or b whichever is more |
| 15. | Maximum diameter of bar for slab thic | knes | s D is |
| | (a) $\frac{D}{4}$ (b) $\frac{D}{5}$ | | D D |
| | (a) $\frac{4}{4}$ (b) $\frac{5}{5}$ | (c) | $\frac{D}{6}$ (d) $\frac{D}{8}$ |
| 16. | Minimum and maximum % of steel in | colur | nn is |
| | (a) 0.8 %, 6 % (b) 0.6 %, 8 % | (c) | 0.12 %, 6 % (d) 0.15 %, 8 % |
| 17. | Drops are provided in flat slab to resis | t | |
| | (a) bending moment (b) thrust | (c) | shear (d) torsion |
| 18. | In RCC beam, side face reinforcement | t is p | provided if depth of web in a beam |
| | exceeds | | |
| | (a) 300 mm (b) 450 mm | (c) | 750 mm (d) 800 mm |
| 19. | The purpose of reinforcement in prestr | essed | l concrete is |
| | (a) to resist tensile stresses | | |
| | (b) To provide adequate bond stress | | |
| | (c) to impart initial compressive stress in | n con | crete (d) All of these |
| 20. | Half of the main steel in a simply support | | |
| | distance of x from the centre of suppor | t, wl | |
| | (a) $\frac{l}{3}$ (b) $\frac{l}{5}$ | (c) | $\frac{l}{7}$ (d) $\frac{l}{10}$ |
| | J | | , 10 |
| 21. | As per IS: 1343 - 1980, minim | | |
| | pre-tensioned concrete and post-tension | | |
| 22 | (a) M 20, M 30 (b) M 30, M 20 | | |
| 22. | For pre-stressed concrete, the creep coef | | |
| 22 | (a) 2.2 (b) 1.6 | | |
| 23. | As per IS: $456 - 2000$, concrete having | g cha | aracteristic strength varying between |
| | M 25 to M 55 is known as | <i>a</i> > | 64 1 1 |
| | (a) Ordinary concrete | (b) | Standard concrete |
| - 1 | (c) Nominal concrete | (d) | Normal concrete |
| 24. | Snow loads are accounted in design as | - | TO 0 M T (2) TV |
| | (a) IS: 875 (Part – I) | | IS: 875 (Part II) |
| | (c) IS: 875 (Part IV) | (d) | IS: 875 (Part V) |
| | | | |

| 25. | Fire resistance of RC struc | tural member de _l | pends on | |
|-----|---|------------------------------|---------------------|-------------------------|
| | (a) Size of member | | concrete cover | |
| | (c) Types of aggregate | , , | all of the above | |
| 26. | A compression member is co | alled pedestal if it | ts effective length | is less than |
| | times its least lateral dimer | | | |
| | (a) 2 (b) 3 | (c) | 4 | (d) 5 |
| 27. | The length of torsion reinfe | orcement in two | way slab is | |
| | (a) $\frac{l_x}{4}$ (b) $\frac{l_x}{5}$ | (c) | $\frac{l_x}{8}$ | (d) $\frac{l_x}{10}$ |
| 28. | The amount of reinforceme | | each of the four | layers shall be equal |
| | to of the maximum | mid span steel. | | |
| | (a) $\frac{1}{2}$ (b) $\frac{2}{3}$ | | 4 . | (d) $\frac{4}{3}$ |
| 29. | For RCC footing, which of | f the following sta | itement is correc | :t ? |
| | (a) Critical section for B.M. | 1. is taken at the f | ace of column. | |
| | (b) Critical section for one- | -way shear is take | n at distance d fro | m the face of column. |
| | (c) Critical section for two- | -way shear is taken | at distance d/2 fro | om the face of column. |
| | (d) all of these. | | | |
| 30. | For footing minimum cove | | | (1) 100 |
| | | | 75 mm | (d) 100 mm |
| 31. | The maximum distance be | tween distribution | n steel (temperat | ure steel) in a slab is |
| | limit <mark>ed to</mark> | | 71.000 | (1) 5d 450 mm |
| | (a) 3d, 300 mm (b) 3d | I, 450 mm (c) | 5d, 300 mm | (a) 5a, 450 mm |
| 32. | Bond stress of concrete | when steel | is embedded in G | compression zone. |
| | (a) reduces | , , | increases | ith location |
| | (c) remains unchanged | 1 | has no relation | With location |
| 33. | _ | | lepends | |
| | (a) Strength of concrete (b | , | meter of bar | |
| | (c) yield stress of steel (d | | | |
| 34. | - | | | 1 hors |
| | (a) HYSD bars | (b) | _ | |
| | (c) TMT bars | (d) |) Prestressing ste | ;61 |
| 35 | | | | on to correction |
| | (a) High tensile strength | (b) | | ce to corrosion |
| | (c) More ductility | (d |) All of these | |
| | | | | |

| 36. | Minimum number of bars in circular | column shall be |
|-----|--|---|
| | (a) 4 (b) 5 | (c) 6 (d) 8 |
| 37. | Spacing of longitudinal bars along the | e periphery of a concrete column is limited |
| | to | |
| | (a) 200 mm (b) 300 mm | (c) 16 × dia of bar (d) 150 mm |
| 38. | Minimum thickness of flat slab should | d be |
| | (a) 75 mm (b) 100 mm | (c) 125 mm (d) 150 mm |
| 39. | In case of flat slab, thickness of slab i | is increased around the column is known as |
| | (a) column head (b) Drop panel | (c) column strip (d) None of these |
| 40. | Edge thickness of a rectangular footing | ng resting on soil will not be less than |
| | (a) 100 mm (b). 150 mm | (c) 200 mm (d) 300 mm |
| 41. | The factor 0.67 introduced in concrete | e strength to account |
| | (a) Variation in concrete strength | (b) Variation in type of steel |
| | (c) Size effect | (d) Partial safety factor |
| 42. | A column is designed as axially loaded | d if minimum eccentricity is less than |
| | times lateral dimension. | |
| | (a) 0.1 (b) 0.05 | (c) 0.5 (d) 0.01 |
| 43. | Punching shear strength of concrete i | n limit state method is taken as |
| | (a) 0.7 $\sqrt{f_{ck}}$ (b) 0.25 $\sqrt{f_{ck}}$ | (c) $0.20 \sqrt{f_{ck}}$ (d) $5000 \sqrt{f_{ck}}$ |
| 44. | The effective height to thickness of R | |
| | (a) 6 (b) 20 | (c) 26 (d) 30 |
| 45. | An over reinforced beam fails by | |
| | (a) Crushing of concrete | (b) yielding of steel |
| | (c) Crushing of steel | (d) None of these |
| 46. | The width of middle strip in two way | slab is |
| | (a) $\frac{1}{2}$ of the span (b) | $\frac{3}{4}$ of the span |
| | (c) $\frac{1}{8}$ of the span (d) | $\frac{1}{4}$ of the span |
| 47. | Minimum grade of concrete in prestre | essing is |
| | (a) M 20 (b) M 25 | (c) M 30 (d) M 40 |
| 48. | In case of continuous prestressed bear | m, the cable is placed as |
| | (a) Straight (b) Parabolic | (c) eccentric straight (d) concordant |
| 49. | The loss of prestress in prestressed co | oncrete member usually varies between |
| | (a) 2 to 5 % (b) 6 to 12 % | (c) 12 to 20 % (d) 20 to 25 % |
| | | |

Design of RCC Structures

| 50. | The loss of prestress in pre-tensioned | l concr | ete due to shi | rinkage is |
|------------|---|-----------|------------------------------|--------------------------|
| | (a) 60 MPa (b) 63 MPa | (c) | 100 MPa | (d) 200 MPa |
| 51. | Maximum w/c ratio for RCC is | | | |
| | (a) 0.45 (b) 0.50 | (c) | 0.55 | (d) 0.60 |
| 52. | Minimum cement content for mild en | vironm | ent using 10 n | nm size aggregate, shall |
| • | be per m ³ of concrete. | | | |
| | (a) 300 kg (b) 320 kg | (c) | 340 kg | (d) 360 kg |
| 53. | In a designed mix, the quantity of cond | erete in | gredients is de | fined in terms of |
| | (a) Cement by weight and aggregate b | y volun | ne | (b) weight |
| | (c) volume (d) either of weig | ht or vo | olume can be t | aken |
| 54. | As per IS: 456 - 2000, in case of n | ominal | mix proporti | ioning of ingredients is |
| | based on | | | |
| | (a) volume (b) weight | (c) | density | (d) specific gravity |
| 55. | To get maximum negative bending mo | ment i | n a <mark>multi sp</mark> ai | n continuous beam, live |
| | load arrangement shall be | | | |
| | (a) Two adjacent span loaded | (b) | Spans random | ily loaded |
| | (c) Alternate spans loaded | | Can not be pr | |
| 56. | A beam is called a deep beam, if its | L/D r | atio is | in simply supported |
| | beam and in continuous beam | | | |
| | (a) 2, 2.5 (b) 2, 2 | | | (d) 1.5, 2.5 |
| 57. | Pitch of lateral ties in column shall b | e not n | ore than | |
| | (a) least lateral dimension | (b) | 16 × smaller | dia longitudinal bar |
| | (c) 300 mm | · (d) | all the above | |
| 58. | Diameter of lateral ties shall not be le | ess thar | 1 | |
| | (a) $\frac{1}{4}$ dia. of largest longitudinal bar | | (b) 6 mm | |
| | (c) $\frac{1}{4}$ × dia. of smallest longitudinal b | oar (d) | both (a) and (| b) above |
| 59. | Maximum spacing of vertical reinforc | ement i | in RCC wall | should not exceed |
| | (a) thickness of the wall | (b) | 1.5 times thic | kness of wall |
| | (c) 2.0 times thickness of wall | (d) | 3.0 times thic | kness of wall |
| 60. | The thickness of any part of shear wa | ıll shall | not be less t | han |
| | (a) 100 mm (b) 150 mm | (c) | 175 mm | (d) 200 mm |
| 61. | The minimum reinforcement % in each | h direc | ction for a sh | ear wall shall be |
| | (a) 0.12 % (b) 0.20 % | | 0.25 % | (d) 0.30 % |
| 52. | As per IS: 13920 - 1993 the lap length | of bars | in beam shall | |
| | (a) development length (b) | 150 1 | | |
| | (c) both (a) and (b) above | | 200 mm | |

| 220 | | | | | | | |
|-----|-------------------------------|---------|-----------------------|---------|------------------------|----------------------|---------------------|
| 63. | Lap splice in bear | m sha | ll not be provi | ded wi | thin | | |
| | (a) Joint | | | | | of 2d | from joint face |
| | (c) within a quar | ter lei | igth of the mem | ber | (d) all of the | se | |
| 64. | M _{ulim} for Fe 415 | | | | , | | |
| | (a) $M_{\text{ulim}} = 0.148$ | | _ | (b) | $M_{\rm ulim} = 0.133$ | 8 f _{ck} bd | 2 |
| | (c) $M_{ulim} = 0.133$ | | | (d) | $M_{\rm ulim} = 0.130$ | o f _{ck} bd | 2 |
| 65. | For Fe 415 grade | | | | | | |
| | (a) 0.53 d | | | (c) | 0.46 d | (d) | 0.33 d |
| 66. | Maximum compre | essive | stress in top m | ost fib | re of a simply | suppo | rted rectangular |
| | beam is | | | | | | |
| | (a) $0.36 f_{ck}$ | (b) | 0.446 f _{ck} | (c) | 0.50 f _{ck} | (d) | 0.6 f _{ck} |
| 67. | Maximum tensile | | | | | | |
| | (a) $0.446 f_{ck}$ | (b) | $0.87 f_{v}$ | (c) | $0.67 f_y$ | (d) | 0.33 f _y |
| 68. | Loss of stress wit | | | | | d | |
| | (a) relaxation | (b) | Creep | (c) | Shrinkage | (d) | Ductility |
| 69. | Minimum % of r | | | | | | tank as per IS |
| | 3370 (Part-II) is | | | | | | |
| | (a) 0.24 % | (b) | 0.12 % | (c) | 0.30 % | (d) | 0.20 % |
| 70. | Doubly beam is c | onsid | ered less econo | mical | than a singly | R.C. be | am because |
| | (a) tensile steel | | | | | | |
| | (c) Concrete is no | | | | | | |
| | | | | | | | |

: ANSWERS :

| 1. (a) | 2. (b) | 3. (c) | 4. (a) | 5. (b) |
|---------|---------|---------|---------|---------|
| 6. (c) | 7. (b) | 8. (b) | 9. (c) | 10. (c) |
| 11. (d) | 12. (b) | 13. (d) | 14. (c) | 15. (d) |
| 16. (a) | 17. (c) | 18. (c) | 19. (c) | 20. (c) |
| 21. (d) | 22. (b) | 23. (b) | 24. (c) | 25. (d) |
| 26. (b) | 27. (b) | 28. (c) | 29. (d) | 30. (b) |
| 31. (d) | 32. (b) | 33. (d) | 34. (b) | 35. (d) |
| 36. (c) | 37. (b) | 38. (c) | 39. (b) | 40. (b) |
| 41. (a) | 42. (b) | 43. (b) | 44. (d) | 45. (a) |
| 46. (b) | 47. (c) | 48. (b) | 49. (c) | 50. (a) |
| 51. (c) | 52. (a) | 53. (b) | 54. (a) | 55. (c) |
| 56. (a) | 57. (d) | 58. (d) | 59. (d) | 60. (b) |
| 61. (c) | 62. (c) | 63. (d) | 64. (b) | 65. (b) |
| 66. (b) | 67. (b) | 68. (b) | 69. (a) | 70. (d) |

EXPLANATIONS

5. (b) Maximum size of aggregate $=\frac{1}{4}$ D $=\frac{1}{4} \times 75$

IS: 456 P. 14

= 18.75 mm

- 10. (c) IS: 456 2000, P. 29
- 11. (d) IS: 456 2000, P. 31, cl. 17.6.2
- 12. (b) IS: 456 2000, P. 27
- 13. (d) IS: 456 2000, P. 41
- 14. (c) IS: 456 2000, P. 37
- 15. (d) IS: 456 2000, P. 48
- 16. (a) IS: 456 2000, P. 48
- 18. (c) IS: 456 2000, P. 47
- 22. (b)

| Age | Creep Coefficient (θ) |
|---------|-----------------------|
| 7 days | 2.2 |
| 28 days | 1.6 |
| 1 year | 1.1 |

- 23. (b) IS: 456 2000, P. 16
- 26. (b) IS: 456 2000, P. 41
- 30. (b) IS: 456 2000, P. 46
- 38. (c) IS: 456 2000, P. 53
- 40. (b) IS: 456 2000, P. 64
- 42. (b) IS: 456 2000, P. 71
- 43. (b) IS: 456 2000, P. 59
- 44. (d) IS: 456 2000, P. 61
- 50. (a) $\Delta \sigma = \varepsilon_{sh} \times E_{s}$ = $3 \times 10^{-4} \times 2 \times 10^{5}$ = 60 MPa
- 51. (c) IS: 456 2000, P. 20
- 52. (a) IS: 456 2000, P. 20
- 56. (a) IS: 456 2000, P. 51



Design of Steel Structures (AS per IS: 800 - 2007)

| C | 17 | Т | ١. | 1 |
|--------|----|---|----|---|
| 7 | H, | | - | ı |
| \geq | _ | _ | | _ |

| | | | | | | - | | | |
|-------|------|-----------------------------|--------|------------|-------------------------|------|----------------|-------|---------------------|
| (1) | The | e minimum pio | ch of | Bolt allo | wed in th | e c | ode is | | |
| | (a) | | | 2.0 · d | | | 2.5 · d | (d) | 3.0 · d |
| | | where d - Dia | amete | r of Bolt | | | | ` / | |
| (2) | Gra | de of 4.6 Bolt | has | nominal | ultimate si | tre | ss of | | |
| | (a) | 400 Mpa | (b) | 460 Mpa | ı (c |) | 240 Mpa | (d) | 600 Mpa |
| (3) | For | M20 Bolt, Di | amete | er of Hole | e will be o | f | | | |
| | (a) | 18 mm | (b) | 22 mm | (c |) | 21.5 mm | (d) | 23 mm |
| (4) | In o | louble Cover | Butt . | Joint, bol | t will be s | ub | jected to | | |
| | (a) | Single shear | | (b) | 1.5 × sing | gle | shear | | |
| | (c) | Double shear | | (d) | No shear | | | | |
| (5) | The | minimum siz | e of f | illet weld | is | | • | | |
| | | 3 mm | | 4 mm | (c) |) | 3.5 mm | (d) | 4 mm |
| (6) | | tial safety fact | or fo | r field we | eldin <mark>g is</mark> | | | | |
| | (a) | | . , | 1.25 | (c) |) | 1.50 | (d) | 2.5 |
| (7) | | imum end ret | | f weld is | | | | | |
| | | 2 × size of w | | | (b) |) | 3 × size of we | eld | |
| | | $2.5 \times \text{size of}$ | | | (d) |) | 4 × size of we | id | |
| (8) | | Type of weld | | | | | | | |
| | | Groove Weld | | _ | ٠, | | | | Fillet Weld |
| (9) | The | maximum sle | ndern | ess ratio | permitted | fo | r the member | whi | ch always remains |
| | | ension is | | | | | | | |
| (4.0) | (a) | 180 | ` ' | 240 | ` ' | | 350 | (d) | 400 |
| (10) | The | tensile stress i | n the | material: | | | | | astic stage will be |
| | | 1.5 × Average | | | | | 3.0 × Average | | |
| /4.4\ | (c) | 2.5 × Average | stres | s · | . (q) | 1 3 | 3.5 × Average | Stre | 3S |
| (11) | The | angle connec | ted t | o G.P. w | ith longer | · le | eg resist mor | e tei | nsile load than if |
| | | ected by Shor | ter le | g to G.P. | | | | | |
| | rue. | /False | | | (St | rik | e out which or | ne is | false) |

| Des | ign of Steel Str | uctures | | 23 | 1 |
|------|------------------------|---|--|-------------------------|-----|
| (12) | | | G.P. by welding rennected with G.P. by | esist more Tension Fore | ce |
| | True/False | one same angle is co | (Strike out which | | |
| (13) | | oles providad on air | | resist same tension for | |
| (20) | | igles are provided o | | resist same tension for | ce |
| | True/False | | (Strike out which | h one is false) | |
| (14) | In equation, T. | $d_n = \alpha \cdot A_n \cdot f_u / \gamma_{m1}, \alpha$ | x = 0.6 can be taken | if | |
| | (a) One or two | o Bolts are provided | (b) Three bo | olts are provided | |
| | (c) Four or mo | ore bolts are provided | l (d) Welding | is provided | |
| (15) | If ISA 75×60 |) × 6 is connected t | o G.P. by longer leg | by welding then area of | o f |
| | connected leg v | | | . • | |
| | (a) 432 mm^2 | (b) 450 mm^2 | (c) 360 mm ² | (d) 342 mm^2 | |
| (16) | If five bolts of | 20 mm dia. are pr | ovided with a pitch | of 50 mm then length of | of |
| | connection will | | | | |
| | (a) 100 mm | (b) 200 mm | (c) 250 mm | (d) 170 mm | |
| (17) | As per IS 800 | | | at one end and hinge a | at |
| | other end is | 360 | | | - |
| | (a) 0.65 · L | (b) 0.8 · L | (c) 1.0 · L | (d) 1.5 · L | |
| (18) | The maximum | slenderness ratio pe | rmitted for strut is | | |
| | (a) 180 | (b) 250 | (c) 350 | (d) 400 | |
| (19) | A compression | member in a roof t | russ is normally trea | ` ' | |
| | (a) Column | | | (d) Main Tie | |
| (20) | The imperfection | on factor for bucklin | | G/A72 | |
| | (a) 0.21 | (b) 0.34 | (c) 0.49 | (d) 0.76 | |
| (21) | The effective le | ` ' | ` ' | placed back to back or | n |
| ` , | each side of G. | | angles | proced back to back of | |
| | (a) 0.7 L | (b) 0.85 L | (c) 0.75 L | (d) . 1.2 L | |
| | | s ratio of lacing shal | | (4) . 1.2 5 | |
| | (a) 180 | (b) 145 | (c) 200 | (d) 140 | |
| | ` ' | ` ' | st transverse shear - | ` ' | |
| | | xial Load on Column | | Axial Load on column | |
| | | xial Load on column | ` ' | | |
| | | | Ith of Lacing Bar sh | Axial Load on Column | |
| | (a) 40 mm | | | | |
| | • • | (b) 50 mm | (c) 60 mm | (d) 80 mm | |
| | | d slenderness ratio of | vattened column | shall be times the | e |
| | (a) 1.05 | | (c) 1.20 | (d) 1.25 | |
| | , | 102 1.10 | 1147 1 737 | 111 1 / 3 | |

| (26) | The Main beams are always supported | on - | | | | | | | | |
|------|---|--|--|--|--|--|--|--|--|--|
| | (a) Secondary Beams (b) One secondary beam and on one other main beam | | | | | | | | | |
| | (c) Columns (d) Cantilever | | | | | | | | | |
| (27) | A beam with a number of regular ope | | | | | | | | | |
| | (a) Hybrid Beam (b) Castellated Beau | n (c) Tapered Beam (d) Latticed Beam | | | | | | | | |
| (28) | In case of laterally supported beam | | | | | | | | | |
| | (a) Compression flange is restrained | (b) Tension flange is restraine | | | | | | | | |
| | (c) Web is Restrained | (d) Web and Tension flange is restrained | | | | | | | | |
| (29) | Shear buckling of web occurs if d/tw | | | | | | | | | |
| | (a) 42 c | (c) 84 ε (d) 105 ε | | | | | | | | |
| (30) | Which type of cross section gives an i | deal behavior as a beam | | | | | | | | |
| | (a) Plastic (b) Compact | (c) Semi compact (d) Slender | | | | | | | | |
| (31) | The buckling class of angle section is | | | | | | | | | |
| | (a) a (b) b | (c) c (d) d | | | | | | | | |
| (32) | | in resistance, governed by yielding, γ_{m0} is | | | | | | | | |
| | (a) 1.25 (b) 1.10 | (c) 0.9 (d) 1.20 | | | | | | | | |
| (33) | | resistance, governed by ultimate stress, γ_{m1} is | | | | | | | | |
| | (a) 1.25 (b) 1.10 | (c) 0.9 (d) 1.20 | | | | | | | | |
| (34) | Design strength of tension member is | taken as greatest of Tdg, Tdn and Tdb. | | | | | | | | |
| | True/False | (Strike out which one is false) | | | | | | | | |
| (35) |) In equation of rupture strength of a | n angle in tension, the term β will have a | | | | | | | | |
| | value always greater than or equals t | 0 U./. | | | | | | | | |
| | True/False | (Strike out which one is false) | | | | | | | | |
| (36) | | of Tension Member, the term Avg stands for (b) Maximum gross area of shear | | | | | | | | |
| | (a) Minimum gross area of shear | (d) Net area of shear | | | | | | | | |
| | (c) Minimum Tension area | | | | | | | | | |
| (37 | 7) As per IS - 800 - 2007, fcd stands for | (b) Design Compressive stress | | | | | | | | |
| | (a) Design Tensile stress | (d) Design Torsion Stress | | | | | | | | |
| | (c) Design Bending stress | • • | | | | | | | | |
| (38 | | of Lacing and Battening in Built up Column | | | | | | | | |
| | is not permitted. | (Strike out which one is false) | | | | | | | | |
| (3.0 | True/False | f beam, Md = $\beta_b Z_p f_y / \gamma mo$, β_b can be taken | | | | | | | | |
| (35 | equals to for plastic and compa | act sections. | | | | | | | | |
| | 43 105 | (c) 1.1 (d) 1.0 | | | | | | | | |
| | (a) 0.75 (b) 1.25 | (-) | | | | | | | | |

| | | : ANSWERS: | <u> </u> | |
|--|----------------------------------|--|-----------------------------------|---|
| 1. (c) | 2. (a) | 3. (b) | 4. (c) | 5. (a) |
| 6. (c) | 7. (a) | 8. (a) | 9. (d) | 10. (b) |
| 11. True | 12. True | 13. True | 14. (a) | 15. (a) |
| 16. (b) | 17. (b) | 18. (a) | 19. (c) | 20. (c) |
| 21. (b) | 22. (b) | 23. (a) | 24. (c) | 25. (b) |
| 26. (c) | 27. (b) | 28. (a) | 29. (b) | 30. (a) |
| 31. (c) | 32. (b) | 33. (a) | 34. False | 35. True |
| 36. (a) | 37. (b) | 38. False | 39. (d) | 40. (a) |
| 41. False | 42. True | 43. (c) | 44. (d) | 45. False |
| 46. True | 47. (d) | 48. True | 49. True . | 50. (a) |
| 26. (c) 31. (c) 36. (a) 41. False | 27. (b) 32. (b) 37. (b) 42. True | 28. (a) 33. (a) 38. False 43. (c) | 29. (b) 34. False 39. (d) 44. (d) | 30. (a) 35. True 40. (a) 45. False |

Objective Civil Eng. \setminus 2016 \setminus 30

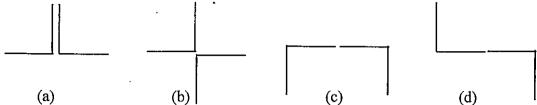
<u>SET-2</u>

| 1. | 'Lug angles' are | · | | | | | | | |
|-----|---|--|--|--|--|--|--|--|--|
| | (a) equal angles | | | | | | | | |
| | (b) equal angles with elliptical perforations | | | | | | | | |
| | (c) unequal angles with elliptical per | rforations | | | | | | | |
| | (d) Angle used to reduce the length | of connection | | | | | | | |
| 2. | In case of lug angles, the strength o | f lug angle should be more than the | | | | | | | |
| | force in the outstanding leg of the r | | | | | | | | |
| | (a) 10 % (b) 20 % | (c) 40 % (d) 50 % | | | | | | | |
| 3. | | nain angle shall be capable of developing a | | | | | | | |
| | strength not less than in ex | xcess of the force in the outstanding leg of | | | | | | | |
| | the angle member. | (N) 50 B/ | | | | | | | |
| | (a) 10 % (b) 20 % | (c) 40 % (d) 50 % | | | | | | | |
| 4. | Bolts are most suitable to carry | | | | | | | | |
| | (a) Shear (b) bending | | | | | | | | |
| 5. | | plate is 10 mm, then the maximum pitch of | | | | | | | |
| | rivets in tension will be taken as | (1) 000 | | | | | | | |
| | (a) 120 mm (b) 160 mm | | | | | | | | |
| 6. | | versal of stresses, the most suitable type of | | | | | | | |
| | bolt is | | | | | | | | |
| | (a) black bolt | (b) ordinary finished bolt | | | | | | | |
| | (c) turned and fitted bolt | (d) high strength bolt | | | | | | | |
| 7. | Which of the following sections proceurs? | referably be used at places where torsion | | | | | | | |
| | (a) angle section | (b) channel section | | | | | | | |
| | (c) box section | (d) Any of above | | | | | | | |
| 8. | The capacity of short column depen | nds on | | | | | | | |
| | (a) yield stress | (b) cross-sectional area | | | | | | | |
| | (c) Slenderness ratio | (d) both a and b | | | | | | | |
| 9. | Density of steel is taken as | | | | | | | | |
| | (a) 2500 kg/m^3 (b) 1600 kg/m^3 | . (c) 7850 kg/m^3 (d) 800 kg/m^3 | | | | | | | |
| 10. | Which of the following section is m | nost efficient column section? | | | | | | | |
| | | | | | | | | | |
| | (a) (b) | (c) (d) | | | | | | | |

| 11. | For design of compression member | | | | | | | | | | |
|-----|----------------------------------|---|-------------|---------|---------------|--------|----------------------------|-----------|--------|----------|----------|
| | (a) | (a) Slenderness ratio should be minimum (b) radius of gyration should be large | | | | | | | | | |
| | (c) | (c) both (a) and (b) (d) none of these The angle of inclination of the lacing bar with longitudinal axis of the column | | | | | | | | | |
| 12. | The | angle of inclina | tion of | the la | cing | bar ' | with longitu | ıdinal a | axis o | if the c | olumn |
| | shou | ıld be between | | | | | | | . 45 | | 700 |
| | (a) | 10° to 30° | (b) 30° | ° to 80 |)° | (c) | 40° to 70° | | (d) | 20° to | /0° |
| 13. | The | most efficient a | nd eco | nomic | al sec | ction | used as a b | eam is | | | .• |
| | (a) | I-section | (b) H- | section | ı | (c) | angles | (d | l) Cir | cular se | ection |
| 14. | Bea | ring stiffners ar | e provi | ded to | plat | e gir | der at | | | | |
| | (a) | Support | | | (b) | Poin | t of applicat | tion of | conce | entrated | load |
| | (c) | both (a) and (b) |) | | (d) | none | e of these | | | | _ |
| 15. | In o | case of design of | f load o | carryii | ng sti | ffner | s the disper | rsion of | f the | load t | arough |
| | web | is taken at an | angle o | f | | | | | | | |
| | (a) | 30° | (b) 45 | 0 | | (c) | 60° | | (d) | | |
| 16. | Ma | ximum deflection | n for s | imply | supp | orted | l steel <mark>be</mark> am | for el | astic | claddir | ig is |
| | | $\frac{L}{150}$ | | _ | | (0) | L | | (d) | L | |
| | (a) | 150 | (b) 18 | 30 | | (c) | 240 | | (4) | 300 | |
| 17. | An | gle purlin can b | e used | in roo | f tru | ss wl | ien angle of | f roof t | russ | is | 0 / 700 |
| | (a) | less than 30° | (b) m | ore tha | in 30° | (c) | less than 45 | 6° (d) | betv | veen 40 | ° to 70° |
| 18. | Fig | . shows the typic | cal sect | ion of | a gar | atry s | girder. The | functio | n of | top cha | innel is |
| | to i | inc <mark>re</mark> ase | | | | | | | | | 7 |
| | (a) | moment of iner | rtia abo | ut vert | ical a | ixis | | | | | |
| | (b) | torsional stiffne | SS | | | | | | | | |
| | (c) | Lateral bucklin | g strens | gth | | | | | | | |
| | (d) | All of these | | | | | | | | | l |
| 19. | | e effective slende | erness | ratio (| kL/r) | e of | laced colur | nn shal | ll be | taken a | as |
| 17 | tin | nes the (kL/r) _o th | he actu | al ma | ximu | m sle | nderness ra | itio. | | | |
| | (a) | 1.0 | (b) 1 | .05 | | (c) | 1.10 | | | 1.25 | |
| 20 | . Th | e effective slend | lerness | ratio | (k L/ | r) o | f battened | column | sha | ll be t | aken as |
| | | times the (k | $(L/r)_0$ t | he act | ual s | lende | erness ratio. | ı | | | |
| | | 1.0 | (b) 1 | | | (c) | 1.10 | | (d) | 1.25 | |
| 21 | . A | steel beam supp | | | from | floor | r slab as we | ell as fr | om v | vall is | called |
| | (a) | m | | | | (c) | Spandrel b | eam | (d) | heade | r beam |
| 22 | | the plastic desig | | | | | | | | | |
| | | 1.5 | | | | | 2.0 | | (d) | 2.5 | |

| 23. | In c | olumns splices should be provided | d at | | | |
|---|--|--|---------|----------------------|-------------------|--|
| | (a) | Floor levels | (b) | | | |
| | (c) | beam- column joints | (d) | | | |
| 24. | In p | n plastic analysis for flexure, which of the following pairs of shape and shape | | | | |
| | | ors are correctly matched? | | | (IES) | |
| | 1. | I 1.4 | 2. | Square 1.5 | | |
| | 3. | Rectangle 1.5 | 4. | Circle 1.7 | | |
| | Sele | elect the correct answer using the codes given below: | | | | |
| | (a) | 1, 2 and 3 (b) 2, 3 and 4 | (c) | 3 and 4 | (d) 1 and 2 | |
| 25. | Plas | tic hinge normally formed at | | | | |
| | (a) | fixed end | (b) | where B.M. is max | ximum | |
| | (c) | below point load | (d) | all the above | | |
| 26. | A ductile structure is defined as one for which the plastic deformation before | | | | | |
| | frac | ture | | | (IES) | |
| | (a) | is smaller than the elastic deforma | ition | | | |
| | (b) | vanishes | | | | |
| | (c) | is equal to the elastic deformation | | | | |
| (d) is much larger than the elastic deformation | | | | | | |
| 27. High yield deformed bars have a | | | | | | |
| | (a) definite yield value (b) Chemical composition different from mild steel | | | | | |
| | | | | | | |
| | (c) | | | | | |
| | (d) | (d) Percentage elongation more than that of mild steel | | | | |
| 28. | The | The height at which wind force acts on a moving vehicle on a bridge deck is (IES) | | | | |
| | | 42 15 | (a) | 1.7 m | (d) 2.0 m | |
| • | (a) | 1.2 m (b) 1.5 m | (c) | | S(// //) | |
| 29. | | Area of openings for buildings of large permeability is more than (a) 10 % of wall area (b) 20 % of wall area | | | | |
| | (a) | - 0 0 / 0 11 | (b) | 50 % of wall area | 1 | |
| 20 | ` ' | (4) | | | | |
| 30 | | ntry girders are designed to resis | ı | (b) longitudinal and | d vertical loads | |
| | | (d) lateral and longitudinal loads | | | | |
| 21 | (c) | (c) lateral, longitudinal and vertical loads (d) lateral and longitudinal loads As per IS: 875 (part - 3), a structure is classified as class 'A' if greatest hori- | | | | |
| 51 | | zontal or vertical dimension is | | | | |
| | | (d) none of these | | | | |
| 22 | (a) less than 20 m (b) 20 m to 50 m (c) greater than 50 m (d) 1 32. The topography factor k₃ for site when upwind slope (θ) is less to the content of the content o | | | | s less than 3° is | |
| 32 | | | (c) | 2.0 | (d) 2.5 | |
| | (a) | 1.0 | (-) | | | |

| 33. | Which of the following four sh | apes for a | compound | steel column | of the same |
|-----|---------------------------------|------------|--------------|---------------|-------------|
| | effective height formed with tw | vo equal a | ngles, has l | nighest axial | compression |
| | load carrying capacity? | | | | ~ |



- 34. If a structure is under fatigue stresses then the welded joints as compared to the riveted joints will fail
 - (a) earlier
- (b) later
- (c) at the same time
- (d) not at all
- 35. Generally for structural steel connections, the process of welding adopted is
 - Carbon arc welding
- (b) Oxy-hydrogen welding

(c) Pressure welding

- (d) metal arc welding
- 36. In roof truss for principal rafter most frequently used section is
 - Two channels placed at a distance apart
 - Two channels placed back to back
 - two angle section placed back to back
 - four angle section
- 37. The structure made of rigid curved surfaces are known as
 - Surface structure

(b) framed structure

shell structure (c)

- Space structure
- 38. Generally, in a simply supported truss the principal rafter will carry
 - Compressive forces

- (b) tensile forces
- Sometimes compressive and sometimes tensile forces
- (d) Shear forces

- 39. In the fillet weld the weakest section is
 - Smaller side of fillet
- (b) throat of the fillet
- Side perpendicular to force
- (d) Side parallel to force
- 40. In plate girder, a web should be checked for shear buckling if

- (a) $\frac{d}{tw} > 67 \epsilon$ (b) $\frac{d}{tw} \le 67 \epsilon$ (c) $\frac{d}{tw} > 90 \epsilon$ (d) $\frac{d}{tw} > 200 \epsilon$
- 41. For a thickness of a thicker plate t = 12 mm the minimum size of fillet weld required is
 - (a) 3 mm
- (b) 5 mm
- (c) 6 mm
- (d) 8 mm
- 42. For fillet weld, minimum throat thickness of weld is taken as
 - (a) 0.5 S
- (b) 0.7 S
- (c) 0.75 S
- (d) 0.80 S

| 238 | | | 2 - 1 | Talina ša |
|-----|---|-----------|-------------------------------------|-----------------------|
| 43. | The design strength of fillet weld for | $f_u = 4$ | $110 \text{ N/mm}^2 \text{ and so}$ | (d) None of these |
| | (a) 158 N/mm ² (b) 189 N/mm ² | (6) | 120 14/11111 | (d) None of those |
| 44. | The design tensile strength of a men | aber (1 | d) is taken as | |
| | (a) design strength due to yielding of | gross | section | |
| | (b) design strength due to rupture of | critical | section | |
| | (c) design strength due to block shear | ır | | |
| | (d) smaller of (a), (b) and (c) | | | |
| 45. | The hest cross section is | | | (1) alandor |
| | (a) Plastic (b) compact | (c) | semi-compact | (d) slender |
| 46. | For gantry girder economic depth o | f girde | er is taken as | т |
| | L | (c) | L | (d) $\frac{L}{20}$ |
| | (a) $\frac{L}{10}$ (b) $\frac{L}{12}$ | (0) | 15 | 20 |
| 47. | For the same depth, the heaviest I-s | section | is | (D) ISWB |
| | (a) ISLB (b) ISHB | (c) | ISMB | (D) 15 W D |
| 48. | In case of rolled steel beams, shear | force | is resisted by | |
| | (a) Web only | (b) | flange omy | |
| | (c) Web and flanges together | (d) | None of these | |
| 49 | a de and used in a roof truss f | or cont | necting | (1) mana of the above |
| | (h) main ties | (C) | Principal rations | d) none of the above |
| 50 | The send to connect | the co | mponents of built | up column are de- |
| | signed to resist | | | |
| | (a) longitudinal shear only | | transverse shear | omy |
| | (c) bending moments and shear due | e to tra | nsverse shear | |
| | (d) none of the above | | | |
| | : AN | SWER | | V /5 |

| | | : ANS | WERS |
|--|-----|-------|------|
| | (1) | 1 2 | (0) |

| | | | 4 (4) | 5. (b) |
|---------|---------|---------|---------|---------|
| 1. (d) | 2. (b) | 3. (c) | 4. (d) | |
| | | 8. (d) | 9. (c) | 10. (a) |
| 6. (d) | 7. (c) | | | 15. (b) |
| 11. (c) | 12. (c) | 13. (a) | 14. (c) | |
| 1 '' | | 18. (d) | 19. (b) | 20. (c) |
| 16. (c) | 17. (a) | , - | ` ′ | \ ' |
| 1 | 22. (b) | 23. (d) | 24. (b) | 25. (d) |
| 21. (c) | ` ' | 1 | 29. (b) | 30. (c) |
| 26. (d) | 27. (c) | 28. (b) | ` ' | 1 . '' |
| 1 | 32. (a) | 33. (b) | 34. (a) | 35. (d) |
| 31. (a) | 1 | 1 | 39. (b) | 40. (a) |
| 36. (c) | 37. (c) | 38. (a) | · · | 1 ' ' |
| | 1 | 43. (b) | 44. (d) | 45. (a) |
| 41. (b) | 42. (b) | 1 ' ' ' | 1 | 50. (c) |
| 46. (b) | 47. (b) | 48. (a) | 49. (a) | Ju. (c) |
| 40. (0) | 1111 | 1 | | |

7.

Soil Mechanics and Foundation Engineering

MCQ'S

| . Water | : Origin and N | | | | |
|----------|--------------------------|--------------------------------------|----------|--|--|
| (a) Col | | | | | |
| | leposited soils are call | led: | | | |
| (a) Res | | (1) 1 | | | |
| . Cohesi | onless soils are forme | d due to: | | | |
| | sical disintegration | (b) Chemical decomposition | | | |
| (c) Ox | | (d) Hydrati <mark>on</mark> | | | |
| | llowing type of soil is | not glair-deposited: | | | |
| (a) Dr | | (c) outwash (d) Loess | | | |
| | r deposited soils are c | ealled : | | | |
| (a) Tai | us (b) Loess | (c) Drift (d) None of about | ve | | |
| . Which | soils have higher shr | inkage and swelling characteristics? | | | |
| | hesionless soils | (b) Black cotton soils | | | |
| (c) La | terites | (d) Residual soils | · · | | |
| 7. Soils | ransported and depos | sited by gravity are called: | | | |
| (a) Ta | | (c) Loess (d) Aeoline | | | |
| 8. Major | part of Maharashtra | is covered by: | | | |
| | uvial soils | (b) Black cotton soils | | | |
| • | olian soils | (d) Marine soils | | | |
| | | : ANSWERS : | | | |
| | | (4) (4) (5) (6 | .—), | | |
| (1) (d), | (2) (b), | (3) (a), (7) (-7) | | | |
| (6) (b), | (7) (a), | (8) (b). | | | |

SET-2: Index properties and Relationships

| 1. | | sity is defined a | | | 37 | V |
|----|-------|--------------------------------|-------------------------|------------------|---------------------------------|-----------------------------|
| | (a) | $n = \frac{V_{v}}{V_{s}}$ | (b) $n = \frac{V}{V}$ | $\frac{7}{}$ (c) | $n = \frac{V_s}{V_v}$ | (d) $n = \frac{V}{V_v}$ |
| 2. | Voil | ratio is defined | as | | | |
| | | $e = \frac{V_v}{V_s}$ | | √ | v _v | (d) $e = \frac{V}{V_v}$ |
| 3. | In o | ver drying metl | hod for de | termination o | f water content th | e sample is kept at |
| | | temperature | for 24 hou | ırs. | | |
| | (-) | 20000 | (b) $105 -$ | - 110°C (c) | > 200°C | (d) None of these |
| 4. | | method of | determina | tion of water | content can be used | i only when specific |
| | grav | vity of soil solid | s is known | in advance. | | |
| | | Pycnometer me | | (b) | | |
| | | Sand both meth | | (d) | Oven drying method | od |
| 5. | ***** | is called fat | ther of soi | l mechanics. | | (1) (1) |
| | | Rankine | (b) Darc | y (c) | Karl Terzaghi | (d) Casagrande |
| 6. | For | fully dry soil, | degree of s | saturation is | | |
| | (a) | 0 | (b) 1 | (c) | 100 | (d) ∞ |
| 7. | For | fully saturated | soil, degr | ee of saturati | on is | |
| • | (a) | | (b) 1 | (c) | 100 | (d) ∞ |
| 8. | | id ratio 'e' for s | soils lies b | etween | | |
| 0. | (0) | 0 to 1 | (b) 1 to | 2 (c) | 0 to ∞ | (d) 0 to 0.1 |
| 9. | Hie | ther the specific | gravity o | f soil particle | s, more is th | ie particles. |
| | (a) | thicker | (b) dens | ser (c) | lighter | (d) none of these |
| 10 | Sn: | ecific gravity of | soil mass | is the ratio o | f | |
| 10 | | $\gamma/\gamma_{\rm w}$ | (b) γ _w /γ | (c) | $\gamma_{\rm s}/\gamma_{\rm w}$ | (d) $\gamma_{\rm w}/\gamma$ |
| 11 | | rcentage air voi | ds (n _a) is | defined as | | |
| 1, | (a) | $\frac{V_a}{V_v}$ | (b) $\frac{V_a}{V}$ | (c) | $\frac{V_a}{V_w}$ | (d) $\frac{V_{v}}{V}$ |
| 12 | 2. Ai | r content (a _c) is | defined a | s | | |
| | | $\frac{V_a}{V}$ | (b) $\frac{V_a}{V_v}$ | | $\frac{V}{V_a}$ | (d) $\frac{V_v}{V_a}$ |

13. Density index is the ratio of

(a)
$$I_D = \frac{e_{max} - e}{e_{max} - e_{min}}$$

(b)
$$I_D = \frac{e_{max} - e_{min}}{e_{max} - e}$$

(c)
$$I_D = \frac{e - e_{max}}{e_{max} - e_{min}}$$

(d)
$$I_D = \frac{e_{max} - e_{min}}{e_{max} - e}$$

14. Generally for soils density index (ID) lies between

- (a) < 0
- (b) > 100 %
- (c) 0 to 100 %
- (d) ∞

15. On site bulk density of cohesionless soils can be determined by

- (a) Core cutter method
- (b) Sand replacement method
- (c) Calcium carbide method
- (d) Pycnometer method

16. On site bulk density of cohesive soils is determined by

- (a) Core cutter method
- (b) Sand replacement method
- (c) Calcium carbide method
- (d) Pycnometer method

17. Which type of soil is not glacier deposited

- (a) Drift
- (b) bentonite
- (c) till
- (d) none of these

18. Cohesionless soils are formed due to

(a) Oxidation

- (b) hydration
- (c) Physical disintegration
- (d) Chemical decomposition

19. The correct ascending order for following densities is

1. Bulk density

2. Dry density

3. Saturated density

4. Submerged density

- (a) 1, 2, 4, 3
- (b) 1, 2, 3, 4
- (c) 4, 2, 1, 3
- (d) 4, 2, 3, 1

20. The value of specific gravity for most of the soils lies around

- (a) 1.0
- (b) 2.0
- (c) 2.68
- (d) 3.1

: ANSWERS:

| 1. (b) | 2. (a) | 3. (b) | 4. (a) | 5. (c) |
|---------|---------|---------|---------|---------|
| 6. (a) | 7. (b) | 8. (a) | 9. (b) | 10. (c) |
| 11. (b) | 12. (b) | 13. (a) | 14. (c) | 15. (b) |
| 16. (a) | 17. (b) | 18. (c) | 19. (c) | 20. (c) |

SET - 3 : General

| ι. | Consistency of soil is used to describe | firm | ness of soil | ls related to water |
|-----|--|---------|---------------------------------|---------------------------------|
| | content. | | 1 | |
| | (a) Coarse grained soils (b) | | grained soils | |
| | (c) Coarse sand | | Fine sand | |
| 2. | For determination of liquid limit in la | abora | itory the soil samp | le passing through |
| | IS sieve is used. | | | |
| | (a) 75μ (b) 150μ | (c) | 425 μ | (d) 600 μ |
| 3. | In plastic limit test the soil ball is roll | led o | n a glass plate into | a thread of |
| • | mm diameter. | | | |
| | (a) 2 mm (b) 3 mm | (c) | 6 mm | (d) 10 mm |
| 4. | For uniformly graded soil the uniform | ity c | oefficient (C _u) is | |
| | (a) 1 (b) 4 | (c) | 6 | (d) 8 |
| E | For gravels uniformity coefficient (Cu | | | |
| 5. | (1) 0 | (c) | greater than 4 | (d) greater than 6 |
| _ | (a) 1 (b) 2 The ratio of unit weight of soil solids | to th | at of water is calle | ed |
| 6. | (a) Water content | (b) | Specific gravity | |
| | | | | |
| _ | (c) degree of saturation (d) The relation between void ratio (e) ar | | | |
| 7. | The relation between void ratio (c) and | 40 | | 1 |
| | (a) $n = \frac{1+e}{e}$ (b) $n = \frac{e}{1+e}$ | (c) | $e = \frac{n}{1}$ | (d) $e = \frac{1 - 11}{1 - 11}$ |
| | (a) $n = \frac{}{}$ (b) $1 + e$ | (0) | I+n | n |
| 8. | The relation between the air content | (a_c) | and degree of satur | ation (S _r) is |
| 0. | (a) $a_c = 1 + S_r$ (b) $a_c = 1 - S_r$ | (c) | $a_c = 1/S_r$ | (d) None of these |
| 9. | The approximate void ratio in sandy | soil i | is | |
| 7. | (a) 0.2 (b) 0.6 | (c) | 0.8 | (d) 1.2 |
| 10 | The clay mineral has high swe | elling | and shrinkage cha | racteristics. |
| TO. | (a) Kaolinite (b) Illite | (c) | Montmorillonite | (d) None of these |
| 44 | . As per Atterberg, if the value of plant | astici | ty index is greater | than 17, the soil is |
| ΙĮ | As per Atterberg, if the value of pro- | | : | |
| | classified as (a) low plastic (b) non plastic | (c) | Medium plastic | (d) highly plastic |
| | () | ` ' | • | |
| 12 | | (b |) Cohesive fine gra | ined soil |
| | (a) Cohesive coarse grained soil | (d | · | |
| | (c) non-cohesive coarse grained soil | • | , hon concernation | |
| 13 | 2 | 111 |) N/m ² | (d) N/m ³ |
| | (a) kg/m^2 (b) kg/m^3 | (C |) 18/111 | (-) |
| | | | | |

| | | | | | | •• | | |
|-----|-----------------|------------------------------|--------|-------------------------------|--------|---------------------------------------|-------------|-------------------------|
| 29. | As p | oer IS soil classif | icati | on system the | | l particles size rang | ges fi | rom |
| | (a) | $75~\mu-4.75~mm$ | | | | 4.75 mm - 20 mm | | |
| | | $2\mu - 75\mu$ | | | • | 2 mm - 20 mm | ٠ | |
| 30. | A so | oil having liquid | limit | t 60 % and pl | astici | ty index 40 % can | | |
| | (a) | CL | (b) | CH | (c) | SM | (d) | CL – ML |
| 31. | Stok | ke's law is used t | o de | termine the | | | | |
| | (a) | Specific gravity | of so | oil solids | | | | |
| | (b) | density of soil s | | | | | | |
| | (c) | Grain size distrib | ution | of those soils | whos | e grain size is finer t | han (|).075 mm |
| | (a) | all of the above | | | | | | |
| 32. | The | effective size of | a so | | | | | |
| | | D ₁₀ | (b) | 20 | | | (d) | D ₆₀ |
| 33. | The | uniformity coef | ficie | nt of soil is de | fined | | | _ |
| | | $Cu = \frac{D_{40}}{D_{10}}$ | 71.5 | $\underline{\mathrm{D}_{40}}$ | (0) | $\frac{D_{60}}{D_{10}}$ | (A) | $\frac{D_{50}}{D_{50}}$ |
| | | 10 | | | | 10 | | - 10 |
| 34. | As | per IS soil classif | icati | on system, wh | en lic | luid limit lies betwe | een 3 | 5 % and 50 % |
| | | given soil is clas | | | | | | |
| | (a) | | | | | Soil with medium of | comp | ressibility |
| | (c) | Soil with high o | omp | ressibility | (d) | None of these | | |
| 35. | IS _I | plasticity chart is | a p | lot of | | 778 | | |
| | | | | | | $I_f v_s W_L$ | (d) | $W_L V_s W_p$ |
| 36. | Cas | agrande equation | n for | Ip – A line is | s · | | . 1. 6 | 70 (III 10) |
| | (a) | $0.63 (W_L - 20)$ | (b) | $0.73 (W_L - 20)$ |) (c) | $0.63 \text{ (W}_{L} - 10) \text{ (}$ | d) 0. | $(W_L - 10)$ |
| 37. | | per IS classificat | | silt size is | | | | |
| | (a) | 0.075 to 4.75 m | m | | | 0.002 to 0.075 mm | | |
| | | < 0.002 mm | | alivii, i | ` ' | > 4.75 mm | | |
| 38. | | per IS classificat | | | | | <i>(</i> 1) | 0.14 |
| | | *Śilty clay | | | | | (d) | Silty sand |
| 39. | Ger | nerally honeycom | | | | | <i>(</i> 1) | o to |
| | (a) | | | Clayey | ٠, | Gravely | • / | Sandy |
| 40. | Ger | nerally flocculate | | | | ed structure is obse | ervec | i in deposits. |
| | (a) | Silt | ` ' | Clay | | Sand | | gravel |
| 41. | Ge | nerally the mont | | | | ry constituent for. | | |
| | (a) | Gravelly | | Sandy | . , | black cotton | (d) | Silty |
| 42. | Ka | olinite is the pri | | | | | / 15 | |
| | (a) | black cotton so | il (b) |) bentonite | (c) | china clay | (d) | gravelly sand |

| 43. | Held water is also classified as | | |
|------------|--|--|----|
| | (a) Structural water (b) adsorbed water | ter (c) Capillary water (d) all of these | |
| 44. | Adsorbed water is also referred as | • | |
| | | (c) Hygroscopic water (d) held water | |
| 45. | is the water held by fine graine | ed soil particles due to electrochemical forces | ŀ |
| | of adhesion. | | |
| | (a) adsorbed water (b) Capillary wat | | |
| 46. | Clay particles carry charges | | |
| | (-) | (c) Neutral (d) None of these | |
| 47. | <u> </u> | | |
| | (a) uniform soil | (b) well graded soil | |
| | (c) poorly graded soil | (d) coarse soil | |
| 48. | The activity of clay is defined as rati | | |
| | (+-) | (b) liquidity index to plasticity index | |
| | | (d) Plasticity index to shrinkage index | |
| 49. | Which of the following soil is highly | 4.15 | |
| | (a) gravel (b) sand | | |
| 50. | Which of the following soil is practic | (c) Clay (d) Coarse sand | |
| - 1 | (a) gravel (b) Sand Which of the following affect permea | | |
| 51. | | (c) degree of saturation (d) all of these | |
| 52 | (a) Grain size (b) void ratio Quick sand is a | (b) dog. 00 01 01111 (1) 1111 (1) | |
| 54. | (a) Moist sand containing fine particl | les (b) Fine sand easily flowing | |
| | (c) Condition in which a cohesionless | s soil loses all its shear strength due to upware | d |
| | flow of water | | |
| | (d) none of these | | |
| 53. | The critical gradient of seepage of w | vater in a soil mass is given by | |
| | 1 - G $G - 1$ | 1+e $1+e$ | |
| | (a) $\frac{1}{1+e}$ (b) $\frac{1}{1+e}$ | (c) $\overline{1-G}$ (d) $\overline{G-1}$ | |
| 54. | Coarse grained soil has a void ratio | of 0.75 and $G = 2.75$. The critical gradient a | ıt |
| | which quick condition occurs is | | |
| | (a) 0.75 (b) 1 | (c) 0.5 (d) 0.25 | |
| 55. | Maximum size of particle for which | Darcy's law is valid is | |
| | '(a) 0.2 mm (b) 0.5 mm | (c) 1 mm (d) 2 mm | |
| 56. | Permeability of soil varies | | |
| | (a) inversely as square of grain size | | |
| | (c) as grain size | (d) inversely as void ratio | |

| 57. | With increase in water content soil suc | ction | | • |
|-----|--|--------|----------------------|-----------------------|
| | (a) decreases (b) remains same | (c) | increases | (d) all the above |
| 58. | A flow net is drawn for a dam, the to | otal 1 | head loss is 6 m, n | number of potential |
| | drop is 10, and length of flow path for | r the | last field is 1m. T | he exit gradient is |
| | (a) 0.7 _ (b) 0.6 | (c) | 1 | (d) 1.6 |
| 59. | In a shear box test the failure plane is | ; | | |
| - | (a) Weakest plane | (b) | horizontal plane | |
| | (c) Vertical plane | (d) | Major principal pla | ane |
| 60. | The pressure on phreatic line is | | | |
| | (a) equal to atmospheric pressure | (b) | Less than atmosph | eric pressure |
| | (c) more than atmospheric pressure | (d) | Not related to atm | ospheric pressure |
| 61. | The coefficient of permeability of soil | | | |
| | (a) increases with increase in temperature | e (b) | increases with decre | ease in temperature |
| | (c) decreases with increase in temperature | re (d) | has no relation with | temperature |
| 62. | In soil classification symbol M stands | for | | |
| | (a) clay (b) silt | (c) | sand | (d) Medium soil |
| 63. | The horizontal permeability is | . tha | n the vertical perm | reability. |
| | (a) More (b) less | (c) | equal to | (d) None of these |
| 64. | For a flow net $N_f = 5$ and $N_d = 20$, th | e sha | pe factor is | |
| | (a) 0.5 (b) 80 | (c) | 5 | (d) 0.25 |
| 65. | The exit gradient is the ratio of | | | |
| | (a) Slope of flow line | (b) | head loss to length | of flow field at exit |
| | (c) total head to total length | (d) | Slope of equipoter | ntial line |
| 66. | Capillary water in soils | | | |
| | (a) Causes negative pore pressure | (b) | reduces effective s | tresses |
| | (c) reduces bearing capacity | ` ' | all of these | |
| 67. | The ratio of unconfined compressive st | | | oil to the unconfined |
| | compressive strength of remoulded soi | | | |
| | (a) Sensitivity (b) activity | | | |
| 68. | 1 1 1 | | | on remoulding in a |
| | short time without change of moisture | | | |
| | (a) sensitivity (b) Activity | (c) | thixotrophy (d) | relative density |
| 69. | For most clays, sensitivity lies between | | | 1 |
| | (a) 2 to 4 (b) 4 to 8 | (c) | 8 to 15 | (d) 15 to 20 |
| 70. | The critical gradient of the seepage wa | ater | | |
| | (a) directly proportional to void ratio | | | |

Soil Mechanics and Foundation Engineering

| (b) | increases with decrease in void ratio |
|-----|---|
| | inversely proportional to specific gravity |
| (d) | increases with decrease in specific gravity of soil |
| The | critical gradient for all soils is normally |
| | |

mally 71.

- 0.5 (a)
- (b) I
- (c) 1.5
- (d) 1.05

72. Flownet is used to determine the

- (a) Seepage flow
- (b) Seepage pressure (c) exit gradient
- (d) all of these

73. The effective stress on the soil is due to

- external load acting on the soil
- (b) weight of the soil particles
- weight of water present in soil pores (d) both (a) and (b)

74. Silt is a

- Material deposited by a glacier (a)
- Soil composed of two different soils
- Fine grained soil with little or no plasticity
- Clay with a high percentage of the clay mineral (d)

75. A flow line in seepage through a soil mass is the

- Path of particles of water through a saturated soil mass
- line joining points of equal head of water (b)
- (c) Flow of movement of fine particles of soil
- direction of the flow particle

76. The relation between coefficient of consolidation (C_v), coefficient volume change (m_v) and coefficient of permeability (k) is given by $C_{v} = \frac{k}{m_{v} \cdot \gamma_{w}} \qquad (b) \quad C_{v} = \frac{k \cdot m_{v}}{\gamma_{w}} \qquad (c) \quad C_{v} = \frac{\gamma_{w}}{k \cdot m_{v}} \qquad (d) \quad C_{v} = \frac{m_{v} \cdot \gamma_{w}}{k}$

(a)
$$C_v = \frac{k}{m_v \cdot \gamma_w}$$

(b)
$$C_v = \frac{k \cdot m_v}{\gamma_w}$$

(c)
$$C_v = \frac{\gamma_w}{k \cdot m_v}$$

(d)
$$C_v = \frac{m_v \cdot \gamma_w}{k}$$

77. The relation between coefficient of consolidation (C_v), time factor (T_v) and drainage path (H), time (t) is given by $T_{v} = \frac{C_{v} \cdot H}{t}$ (b) $T_{v} = \frac{C_{v} \cdot t}{H^{2}}$ (c) $T_{v} = \frac{C_{v} \cdot t^{2}}{H}$ (d) $T_{v} = \frac{C_{v} \cdot t}{H}$

(a)
$$T_v = \frac{C_v \cdot H}{t}$$

(b)
$$T_v = \frac{C_v \cdot t}{H^2}$$

(c)
$$T_{v} = \frac{C_{v} \cdot t^{2}}{H}$$

(d)
$$T_v = \frac{C_v \cdot t}{H}$$

78. The coefficient of consolidation is measured in

- (a) cm^2/g
- (b) cm^2/s
- (c) $g/cm^2/s$

79. The time factor for 25 % degree of consolidation is given by

- (a)
- (b) $\frac{\pi}{16}$
- (d) $\frac{\pi}{64}$

80. The strength of soil is usually identified by

- direct tensile stress
- direct compressive stress
- ultimate shear stress
- (d) effective stress

| 81. | 1. With increase in liquid limit compression index | • |
|-----|---|---------------------|
| | (a) decreases (b) increases | |
| | (c) remain constant (d) May increase or decrease | |
| 82. | 2. The coefficient of compressibility is the ratio of | • |
| | (a) Change in void ratio to change in effective stress | |
| | (b) Volumetric strain to change in effective stress | |
| | (c) Change in thickness to change in effective stress | |
| | (d) Stress to strain | |
| 83. | 33. When drainage is permitted throughout the triaxial test, it is known as | |
| | (a) Quick test (b) Drained test | |
| | (c) Consolidated undrained test (d) none of these | |
| 84. | 84. Coulomb's equation for shear strength of sand is | |
| | (a) $S = C + \sigma \tan \phi$ (b) $S = \sigma \tan \phi$ (c) $S = C$ (d) None of t | iese |
| 85. | 85. Coulomb's equation for shear strength of cohesive soil is | • |
| | (a) $S = C + \sigma \tan \phi$ (b) $S = \sigma \tan \phi$ (c) $S = C$ (d) None of t | nese |
| 86. | 86. The common size of direct shear test box is | |
| | (a) $50 \times 50 \times 60 \text{ mm}$ (b) $50 \times 50 \times 40 \text{ mm}$ | |
| | (c) $60 \times 60 \times 40 \text{ mm}$ (d) $60 \times 60 \times 50 \text{ mm}$ | atuonath |
| 87. | 87. The relation between undrained cohesion (C _u) and unconfined compressive | strength |
| | for cohesive soil is | a |
| | (a) $C_u = 2 q_u$ (b) $C_u = \frac{q_u}{2}$ (c) $C_u = 3 q_u$ (d) $C_u = \frac{q_u}{2}$ | 3 |
| QQ | 88. In triaxial compression test, the deviator stress (σ _d) is given by | |
| 00. | (a) $\sigma_d = \sigma_1 + \sigma_3$ (b) $\sigma_d = \sigma_1 - \sigma_3$ (c) $\sigma_d = \sigma_1 \times \sigma_3$ (d) $\sigma_2 = \sigma_1 \times \sigma_3$ | σ_1/σ_3 |
| 20 | 89. The angle of failure plane with major principal plane is given by | |
| 0, | | ф |
| | (a) $45^{\circ} + \phi$ (b) $45^{\circ} + \phi/2$ (c) $45^{\circ} - \phi$ (d) $45^{\circ} - \phi$ | 2 |
| 90. | 90. Rollers suitable for compacting cohesionless soils are | |
| | (a) Smooth wheeled rollers (b) Sheep foot rollers | |
| | (c) Pneumatic tyred rollers (d) None of these | . (|
| 91 | 91. Pneumatic tyred rollers are useful for compacting | |
| | (a) Cohesive soils (b) Cohesionless soils | |
| | (c) both (a) and (b) (d) For soils in confined space | |
| 92 | 92. Vibroflotation technique is best suited for | |
| | (a) Clays (b) Silt | |
| | (c) Coarse sand and gravel (d) Organic soils | |
| • | | |

respectively in a two dimensional flow. The effective permeability keg for the soil

is given by

Objective Civil Eng. \ 2016 \ 32

(GATE)

Open drive thin walled tube sampler (b) Standard split spoon sampler

(d) Rotary sampler

Stationary piston sampler

(Civil Services)

2.

4.

Degree of saturation

Grain size

1.

3.

Void ratio

Pressure head

| Sel | ect the correct an | swer using the cod | es give | n below: | |
|--------|---------------------------------------|---------------------------|---------|-------------------------------|---|
| | les: | | | | |
| | | (b) 1, 2 and 3 | | | (d) 1, 3 and 4 |
| | | | single | degree of freedo | om, resonance occurs |
| at : | a frequency ratio | o of | | | (Civil Services) |
| (a) | 1 | (b) 0 | • ′ | less than 1 | (d) greater than 1 |
| 128. A | clear dry sand s | ample is tested in | a dire | ect shear test. Th | ne normal stress and |
| | | re are both equal to | D 120 R | an/m ² . The angle | of shearing resistance (Civil Services) |
| | the sand will be | (1.) 250 | (c) | 150 | (d) 55° |
| (a) | 25° | (b) 35° | , , | | • • |
| | | mpling, the area ra | tio for | thin wan sample | r should not normally (IES) |
| | eed | (b) 15 % | (c) | 20 % | (d) 25 % |
| (a) | | | * * | | l proctor test and the |
| | e ratio of the en ndard proctor to | | 3011 32 | in pie in mourre | (IES) |
| | 10 | (b) 4.5 | (c) | 2.2 | (d) 1.8 |
| (a) | | as the following pi | | | (IES) |
| | uid limit = 45 % | • | | | |
| | stic limit = 25% | | | | |
| | | | | | |
| | rinkage limit = 1 | | | | |
| | tural moisture co | | | | |
| | e consistency ind | | (0) | 8/20 | (d) 5/20 |
| (a) | | (b) 13/20 | (c) | | |
| 132. A | stratum of soil c | consists of three lay | yers or | equal interness. | The permeabilities of ddle layer is $k = 10^{-3}$ |
| to. | y and bottom ia | lue of horizontal | coeffic | ient of permeabi | lity k _H for the entire |
| · so | il laver is | | | | (IES) |
| (a` | $1.2 \times 10^{-3} \text{ cm/s}$ | (b) 4×10^{-4} cm | /s (c) | 3×10^{-4} cm/s | (d) 1.5×10^{-4} cm/s |
| | | ist II and select c | | | |
| 20012 | List I | | | List II | |
| A | | | 1. | Stiff clay | |
| В. | | | 2. | Loose granular | soil |
| C. | - | | 3. | Lateral load | |
| D | | ile . | 4. | Uplift load | |
| . Б | . Compaction p | | | • | |
| | | • | | | |

Objective Civil Engineering

254

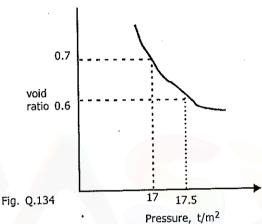
| Codes: | | | | | | |
|------------------|---|---|---|---|--|--|
| | A | В | С | | | |
| (a) | 3 | 1 | 2 | 4 | | |
| (b) | 1 | 3 | 4 | 2 | | |
| (c) | 3 | 1 | 4 | 2 | | |
| (1) | 1 | 3 | 2 | 4 | | |

134. The void ratio - pressure diagram is shown in the figure below:

(IES)

The coefficient of compressibility is

- $0.05 \text{ m}^2/\text{t}$
- $0.073 \text{ m}^2/\text{t}$
- $0.20 \text{ m}^2/\text{t}$
- $0.25 \text{ m}^2/\text{t}$

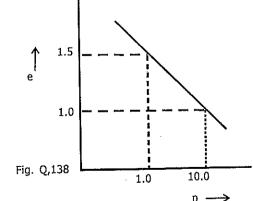


135. Sheep foot rollers are recommended for compacting

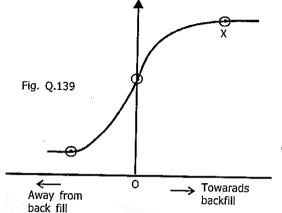
(IES)

- (a) granular soils
- (b) Cohesive soils (c) hard rock
- any type of soil (d)
- 136. A soil has a discharge velocity of 6×10^{-7} m/s and a void ratio of 0.5. Its seepage velocity is

 - (a) 18×10^{-7} m/s (b) 12×10^{-7} m/s (c) 6×10^{-7} m/s
- (d) 3×10^{-7} m/s
- 137. In Newmark's influence chart for stress distribution there are ten circles and ten radial lines. The influence factor of the chart is (IES)
- (b) 0.01
- (c) 0.001
- (d) 0.0001
- 138. The virgin compression curve for a particular soil is shown in the figure. The (IES) compression index of the soil is
 - 0.35 (a)
 - 0.50
 - 1.0
 - (d) 1.5



139. Earth pressure and resultant possibilities of wall movement are shown in the figure below. The point marked 'x' in the diagram indicates



- (a) earth pressure at rest
- (b) active earth pressure
- (c) passive earth pressure
- (d) arching active pressure

140. Minimum centre to centre spacing of friction piles of diameter (D) as per BIS code is

- (a) 1.5 D
- (b) 2 D
- (c) 2.5 D
- (d) 3 D

141. Which one of the following tests can not be done without undisturbed sampling?
(IES)

(a) Shear strength of sand

- (b) Shear strength of clay
- (c) determination of compaction parameters (d) Atterberg limits
- 142. If an unconfined compressive strength of 4 kg/cm² in the natural state of clay reduces by four times in the remoulded state, then its sensitivity will be (IES)
 - (a) 1
- (b) 2
- (c) 1

(d) 8

143. Ratio of bearing capacity of double under reamed pile (U.R.) to that of single U.R. pile is nearly (IES)

- (a) 2
- (b) 1.5
- (c) 1.2
- (d) 1.7

144. A soil sample has void ratio of 0.5 and its porosity will be close to (GATE)

- (a) 50 %
- (b) 66 %
- (c) 100 %
- (d) 33 %

145. A borrow pit has a dry density of 17 kN/m³. How many cubic meters of this soil will be required to construct an embankment of 100 m³ volume with a dry density of 16 kN/m³?

(GATE)

- (a) 94 m^3
- (b) 106 m^3
- (c) 100 m^3
- (d) 90 m^3

146. The group efficiency of a pile group

(GATE)

- (a) will be always less than 100 %
- (b) will be always greater than 100 %
- (c) may be less than 100 % or greater than 100 %
- (d) will be more than 100 % for pile group in cohesionless soil and less than 100 % for pile group in cohesive soil

| 147. The two criteria for the det | ermination of allowable beari | ing capacity of a foundation |
|-----------------------------------|-------------------------------|------------------------------|
| are | | (GATE) |

- (a) tensile failure and compression failure (b) tensile failure and settlement
- (c) bond failure and shear failure (d) Shear failure and settlement
- 148. The void ratio and specific gravity of a soil are 0.65 and 2.72 respectively. The degree of saturation (in percent) corresponding to water content of 20 % is (GATE)
 - (a) 65.3
- (b) 20.9
- (c) 83.7
- (d) 54.4
- 149. The undrained cohesion of a remoulded clay soil is 10 kN/m². If sensitivity of the clay is 20, then corresponding remoulded compressive strength is (GATE)
 - (a) 5 kN/m^2
- (b) 10 kN/m^2
- (c) 20 kN/m^2
- (d) 200 kN/m^2
- 150. Negative skin friction in a soil is considered when the pile is constructed through a
 - (a) Fill material

- (b) Dense coarse sand
- (c) Over consolidated stiff clay
- (d) dense fine sand

: ANSWERS :

| 1. (b) | 2. (c) | 3. (b) | 4. (b) | 5. (c) |
|-----------------------|------------|----------|----------|----------|
| 6. (b) | 7. (b) | 8. (b) | 9. (b) | 10. (c) |
| 11. (d) | 12. (c) | 13. (b) | 14. (d) | 15. (b) |
| 16. (a) | 17. (c) | 18. (a) | 19. (a) | 20. (a) |
| 21. (a) | 22. (a) | 23. (b) | 24. (b) | 25. (a) |
| 26. (b) | 27. (c) | 28. (a) | 29. (a) | 30. (b) |
| 31. (c) | 32. (a) | 33. (c) | 34. (b) | 35. (a) |
| 36. (b) | 37. (b) | 38. (d) | 39. (a) | 40. (b) |
| 41. (c) | 42. (c) | 43. (d) | 44. (c) | 45. (a) |
| 46. (b) | 47. (b) | 48. (c) | 49. (a) | 50. (c) |
| 51. (d) | 52. (c) | 53. (b) | 54. (b) | 55. (b) |
| 56. (b) | 57. (c) | 58. (b) | 59. (b) | 60. (a) |
| 61. (a) | 62. (b) | 63. (a) | 64. (d) | 65. (b) |
| 66. (a) | 67. (a) | 68. (c) | 69. (a) | 70. (b) |
| 71. ³⁻ (b) | 72. (d) | 73. (d) | 74. (c) | 75. (a) |
| 76. (a) | 77. (b) | 78. (b) | 79. (d) | 80. (c) |
| 81. (b) | 82. (a) | 83. (b) | 84. (b) | 85. (c) |
| 86. (d) | 87. (b) | 88. (b) | 89. (b) | 90. (a) |
| 91. (c) | 92. (c) | .93. (b) | 94. (b) | 95. (b) |
| 96. (d) | 97. (c) | 98. (a) | 99. (b) | 100. (c) |
| 101. (a) | . 102. (b) | 103. (a) | 104. (b) | 105. (d) |

Soil Mechanics and Foundation Engineering

| | _ | _ | _ |
|--|---|---|----|
| | | _ | _ |
| | | | ı |
| | | | ı |
| | | | 1 |
| | | | 1 |
| | | | П |
| | | | 1 |
| | | | 1 |
| | | | -1 |

257

| 106 (1) | 107 (0) | 108. (c) | 109. (d) | 110. (b) |
|----------|-----------------|----------|----------|----------|
| 106. (b) | 107. (c) | 113. (c) | 114. (a) | 115. (b) |
| 111. (c) | 112. (c) | ì | 119. (a) | 120. (a) |
| 116. (a) | 117. (b) | 118. (c) | 1 | 125. (c) |
| 121. (c) | 122. (c) | 123. (c) | 124. (d) | · · |
| 126. (a) | 127. (a) | 128. (c) | 129. (a) | 130. (b) |
| 131. (a) | 132. (b) | 133. (b) | 134. (c) | 135. (b) |
| 136. (a) | 137. (b) | 138. (b) | 139. (c) | 140. (d) |
| 141. (b) | 142. (c) | 143. (b) | 144. (d) | 145. (b) |
| ` ' | • • | 148 (c) | 149. (c) | 150. (a) |
| 146. (a) | 147. (d) | 140. (0) | <u> </u> | <u> </u> |

EXPLANATIONS

22. (a)
$$I_c = \frac{W_L - w}{I_p} = \frac{W_L - W_L}{I_p} = 0$$

At liquid limit

$$w = W_L$$

23. (b)
$$I_c = \frac{W_L - W}{I_p} = \frac{W_L - W_p}{I_p} = \frac{I_p}{I_p} = 1$$

at plastic limit

$$w = Wp$$

24. (b)
$$I_L = \frac{W - W_p}{I_p} = \frac{W_L - W_p}{I_p} = 1$$

at liquid limit

$$w = W_L$$

25. (a)
$$I_L = \frac{w - W_p}{I_p} = \frac{W_p - W_p}{I_p} = 0$$

at plastic limit

$$w = W_p$$

26. (b)

| Ip | Plasticity |
|--------|----------------|
| 0 | non plastic |
| < 7 | low plastic |
| 7 – 17 | Medium plastic |
| > 17 | Highly plastic |

29. (a)

| 00 | 2 mm | 0.075 mm | 4.75 mm | 80 mm | 300 mm |
|------|------|----------|---------|--------|---------|
| Clay | silt | Sand | gravel | cobble | Boulder |

Objective Civil Eng. \ 2016 \ 33

Objective Civil Engineering

258

30. (b) Ip A – line = 0.73 (
$$W_L$$
 – 20)
= 0.73 (60 – 20)
= 29 < 40

The point is above I_p - Aline

soil is CH type

79. (d) For
$$u < 60 \%$$

$$T_{V} = \frac{\pi}{4} \left(\frac{u}{100}\right)^{2} = \frac{\pi}{4} \left(\frac{25}{100}\right)^{2} = \frac{\pi}{64}$$
81. (b) $C_{c} = 0.007 \text{ (W}_{L} - 10)$

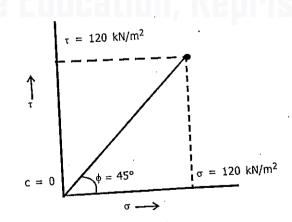
82. (a)
$$a_v = \frac{e_0 - e}{\sigma - \sigma_0}$$

119. (a)
$$T_V = \frac{\pi}{4} \left(\frac{u}{100} \right)^2 = \frac{\pi}{4} \left(\frac{50}{100} \right)^2 = 0.196 \approx 0.20$$

121. (c)
$$q_{nu} = q_u - \gamma.D$$

 $q_{ns} = \frac{q_{nu}}{F} = \frac{25}{2.5} = 10 \text{ t/m}^2$
 $q_s = q_{ns} + \gamma.D$
 $= 10 + 1.7 \times 1 = 11.7 \text{ t/m}^2$

128. (c)



For modified proctor test, Energy = 2700 kJ/m^3 For standard proctor test, energy = 592 kJ/m^3

$$\therefore \quad \text{Energy ratio} = \frac{2700}{592} = 4.56$$

131. (a)
$$I_{c} = \frac{W_{L} - w}{I_{p}} = \frac{W_{L} - w}{W_{L} - W_{p}}$$
$$= \frac{45 - 30}{45 - 25} = \frac{15}{20}$$

132. (b)
$$k_{H} = \frac{k_{1}d_{1} + k_{2}d_{2} + k_{3} d_{3}}{d_{1} + d_{2} + d_{3}}$$

$$= \frac{10^{-4} d + 10^{-3} d + 10^{-4} d}{3d}$$

$$= \frac{12 \times 10^{-4} d}{3d} = 4 \times 10^{-4} \text{ cm/sec}$$

134. (c)
$$a_v = \frac{e_o - e}{\sigma' - \sigma'_0} = \frac{(0.7 - 0.6)}{(17.5 - 17)} = \frac{0.1}{0.5} = 0.2 \text{ m}^2/\text{t}$$

136. (a)
$$V_s = \frac{V}{n} = \frac{V(1+e)}{e} = \frac{6 \times 10^{-7} (1+0.5)}{0.5}$$

= 18 × 10⁻⁷ m/s

137. (b) Influence factor
$$=\frac{1}{\text{No. of sub areas}}$$

$$=\frac{1}{\text{No. of circles} \times \text{No. radial lines}}$$

$$=\frac{1}{10 \times 10} = 0.01$$

138. (b) Compression index,

$$C_{c} = \frac{e_{o} - e}{\log_{10} \frac{\sigma'}{\sigma'_{o}}} = \frac{0.5}{\log_{10} \left(\frac{10}{1}\right)} = \frac{0.5}{1} = 0.5$$

142. (c) Sensitivity, St =
$$\frac{q_u \text{ (undisturbed)}}{q_u \text{ (remoulded)}}$$

144. (d)
$$n = \frac{e}{1+e} = \frac{0.5}{1+0.5} = 0.33$$

145. (b)
$$\frac{\gamma_{d_1}}{\gamma_{d_2}} = \frac{V_1}{V_2}$$
 $\frac{17}{16} = \frac{V_1}{100}$ $\therefore V_1 = 106 \text{ m}^3$

148. (c)
$$e = \frac{wG}{S_r}$$
 \therefore $S_r = \frac{wG}{e}$

$$= \frac{0.20 \times 2.72}{0.65} = 83.7 \%$$

$$= \frac{0.20 \times 2.72}{0.65} = 83.7 \%$$

$$149. (e) \quad C_u = \frac{q_u}{2} \qquad \therefore \quad 10 = \frac{q_u}{2} \qquad \therefore \quad q_u = 20 \text{ kN/m}^2$$

$$S_t = \frac{q_u \text{ undisturbed}}{q_u \text{ remoulded}}$$

Since compressive strength itself is q_n

remoulded comp. strength = 20 kN/m^2

SET-4:

- 1. The plasticity of clays are due to
 - (a) adsorbed water (b) free water (c) water molecules bond (d) None of above
- 2. In fine sands and silts, the most common type structure is
 - (a) single grained (b) flocculated (c) Honey combed
- (d) Dispersed

- 3. The behaviour of sand is governed by
 - (a) Mass energy
 - (b) surface energy (c) both a and b
- (d) None of the above

- 4. The behaviour of clay is governed by
 - (a) Mass energy
- (b) Surface energy (c) both a and b
- (d) None of the above

- 5. Honey combed structure is found in
 - (a) Gravels
- (b) Coarse sands (c) Fine sands and silts (d) Clay
- 6. The grains of sands and gravels are
 - (a) bulky
- (b) Flaky
- (c) Elongated
- (d) None of the above
- 7. The particles with a high value of sphericity are
 - (a) Easy to maniputate in construction (b) Low tendency to fracture
 - (c) Both a and b

- (d) High tendency to fracture
- 8. The smallest clay mineral is
 - (a) Kaolinite
- (b) Illite
- (c) Chlorite
- (d) Montmorillonite

: ANSWERS :

| (1) (a), | (2) (c), | (3) (a), | (4) (b), | (5) (c), | |
|----------|----------|----------|----------|----------|--|
| (6) (a), | (7) (c), | (8) (d). | | | |

148. (c)
$$e = \frac{wG}{S_r}$$
 \therefore $S_r = \frac{wG}{e}$ $= \frac{0.20 \times 2.72}{0.65} = 83.7 \%$

149. (c)
$$C_u = \frac{q_u}{2}$$
 \therefore $10 = \frac{q_u}{2}$ \therefore $q_u = 20 \text{ kN/m}^2$ $S_t = \frac{q_u \text{ undisturbed}}{q_u \text{ remoulded}}$

Since compressive strength itself is $q_{\mathfrak{u}}$

remoulded comp. strength = 20 kN/m^2

SET-4:

- The plasticity of clays are due to 1.
 - (a) adsorbed water (b) free water (c) water molecules bond (d) None of above
- In fine sands and silts, the most common type structure is 2.
 - (a) single grained (b) flocculated
- (c) Honey combed
- (d) Dispersed

- The behaviour of sand is governed by 3.
 - (a) Mass energy
- (b) surface energy (c) both a and b
- (d) None of the above

- The behaviour of clay is governed by 4.
 - (a) Mass energy
- (b) Surface energy (c) both a and b
- (d) None of the above

- Honey combed structure is found in 5.
 - (a) Gravels
- (b) Coarse sands (c) Fine sands and silts (d) Clay
- The grains of sands and gravels are 6.
 - (a) bulky
- (b) Flaky
- (c) Elongated
- (d) None of the above
- The particles with a high value of sphericity are 7.
 - (a) Easy to maniputate in construction (b) Low tendency to fracture
 - (c) Both a and b

- (d) High tendency to fracture
- The smallest clay mineral is 8.
 - (a) Kaolinite
- (b) Illite
- (c) Chlorite
- (d) Montmorillonite

: ANSWERS:

| (1) (a), | (2) (c), | (3) (a), | (4) (b), | (5) (c), |
|----------|----------|----------|----------|----------|
| (6) (a), | (7) (c), | (8) (d). | | _ |

SET-5:

| 1. | The minimum v | vater content at wh | nich soil starts getti | ng shear strength is |
|----|------------------------|-------------------------|--------------------------|----------------------|
| | termed as, | | | |
| | (a) Liquid limit | (b) Plastic limit | (c) Shrinkage limit | (d) Plasticity index |
| 2. | At shrinkage limi | it, the soil is | | |
| | (a) Dry | (b) Partially satura | ted (c) Saturated (d) N | lone of the above |
| 3. | The liquidity inde | x of a soil indicates t | he nearness of its wate | r content to its, |
| | (a) Liquid limit | (b) Plastic limit | (c) shrinkage limit | (d) None of above |
| 4. | When the soil is | at the plastic limit, i | ts $\mathbf{I_L}$ is | |
| | (a) 100 % ² | (b) 50 % | (c) 0 | (d) 25 % |
| 5. | The consistency in | ndex of a soil indicate | s the nearness of its wa | ater content to its, |

(a) Liquid limit (b) Plastic limit (c) Shrinkage limit (d) None of above

6. When the soil is at the plastic limit, its I_c is

(a) 100 % (b) 50 9

(b) 50 % (c) 0

(d) 25 %

7. The toughness is the ratio of

(a) Plasticity index to flow index

(b) Liquidity index to flow index

(c) Consistency index to flow index

(d) Shrinkage index to flow index

8. The shrinkage index is equal to

(a) Liquid limit minus plastic limit

(b) Plastic limit minus shrinkage limit

(c) Liquid limit minus shrinkage limit

(d) None of the above

9. A stiff class has a consistancy index

(a) 50-75

(b) 75 - 100

(c) Greater than 100 (d) Less than 50

10. The Plasticity index of a highly plastic soil is about

(a) 10 - 20

(b) 20 - 40

(c) Greater than 40

(d) Less than 10

11. For most clays sensitivity is

(a) Less than 1.0

(b) 2 to 4

(c) 4 - 16

(d) more than 16

12. For a soil sample if $I_p = 30\%$ and % of particles finer than 2μ size is 20 %, its activity is

(a) 0.67

(b) 1.0

(c) 1.50

(d) None of the above

: ANSWERS:

| William Manager | | | · | | |
|-----------------|----------|----------|-----------|-----------|-----------|
| (1) (a), | (2) (c), | (3) (a), | (4) (c), | (5) (b), | (6) (a), |
| (7) (a), | (8) (b), | (9) (b), | (10) (b), | (11) (b), | (12) (c). |

262

| \sim | | |
|------------|-------|------|
| | , II. | - |
| S H | | _ [7 |
| | | |

- 1. The maximum particle size for which Darcy's law is valid is,
 - (a) 0.2 mm
- (b) 0.5 mm
- (c) 1.0 mm
- (d) 2.0 mm

- 2. The coefficient of permeability of a soil
 - (a) increases with increase in temperature
 - (b) increases with decrease in temperature
 - (c) increases with a decrease in unit weight of water
 - (d) decreases with an increase in void ratio
- 3. The constant head permeability test is conducted for
 - (a) coarse grained soils (b) silty soils (c) clayey soils
- (d) organic soils
- 4. A soil has a discharge velocity 9.51×10^{-3} cm/s and void ratio of 0.675. It's seepage velocity is
 - (a) 6.426×10^{-3} cm/s
- (b) 14.10×10^{-3} cm/s
- (c) 2.36×10^{-2} cm/s
- (d) 3.2×10^{-3} cm/s
- 5. The permeability of soil varies
 - (a) as square of grain size
- (b) inversely as square of grain, size

(c) as grain size

(d) inversely as void ratio

: ANSWERS:

| (1) (b), (2) (a), | (3) (a), | (4) (c), | (5) (a). |
|-------------------|----------|----------|----------|

SET-7:

- 1. Which of the following is incorrect for compaction?
 - (a) Decrease in volume of soil is due to removal of air from voids.
 - (b) The load is static
 - (c) Process is rapid
 - (d) It is an artificial process.
- 2. The line of optimum generally corresponds to percentage air voilds of about
 - (a) zero percent
- (b) 5 %
- (c) 10 %
- (d) 20 %
- 3. Soil compacted dry of OMC as compared to wet of OMC,
 - (a) has less permeability

(b) swells less

(c) shrinkage less

- (d) has less resistance to compression
- 4. For a Standard Proctor test, the mass of hammer and the drop of hammer are
 - (a) 2.6 kg and 310 mm

(b) 2.6 kg and 450 mm

(c) 4.9 kg and 310 mm

(d) 4.9 kg and 450 mm

264

| CI. | | Γ. | n | |
|-----|------|----|---|--|
| | М, Т | - | 7 | |

| SET-9: | | | | |
|--|--|--|--|--|
| The ultimate settlement of a | soil deposit increases with | | | |
| (a) an increase in the compres | | | | |
| (b) an increase in the initial v | void ratio | | | |
| (c) decrease in the thickness | | | | |
| ` ' | | | | |
| With increase in liquid limit, compression index | | | | |
| | (b) decreases | | | |
| | (d) may increase or decrease | | | |
| The coefficient of compressibility is the ratio of | | | | |
| (a) change in void ratio to change in effective stress | | | | |
| (b) volumetic strain to change in effective stress | | | | |
| | | | | |
| | ange in effective stress | | | |
| | The ultimate settlement of a (a) an increase in the compres (b) an increase in the initial v (c) decrease in the thickness (d) an increase in time With increase in liquid limit (a) remains constant (c) increases The coefficient of compress (a) change in void ratio to che | | | |

- A remoulded clay has a liquid limit 50%, its compression index will be
 - (b) 0.28(a) 0.18
- (c) 0.38
- (d) 0.48
- The recompression index is about _____ of the compression index. 5.
 - (a) 5 times
- (b) $\frac{1}{5}$
- (c) $\frac{1}{2}$
- (d) $\frac{1}{20}$

: ANSWERS:

| (1) (a), | (2) (c), | (3) (a), | (4) (b), | (5) (b). | |
|----------|----------|----------|----------|-------------|--|
| | l | | | | |

SET-10:

- The inclination of the failure plane behind a vertical wall in the passive pressure case is inclined to the horizontal at -
 - (a) $45^{\circ} \frac{\phi}{2}$ (b) $45^{\circ} \phi$ (c) $45^{\circ} + \frac{\phi}{2}$ (d) $45^{\circ} + \phi$

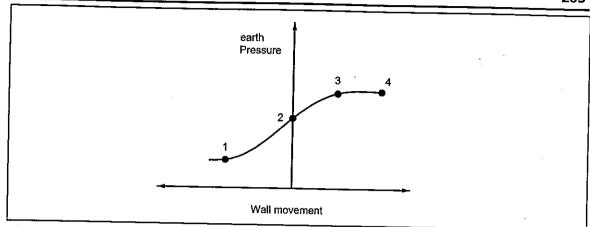
Ans. (a)

- The variation of earth pressure with wall movement is shown in the figure by the (IES 1995) points labelled
 - (a) 1 and 2
- (b) 2 and 3
- (c) 3 and 4
- (d) 1 and 4

Ans. (d)

Soil Mechanics and Foundation Engineering

265



For a sand having an internal friction of 30°, the ratio of passive to active lateral earth pressure will be -(IES 2001)

Ans. : (d)

$$k_a = \frac{1 - \sin 30^{\circ}}{1 + \sin 30^{\circ}} = \frac{1}{3}$$

$$k_p = \frac{1}{k_o} = 3$$

$$\therefore \quad \frac{k_p}{k_a} = \frac{3}{\left(\frac{1}{3}\right)} = 9$$

An earth retaining structure may be subjected to the following lateral earth pressures:

(1) Earth pressure at rest (2) Passive earth pressure (3) Active earth pressure (IES 2001)

The correct sequence of the increasing order of the magnitude of these pressures is

(a) 3, 2, 1

Ans. (d)

5. When movement of a wall under the earth pressure from the backfill was prevented the coefficient of earth pressure was recorded as 0.5. The ratio of the coefficients of passive and active earth pressures of the backfill is (IES 2002)

(a)

(b) 3

(c) $\frac{1}{9}$

(d) 9

Ans. (d)

Objective Civil Eng. \ 2016 \ 34

Coefficient of earth pressure at rest,

$$k_a = 1 - \sin \phi = 0.5$$

$$\therefore \sin \phi = 0.5^{\bullet}$$

·∴
$$\phi = 30^{\circ}$$

$$\therefore k_a = \frac{1 - \sin\phi}{1 + \sin\phi} = \frac{1 - 0.5}{1 + 0.5} = \frac{1}{3}$$

$$k_p = \frac{1}{k_a} = 3$$

$$\therefore \frac{k_p}{k_a} = \frac{3}{\left(\frac{1}{3}\right)} = 9$$

- 6. To have zero active pressure intensity at the top of a wall in cohesive soil, one should apply a uniform surcharge intensity of (GATE)
 - (a) $2c \tan \alpha$
- (b) $2c \cot \alpha$
- (c) $-2c \tan \alpha$
- $(d) 2c \cot \alpha$

Ans. (a)

For backfill with surcharge in cohesive soil,

$$p_a = \gamma \cdot z \cot^2 \alpha - 2c \cot \alpha + q \cot^2 \alpha$$

For zero active pressure at top,

$$z = 0, p_a = 0$$

$$\therefore q \cot^2 \alpha = 2c \cot \alpha$$

$$\therefore q = \frac{2c}{\cot \alpha} = 2c \tan \alpha$$

7. A retaining wall of height 8 m retain dry sand. In the initial state soil is loose and has a void ratio of 0.5, $\gamma_d = 17.8 \text{ kN/m}^3$ and $\phi = 30'$.

Subsequently, the backfill is compacted to a state where void ratio is 0.4, $\gamma_d = 18.8 \text{ kN/m}^3$ and $\phi = 35^\circ$. The ratio of initial passive thrust, to the final passive thrust according to Rankine earth pressure theory is – (GATE)

(d) 1.55

Ans. (c)

Before compaction:

e = 0.5
H = 8 m

$$\gamma_{d_1} = 17.8 \text{ kN/m}^3$$

 $\phi = 30^\circ$
 $k_{p_1} = \frac{1 + \sin 30'}{1 - \sin 30^\circ} = 3$

After compaction:

$$e = 0.4$$

 $\gamma_d = 18.8 \text{ kN/m}^3$
 $\phi = 35^\circ$

$$k_{p_2} = \frac{1 + \sin 35'}{1 - \sin 35^{\circ}} = 3.69$$

$$\frac{p_{p_1}}{p_{p_2}} = \frac{\frac{1}{2} k p_1 \cdot \gamma_{d_1} \cdot H^2}{\frac{1}{2} k p_2 \cdot \gamma_{d_2} \cdot H^2} = \frac{k p_1 \cdot \gamma_{d_1}}{k p_2 \cdot \gamma_{d_2}} = \frac{3 \times 17.8}{3.69 \times 18.8} = 0.77$$

- 8. An unsupported excavation is made to the maximum possible depth in a clay soil having $\gamma = 18 \text{ kN/m}^3$, $c = 100 \text{ kN/m}^2$, $\phi = 30^\circ$. The active earth pressure, according to Rankine's theory at the base level of the excavation is (GATE)
 - (a) 115.47 kN/m²

(b) 54.36 kN/m^2

(c) 27.18 kN/m^2

(d) 13 kN/m²

Ans. (a)

Depth of unsupported vertical cut is

$$H_c = \frac{4c}{\gamma} \tan \alpha$$

$$= \frac{4 \times 100}{18} \tan 60^{\circ}$$

$$= 38.49 \text{ m}$$

$$p_a = \gamma \cdot \text{H } \cot^2 \alpha - 2c \cot \alpha$$

$$= 18 \times 38.49 \times (\cot 60^{\circ})^2 - 2 \times 100 \cot 60^{\circ}$$

$$= 230.94 - 115.47$$

$$= 115.47 \text{ kN/m}^2.$$

9. A 3 m high retaining wall is supporting a saturated sand (saturated due to capillary action) of bulk density 18 kN/m³ and angle of shearing resistance 30′. The change in magnitude of active earth pressure at the base due to rise in ground water table from the base of the footing to the ground surface shall $(\gamma_w = 10 \text{ kN/m}^3)$

(GATE)

(a) increase by 20 kN/m²

(b) decrease by 20 kN/m²

(c) increase by 30 kN/m²

(d) decrease by 30 kN/m²

Ans. (d)

Change in active earth pressure
$$= \gamma_w \cdot H$$

= 10 × 3
= 30 kN/m²

Usually with rise in water table earth pressure also increases, but due to capillary action it decreases by $\gamma_w \cdot H$.

- 10. The yield of a retaining wall required to reach plastic equilibrium in active case is
 - (a) more than that in the passive case
- (b) less than that in the passive case
- (c) equal to that in the passive case
- (d) None of above

Ans.: (b)

- 11. The active earth pressure coefficient k_a generally referes to :
 - (a) effective stresses (b) total stresses
- (c) neutral stress
- (d) All the above

Ans. : (a)

- 12. If a uniform surcharge of 120 kN/m² is placed on the backfill with $\phi = 30^{\circ}$, the increase in pressure is
 - (a) 12 kN/m^2
- (b) 30 kN/m^2
- (c) 40 kN/m^2
- (d) 120 kN/m^2

Ans. : (c)

Pressure due to surcharge = $k_a \cdot q$

$$= \frac{1}{3} \times 120$$
$$= 40 \text{ kN/m}$$

$$= \frac{1}{3} \times 120 \qquad k_a = \frac{1 - \sin 30'}{1 + \sin 30^\circ} = \frac{1}{3}$$

- 13. A wall 8 m high with smooth vertical back retains dry cohesionless sand with $\gamma = 18 \text{ kN/m}^3$, $\phi = 30^\circ$. Determine the total lateral pressure per metre length of (IES 2004) wall in passive state.
 - (a) 144 kN/m
- (b) 1728 kN/m
- (c) 432 kN/m
- (d) 576 kN/m

Ans. : (b)

$$k_p = \frac{1 + \sin \phi}{1 - \sin \phi} = \frac{1 + \sin 30'}{1 - \sin 30^{\circ}} = 3$$

 $\therefore p_p = k_p \cdot \gamma \cdot H = 3 \times 18 \times 8 = 432 \text{ kN/m}^2$

- Total passive pressure,

$$p_p = \frac{1}{2} \times 432 \times 8 = \boxed{1728 \text{ kN/m}}$$

14. A vertical cut is made clay with $c = 15 \text{ kN/m}^2$, $\phi = 0$ and $g = 20 \text{ kN/m}^3$.

What is the theoretical depth to which the clay can be excavated without side collapse?

- (a) 6 m
- (b) 2 m
- (c) 2.5 m
- (d) 3 m

Ans. (d)

$$H_c = \frac{4c}{\gamma} \tan \alpha$$

$$=\frac{4\times15}{20}\tan 45^{\circ}=3m$$

$$\alpha = 45' + \frac{\phi}{2}$$

$$= 45^{\circ} + 0 = 45^{\circ}$$

Soil Mechanics and Foundation Engineering

269

15. Match List-I (Type of structure) with List-II (Type of pressure exerted by sandy backfill) and select the correct answer using the codes given below the lists:

| List-I | List-II |
|--|----------------------------|
| (A) A masonary retaining wall founded on | (1) Active pressure |
| compressible clay | |
| (B) Pressure on the back of a cantilever sheet | (2) Earth pressure at rest |
| pile wall near the embedded wall | |
| (C) A masonry retaining wall founded on rock | (3) Passive earth pressure |

(IES 1999)

Codes:

 \mathbf{C} \mathbf{C} B 2 3 3 1 (a) 1 2 (c) (b) 3 1 (d) 1

16. For a soil if Poisson's ratio $\mu = 0.333$, the coefficient of earth pressure at rest will be

Ans. (a)

$$k_0 = \frac{\mu}{1 - \mu} = \frac{0.333}{1 - 0.333} = 0.5$$

SET-11:

1. σ_z is the vertical stress at a depth equal to z in the soil mass due to surface point load Q. The vertical stress at depth equal to 2z will be (IES 2002)

(a)
$$0.25 \sigma_z$$

(b)
$$0.50 \, \sigma_z$$

(c)
$$1.0 \sigma_z$$

Ans. : (a)

$$\sigma_z \propto \frac{1}{z^2}$$
 : $\sigma_z \propto \frac{1}{(2z)^2} \propto \frac{0.25}{z^2}$

2. A concentrated load of 50 t acts vertically at a point on the soil surface. If Boussinesq's equation is applied for computation of stress, then the ratio of vertical stresses at depths of 3 m and 5 m respectively, vertically below the point of application of load will be ______. (IES 1995)

Ans. : (d)

$$\sigma_z = \frac{3Q}{2\pi} \cdot \frac{1}{z^2} \left[\frac{1}{1 + \left(\frac{r}{z}\right)^2} \right]^{5/2} \quad \text{since } r = 0$$

$$\therefore \quad \sigma_z = \frac{3Q}{2\pi} \cdot \frac{1}{z^2}$$

$$\therefore \quad \sigma_z \propto \frac{1}{z^2}$$

$$\therefore \quad \frac{\sigma_z(3)}{\sigma_z(5)} = \frac{\frac{1}{3^2}}{\frac{1}{5^2}} = \frac{25}{9} = 2.77$$

- 3. Newmark's influence chart can be used for the determination of vertical stress under:
 - (a) circular loaded area only
- (b) rectangular loaded area only

(c) strip load only

(d) any shape of loaded area

Ans. : (d)

- 4. Westergaard's analysis is used for :
 - (a) homogeneous (b) cohesive soils (c) sandy soils (d) stratified deposits

Ans. : (d)

5. A 25 kN point load acts on the surface of an infinite elastic medium. The vertical pressure intensity in kN/m² at a point 6.0 m below and 4.0 m away from the load will be:

(GATE)

Ans. : (d)

$$\therefore \quad \sigma_z = \frac{3Q}{2\pi z^2} \left[\frac{1}{1 + \left(\frac{r}{z}\right)^2} \right]^{5/2} = \frac{3 \times 25}{2\pi \times 6^2} \times \left[\frac{1}{1 + \left(\frac{4}{6}\right)^2} \right]^{5/2}$$

$$= 0.132 \text{ kN/m}^2$$

- 6. An isobar is a curve which:
 - (a) joins points of equal horizontal stress (b) joins points of equal vertical stress

(c) joins points of zero vertical stress (d) joins points

Ans.: (b)

- stress (d) joins points of maximum vertical stress
- 7. For a strip of width B subjected to a load intensity of q at the surface, the pressure bulb of intensity 0.2 q extends to a depth of:

3. Two circular footings of diameter D_1 and D_2 are resting on the surface of a purely cohesive soil. The ratio $\frac{D_1}{D_2}=2$. If the ultimate load carrying capacity of

the footing of diameter D_1 is 200 kN/m² then the ultimate bearing capacity (in kN/m²) of the footing of diameter D_2 will be: [IES 2001]

(a) 100

(b) 200

(c) 314

(d) 571

Ans. : (b)

For purely cohesive soil, bearing capacity does not depend upon the size of footing.

$$\therefore q_f = q_p$$

But, for footings on sand, bearing capacity also depends upon the width of footing.

$$\frac{q_f}{q_p} = \frac{\mathbf{B}_f}{\mathbf{B}_p}$$

4. In a plate load test on Sandy soil the test plate of 60 cm \times 60 cm, undergoes settlement of 5 mm at a pressure of 12×10^4 N/m². What will be the expected settlement of 3 m \times 3 m footing under same pressure? [IES 2002]

(a) 25 mm

(b) 20 mm

(c) 15 mm

(d) 9 mm

Ans. : (d)

For footing on sand,

$$S_f = S_p \left[\frac{B_f (B_p + 0.3)}{B_p (B_f + 0.3)} \right]^2$$
$$= 5 \left[\frac{3.0 (0.6 + 0.3)}{0.6 (3 + 0.3)} \right]^2$$

= 9.29 mm

5. Two footings, one circular and the other square, are founded on the surface of a purely cohesionless soil. The diameter of circular footing is same as that of the side of the square footing. The ratio of their ultimate bearing capacities is: [GATE]

(a) 3/4

(b) 4/3

(c) 1,0

(d) 1.3

Ans. : (a)

For cohesionless soil, c = 0

for surface footing, D = 0

$$\therefore \quad \frac{(q_u)_{cir}}{(q_u) \ s_q} = \frac{0.3 \cdot \gamma \cdot \mathbf{B} \cdot \mathbf{N}_{\dot{\gamma}}}{0.4 \ \gamma \cdot \mathbf{B} \cdot \mathbf{N}_{\gamma}} = \frac{3}{4}$$

6. The ultimate bearing capacity of a soil is 300 kN/m². The depth of foundation is 1 m and unit weight of soil is 20 kN/m³. Choosing a factor of safety of 2.5, the net safe bearing capacity is [GATE]

- (a) 100 kN/m^2
- (b) 112 kN/m^2
- (c) 80 kN/m^2
- (d) 100.5 kN/m^2

Ans.: (b)

 $q_u = 300 \text{ kN/m}^2$

 $\gamma = 20 \text{ kN/m}^3$

D = 1.0 m

F = 2.5

 $q_{nu} = q_u - \gamma \cdot D = 300 - 20 \times 1 = 280 \text{ kN/m}^2$

 $q_{ns} = \frac{q_{uu}}{F} = \frac{280}{2.5} = 112 \text{ kN/m}^2$

7. The two criteria for the determination of allowable bearing capacity of a foundation are:

- (a) tensile failure and compression failure (b) tensile failure and settlement
- (c) bond failure and shear failure
- (d) shear failure and settlement

Ans. : (d)

- 8. The following two statements are made with reference to the calculation of net bearing capacity theory. Identify if they are True or False.
 - I. Increase in footing width will result in increase in bearing capacity.
 - II. Increase in depth of foundation will result in higher bearing capacity.

[GATE]

- (a) Both statements are TRUE
- (b) Both statements are FALSE
- (c) I is TRUE but II is FALSE
- (d) I is FALSE but II is TRUE

Ans. : (b)

9. The width and depth of a footing are 2 and 1.5 m respectively. The water table at the site is at a depth of 3 m below the ground level. The water table correction factor for the calculation of the bearing capacity of soil is:

[GATE]

- (a) 0.875
- (b) 1.0
- (c) 0.925
- (d) 0.5

Ans. : (a)

$$Rw_2 = 0.5 \left(1 + \frac{Zw_2}{B} \right) = 0.5 \left(1 + \frac{1.5}{2} \right) = 0.875$$

10. Two circular footings of diameters \mathbf{D}_1 and \mathbf{D}_2 are resting on the surface of the same purely cohesive soil. The ratio of their gross ultimate bearing capacities is : [GATE]

Objective Civil Eng. \ 2016 \ 35

274

(a)
$$\frac{D_1}{D_2}$$

(b) 1.0

(c)
$$\frac{D_1^2}{D_2^2}$$

(d) $\frac{D_2}{D_1}$

Ans. : (b)

$$q_u = 1 \cdot 3 \ c \cdot N_c + \gamma \cdot D \cdot N_q + 0.3 \ \gamma \cdot B \cdot N_{\gamma}$$

For clay, $\phi = 0$

$$N_C = 5.7, N_q = 1, N_{\gamma} = 0$$

for surface footing, D = 0

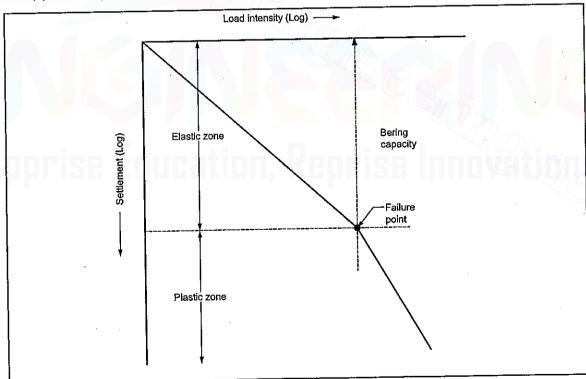
$$\therefore q_u = 1 \cdot 3 \ cN_c$$

- \therefore Ratio = 1.0
- 11. In case of footing on the surface or shallow depth in very dense sand which one of the following types of failure is likely to occur? [IES 2006]
 - (a) Punching shear failure
- (b) local shear failure
- (c) General shear failure
- (d) Any of the above three

Ans. : (c)

In general shear failure clear and sudden point of failure on the load settlement curve.

- 12. In a plate load test how is the ultimate load estimated from the load settlement curve on a log log graph? [IES 2005]
 - (a) directly



- (b) By drawing tangents to the curve at the initial and final points
- (c) By the secant method
- (d) At 0.2 percent of the maximum settlement.

Ans. : (a)

The log log graph of load versus settlement curve distincitly shows the failure point. The pressure corresponding to that point is taken as the bearing capacity.

13. The ultimate bearing capacity of a square footing on surface of a saturated clay having unconfined compression strength of 50 kN/m². (using Skempton's equation) [IES 2003]

(a) 250 kN/m^2

(b) 180 kN/m^2

(c) 150 kN/m^2

(d) 125 kN/m^2

Ans. : (c)

For square footing,

$$q_u = c \cdot N_c$$

$$N_{C} = 6 \left[1 + 0.2 \left(\frac{D}{B} \right) \right]$$

$$= 6 \left[1 + 0.2 (0) \right]$$

$$\therefore q_u = c \cdot N_c$$

$$= 25 \times 6$$

$$= 150 \text{ kN/m}^2$$

$$D = 0$$

for surface footing

$$c = \frac{q_u}{2}$$

$$=\frac{50}{2}$$

$$= 25 \text{ kN/m}^2$$

14. The allowable bearing capacity at 25 mm allowable settlement for a footing in a Sandy soil is 15 t/m^2 . The allowable bearing capacity for the same footing permitting a settlement of 40 mm is:

(a) 24 t/m^2

(b) 30 t/m^2

(c) 35 t/m^2 (d) 40 t/m^2

Ans.: (a)

For Sandy soil

$$\frac{q_2}{q_1} = \frac{S_2}{S_1}$$

$$\therefore q_2 = q_1 \times \frac{S_2}{S_1} = 15 \times \frac{40}{25} = 24 \ t/m^2$$

- 15. If two foundations one narrow and another wide, are resting on a bed of sand carying the same intensity of load per unit area, then which one is likely to fail early?

 [IES 2003]
 - (a) Narrow foundation
 - (b) Wide foundation
 - (c) Both will fail simultaneously
 - (d) Difficult for judge since other conditions are unknown.

Ans. : (a)

For foundation on sand, bearing capacity increase with width of the foundation. So narrow foundation will have lesser bearing capacity and it may fail early.

SET-13:

- 1. The maximum net pressure intensity causing shear failure of soil, is known as
 - (a) Safe bearing capacity

- (b) net safe bearing capacity
- (c) Net ultimate bearing capacity
- (d) Ultimate bearing capacity

Ans : (c)

 q_u = ultimate B.C. = gross pressure at the base of foundation at which soil fails in shear.

$$q_{nu}$$
 = net ultimate B.C.
= $qu - \gamma$. D

$$q_{us} = \frac{q_{nu}}{F}$$

$$q_s = q_{ns} + \gamma$$
. D

- 2. The rario of the volume of water present in a given soil mass to the total volume of its voids, is known as
 - (a) porosity

(b) void ratio

(c) percentage air voids

(d) degree of saturation

Ans. (d)

Void ratio,
$$e = \frac{V_v}{V_s}$$
 % air voids na = $\frac{V_a}{V}$

Porosity, $n = \frac{V_v}{V}$ degree of saturation $S_r = \frac{V_w}{V_w}$

- The value of angle of internal friction in cohesive soil is 3.
 - (a) 10°
- (b) negligible
- (c) 30°
- (d) 6.5°

Ans. (b)

- The fundamental equation of specific gravity (G), dry density (\gamma_d), unit weight of 4. water (γ_w) and void ratio (e) is
- $e = \frac{G\gamma_w}{\left(1 + \gamma_d\right)} \qquad \qquad (b) \quad G = \frac{\gamma_d\gamma_w}{\left(1 + e\right)} \qquad \qquad (c) \quad \gamma_d = \frac{G\gamma_w}{\left(1 + e\right)} \qquad (d) \quad \gamma_w = \frac{G\gamma_d}{\left(1 + e\right)}$

Ans. (c)

- 5. Coulomb assumed in his theory that
 - (a) Wall surface is vertical

- (b) wall surface is smooth
- (c) Sliding wedge behaves as rigid body
- (d) Soil is non-isotropic

- The value of time factor for 100% consolidation in double drainage is taken as 6.
 - (a) ∞
- (b) 0.85
- (c) 0.94

Ans. (a)

When $u \le 60$ %, $T_v = \frac{\pi}{4} \left(\frac{U}{100} \right)^2$

when u > 60 %, $T_v = 1.7813 - 0.933 \log_{10} (100 - U)$

- If C is cohesion, F is factor of safety, γ is unit weight of soil and H is maximum height of embankment the stability number is
- (b) $\frac{C}{F \gamma H}$ (c) $\frac{H}{C F \gamma}$ (d) $\frac{\gamma}{C F H}$

- If W is the weight of soil having moisture content w, and V is volume of proctor's mould the dry density of soil is
- (b) $\frac{V}{W(1+w)}$ (c) $\frac{W}{V(1+w)}$ (d) $\frac{V(1+w)}{w}$

Ans. (c) $\gamma_d = \frac{\gamma_b}{(1+w)} = \frac{W}{V(1+w)}$

- 9. Plate load test is used to estimate
 - (a) settlement of foundation
- (b) Bearing capacity of foundation
- (c) Both bearing capacity and settlement of foundation
- (d) depth of foundation

Ans. (c)

| 10. | Vibratory rollers are more suitable for compa | acti | ng which of tl | ne following soils? |
|-----|--|---------|-----------------|-----------------------|
| | (a) clayey (b) silty | (c) | Sandy | (d) mixed |
| | Ans. (c) | | | _ |
| 11. | Ratio of bearing capacity of double under | rear | ned pile to th | at of single under |
| | reamed pile is nearly. | | | (I) O O |
| | (a) 1.2 (b) 1.5 | (c) | 1.7 | (d) 2.0 |
| | Ans. (b) | _ | | |
| 12. | | | 0.5 | (4) 2.0 |
| | (a) 1.5 m (b) 2.0 m | (c) | 2.5 m | (d) 3.0 m |
| | Ans. (a) | | | |
| 13. | - | TH | hono coil | (d) Four phase soil |
| | (a) one phase soil (b) two phase soil (c) | | ee phase son | (u) Four phase son |
| | Ans. (c) Three phases – soil particles, water, | air | anth landad a | roa is annlicable to |
| 14. | Westergard's analysis for stress distribution | (a) | clayey soils | (d) silty soils |
| | (a) Stratition some | (0) | Clayey sons | (d) billy bolls |
| | Ans. (a) | | | |
| 15. | Rankine theory of earth pressure applies to | (b) | Cohesive soil | s , |
| | (a) dry concilionics con | ` ' | all of the abo | |
| | (c) moist concerement | (u) | | |
| 16 | Ans. (a) Among the given soils, the specific surface a | rea | is highest for | |
| 16. | (a) gravel (b) sand | (c) | silt | (d) clay |
| | Ans. (d) | | | |
| 17. | T. J. J. A. | | | |
| 17. | (a) Water content and void ratio | (b) | specific gravi | ity |
| | (c) Specific gravity and dry density | (d) | Specific grav | ity and water content |
| | Ans. (d) | | | |
| 18. | . Compaction of a soil is measured in terms of | of | | |
| | (a) dry density (b) Specific gravity | (c) | compressibili | ty(d) permeability |
| | Ans. (a) | | | |
| 19. | and the second s | con | tent at which | |
| | (a) Settlement is maximum | (b) |) permeability | |
| | (c) dry density is maximum | (d) |) Shear strengt | th is less |
| • | Ans. (c) | | | • |
| | | | | |



Highway Engineering

MCO'S The grant trunk (G.T.) road was constructed during (a) 1440 to 1445 A.D. (b) 1540 to 1545 A.D. 2000 to 2500 B.C. (d) 2500 to 3000 B.C. Shershah Suri constructed longest road from (a) Delhi to Lahore (b) Delhi to Kolkata (c)Lahore to Kolkata (d) Lahore to Agra Minimum shoulder width for roads recommended by IRC is (a) 2.5 m (b) 2 m (d) 1.85 m (c) 1.5 m As per IRC standards, minimum width of two-lane carriageway with raised curbs is (b) 7.0 m (c) 5.5 m (d) 6.0 m As per IRC standards, minimum roadway width of single lane ODR in plain and rolling terrain is (b) 12 m (c) 7 m (d) 7.5 m IRC recommends total reaction time for SSD calculation is (b) 2.5 sec (c) 2.3 sec (d) 1.8 sec In PIEV theory, I stands for (a) Intention (b) Information (c) Intellection (d) Interpretation The rate of rise or fall of the road surface along its length is called (b) super elevation (c) gradient (d) banking The equilibrium superelevation is given by (b) $\frac{V^2}{g.R}$ (d) none of these 10. As per IRC, super elevation (e) should not exceed (a) 5.7 % (b) 6.7 % (c) 0.15 % (d) 0.35 % 11. In the equation $e + f = \frac{V^2}{127R}$, the maximum value of lateral frictional coefficient

(c) 0.20

(a) 9 m

(a) 2 sec

(f) is taken as

(b) 0.15

(a) 0.10

7.

(d) 0.25

(c) 0.20

(b) 0.15

Objective Civil Eng. \ 2016 \ 36

(d) 0.25

| 43. | Degree of a road curve is defined as t | he an | gle in degrees subte | nded | l at the centre |
|-----|---|--------|-----------------------|-------------|-------------------|
| | by an arc of | | | <i>(</i> 1) | 20 |
| | (a) 10 m (b) 20 m | | | • • | 30 m |
| 44. | If degree of a road curve is defined by | y assu | ming the standard | lengt | th of an arc as |
| | 30 m, the radius of 1° curve is equal | to . | , | <i>(</i> 1) | |
| | (a) 1046 (b) 1146 | | 1719 | ` ' | 1619 |
| 45. | For clear distinct vision, images of o | bstruc | ctions should fall or | the | e retina with a |
| | cone of | | | | |
| | (a) 2° (b) 3° | (c) | | (d) | |
| 46. | Along horizontal curves, if centrifuga | l forc | e exceeds lateral fri | ction | 1, venicles may |
| | (a) skid (b) slip | ` ' | not be affected | (d) | none of these |
| 47. | In multi lane road, overtaking is gen | erally | permitted | | |
| | (a) from right | (b) | From left | | |
| | (c) From both sides right and left | | not at all | | |
| 48. | If cross slope of a country is up to 1 | 0 % t | he terrain is classif | ied a | as |
| | (a) plain (b) rolling | (c) | mountainous | (a) | Steep |
| 49. | If cross slope of a country is 10 to 25 | 5 % t | he terrain is classif | ied a | as |
| | (a) plain (b) rolling | (c) | mountainous | (d) | steep |
| 50. | c 1. 00/100 moone ite n | enetra | ation value is | | u . |
| ·A | (a) 8 mm (b) 10 mm | (c) | 8 to 10 mm | (d) | 8 to 10 cm |
| 51. | . Tie bars are provided in cement con- | crete | pavements at | | |
| | (a) expansion joints | (b) | Contraction joints | | |
| | (c) Warping joints | (d) | longitudinal joints | | |
| 52 | . Reflection cracking is observed in | | | | |
| | (a) flexible pavement | | | | |
| | (b) rigid pavement | | | | |
| | (c) bituminous overlays over cement | concr | ete surface | | |
| | (d) riding overlay over flexible paver | ment | | | |
| 53 | i. In the design of highways, expansion | and o | contraction joints sh | ould | l respectively be |
| | provided at | | | | |
| | (a) 50 m, 32 m (b) 50 m, 10 m | (c) | 25 m, 10 m | (d) |) 25 m, 32 m |
| 54 | | | | | |
| • | (a) Provide bond between old and no | ew sui | facing | | |
| | (b) Improving riding quality of pave | ment | | | |
| | (c) Provide bond between the existing | ng bas | e and surfacing of no | ew c | onstruction |
| | (d) Control dust nuisance | - | | | |
| | (u) Connot but material | | | | |

| Engineering |
|-------------|
| |

283

| 55. | In a | a bir | tuminous | paver | nent, aliga | tor cracki | ng is mainly due | e to |
|-----|------------------|-------------|------------|----------|--------------|-------------|-------------------------|----------------------------------|
| | (a) | | adequate | | | | - | • |
| | (b) | Fa | tigue aris | ing fro | m repeated | stress app | lications | |
| | (c) | | | | ess of sub-b | | | |
| | (d) | us | e of exce | ssive b | ituminous 1 | material | | |
| 56. | Inte | rme | ediate sig | ht dis | tance as pe | er IRC is | | |
| | | | tropping | | | | | |
| | OSI |) : (| Overtakin | g sight | distance | | | |
| | (a) | 2 5 | SSD | (ł | o) 2 OSD | (c) | $\frac{(SSD + OSD)}{2}$ | (d) $\frac{(OSD - SSD)}{2}$ |
| 57. | Wh clim | ich iate | one of? | the fo | ollowing b | oinders is | recommended | for a wet and cold |
| | (a) | 80, | /100 pene | tration | asphalt | (b) | tar | |
| | (c) | cut | back | | | (d) | emulsion | |
| 58. | Rap | id c | uring cu | tback | bitumen is | produced | by blending bit | umen with |
| | (a) | Ke | rosene | (b |) benzene | (c) | Petrol | (d) diesel |
| 59. | Mat | ch I | ist I witl | ı list I | I and selec | t the corre | ect answer using | the codes given below |
| | the | list | : | | | | | (IES) |
| | | | List | t - I | | | List - II | |
| | | A. | Penetrat | ion tes | it | 1. | Design of bitum | ninous concrete mix |
| | | B. | Marshal | l test | | 2. | Overlay design | |
| | | C. | Ring an | d ball | test | 3. | Gradation of as | phalt cement |
| | | D. | Benkeln | nan bea | am test | 4. | Determination of | of softening point |
| | Cod | es : | | | | | | |
| | | Α | В | C | D | | | |
| | (a) | 3 | 2 | 4 | 1 | | | |
| | (b) | 3 | 1 | 4 | 2 | | | |
| | (c) | 2 | 3 | 1 | 4 | | | |
| | (d) [^] | 4 | 2 | 3 | 1 | | | |
| 60. | Psyc | hol | ogical wi | dening | on road c | urves is g | iven by | (IES) |
| | | nL | 2 | | 0.1 V | | 0.1 nV | $0.1 \mathrm{V} - \mathrm{nL}^2$ |

(a)
$$\frac{nL^2}{2R}$$

(b)
$$\frac{0.1 \text{ V}}{\sqrt{R}}$$

(c)
$$\frac{0.1 \,\mathrm{nV}}{\sqrt{\mathrm{R}}}$$

$$\frac{0.1 \text{ V}}{\sqrt{R}} + \frac{\text{nL}^2}{2R}$$

61. Which one of the following pairs is not correctly matched?

(IES)

- (a) Horizontal curves super elevation
- (b) Origin and destination studies desire lines(c) Los Angeles test Hardness of aggregates
- (d) Soundness test purity of bitumen

| 62. | For a circular curve of | radius 200 m, the | e coeff | ficient of lateral fr | iction is 0.15 | and the |
|-------------|--|-------------------------------------|---------|-----------------------|-----------------|----------|
| | design speed is 40 kmph. | The equilibrium | supere | elevation would be | | (IES) |
| | | | (c) | 6.3 | (d) 4.6 | |
| | Hint : $\frac{V^2}{127 R} = \frac{0}{12}$ | $\frac{(40)^2}{7 \times 200} = 6.3$ | | | · | |
| 63. | The general requireme | nt in constructi | ng a 1 | reinforced concret | e road is to | place a |
| | single layer of reinforc | ement | | | | (IES) |
| | (a) Near the bottom of | the slab | (b) N | lear the top of the | slab | |
| | (c) at the middle | | , , | qually distributed a | - | |
| 64. | It was noted on a sec | | | | | |
| | density was 70 vpkm. | The maximum f | flow i | n vph that could | be expected | |
| | road is | | | | | (IES) |
| | |) 1400 | | | (d) 5600 | |
| | Hint: Maxi. flow | $= 80 \times 70 = 560$ | 00 vpl | 1 | | (77.0) |
| 65. | In desire line diagram | <i>::</i> | | | | (IES) |
| | (a) Width of desire lin | | | | | |
| | (b) length of desire line | | | | | |
| | (c) Width of desire line | | | | | |
| | (d) Both length and w | idth of desire li | ne are | proportional to th | e number of | trips in |
| | both directions. | 001 | | | | |
| 66. | A flyover segregates tr | | | size of vahiala (d | l) arada af w | shiolog |
| - | • • | | | size of vehicle (d |) grade of ve | alleles |
| 67. | A dividing strip in the | | | | (d) adaing | |
| | (a) Central strip (b) The number of vehicle | | | | | |
| 08. | | | au pe | i nour during pe | ak perious | and the |
| | average of several peal (a) traffic volume (b) | | (c) | traffic concentration | on(d) traffic | rate |
| 60 | A road open at one en | | (0) | titilio concontituti | sin(a) training | |
| U). | (a) one way road (b | | (c) | blind alley | (d) service | road |
| 70 | As per IRC, the maxim | , nanway roud num nermissible | e leno | oth of a single un | it with more | than 2 |
| /0. | axles is | num permission | · | , | | |
| | |) 12 m | (c) | 14.4 m | (d) 16 m | |
| 71. | As per IRC, the maxin | • | | • | | |
| | |) 6.165 tonnes | | | (d) 10.273 | tonnes |
| 72. | The concrete paveme | • | | | • • | |
| | exceeds | - | | | • | |
| | |) 5000 tonnes | (c) | 10,000 tonnes | (d) 12,000 | tonnes |
| | | | | • | | |
| | | | | | | |

| 87. The percentile cumulative frequency on the basis of w | hich speed regulation |
|--|---------------------------------------|
| roads is decided is | (d) 75 |
| 71.\ 05 | • • |
| on While deciding PCU, the standard vehicle considered is | (d) truck |
| | (4) |
| the design speed is taken as | tile speed |
| ==th ===tilo_cneen (5) | the speed |
| (a) 75th percentile speed (b) None of the speed (c) 98th percentile speed (d) None of the speed less than | than that and 50 % ve- |
| on an anad such that 50 % vehicles travel at specu los | s than that are |
| 1 - 4 amond mare man that the to the | • • • • • • • • • • • • • • • • • • • |
| (b) modal speed (c) avoids of | sed (d) 1 dd C 1 |
| of The colour of light used for visibility during log is | (d) yellow |
| (a) white (b) Red | (4) 3 |
| or Area provided for parking of a car is | (d) 50 m^2 |
| (a) 10 m^2 (b) 20 m^2 | |
| Test I and List II correctly: | |
| List I | |
| (A) 1. Rear and collisio | n |
| Overturned vehic | le |
| (B) | _ |
| (C) 3. Head on collisio | n |
| Fatal aggident m | otor vehicle pedestrian |
| (D) 4. Fatal accident in | |
| Codes: | |
| A B C D | |
| 1 2 | |
| (a) 3 | |
| (b) 3 4 - | |
| (c) 4 3 1 | |
| / 4 | |
| 94. Corrugations occur on treated surfaces due to (a) instability of base (b) lack of binder (c) Excess | rains (d) excess binder |
| (a) instability of base (b) lack of binder (c) | |
| 95. The pavement suitable for heavy traffic load is | ous grouted macadam |
| (a) Asphart concreting | dressed macadam |
| (c) Cement concrete | akes are applied) is more than |
| (c) Cement concrete (d) Surface (d) Surface (e) Surface (e) When the distance travelled by a vehicle (when brace) (e) When the distance travelled by a vehicle (when brace) (f) Surface (g) Surfa | d |
| the circumferential movement of whoels, | (d) wear |
| (a) slip (b) skid (c) drag | , (-) |
| • | • |

Highway Engineering

287

97. The steepest gradient permitted on roads called

(a) Ruling gradient

- (b) floating gradient
- (c) Maximum gradient
- (d) exceptional gradient

98. A road sign showing hair pin bend is a

(a) Regulatory sign

(b) Warning sign

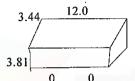
- (c) informatory sign
- (d)
- Mandatory sign

99. Which one of the following is not a traffic control device?

- (a) road signs
- (b) Markings
- (c) Foot path
- (d) Islands

100. The height of single decked vehicle is taken as

- (a) 3.81 m
- (b) 4.11 m
- (c) 5.11 m
- (d) 5.47 m



: ANSWERS:

| 2. (c) | 3. (a) | 4. (a) | 5. (d) |
|---------|--|---|---|
| 7. (c) | 8. (c) | 9. (c) | 10. (b) |
| 12. (b) | 13. (d) | 14. (b) | 15. (c) |
| 17. (b) | 18. (a) | 19. (b) | 20. (d) |
| 22. (b) | 23. (a) | 24. (c) | 25. (b) |
| 27. (d) | 28. (d) | 29. (c) | 30. (d) |
| 32. (c) | 33. (b) | 34. (c) | 35. (c) |
| 37. (d) | 38. (d) | 39. (c) | 40. (a) |
| 42. (d) | 43. (d) | 44. (c) | 45. (d) |
| 47. (c) | 48. (a) | 49. (b) | 50. (c) |
| 52. (c) | 53. (b) | 54. (c) | 55. (b) |
| 57. (d) | 58. (c) | 59. (b) | 60. (b) |
| 62. (c) | 63. (b) | 64. (d) | 65. (c) |
| 67. (c) | 68. (a) | 69. (c) | 70. (b) |
| 72. (a) | 73. (c) | 74. (d) | 75. (b) |
| 77. (c) | 78. (c) | 79. (a) | 80. (c) |
| 82. (d) | 83. (b) | 84. (d) | 85. (b) |
| 87. (b) | 88. (a) | 89. (c) | 90. (b) |
| 92. (c) | 93. (b) | 94. (a) | 95. (c) |
| 97. (c) | 98. (b) | 99. (c) | 100. (a) |
| | 7. (c) 12. (b) 17. (b) 22. (b) 27. (d) 32. (c) 37. (d) 42. (d) 47. (c) 52. (c) 57. (d) 62. (c) 67. (c) 72. (a) 77. (c) 82. (d) 87. (b) 92. (c) | 7. (c) 8. (c) 12. (b) 13. (d) 17. (b) 18. (a) 22. (b) 23. (a) 27. (d) 28. (d) 32. (c) 33. (b) 37. (d) 43. (d) 42. (d) 43. (d) 47. (c) 53. (b) 57. (d) 58. (c) 62. (c) 63. (b) 67. (c) 68. (a) 72. (a) 73. (c) 77. (c) 78. (c) 78. (c) 82. (d) 83. (b) 87. (b) 88. (a) 92. (c) 93. (b) | 7. (c) 8. (c) 12. (b) 13. (d) 14. (b) 17. (b) 18. (a) 19. (c) 22. (b) 23. (a) 24. (c) 27. (d) 28. (d) 29. (c) 32. (c) 33. (b) 34. (c) 37. (d) 38. (d) 39. (c) 42. (d) 43. (d) 44. (c) 47. (c) 48. (a) 49. (b) 52. (c) 53. (b) 54. (c) 57. (d) 58. (c) 59. (b) 62. (c) 63. (b) 62. (c) 63. (b) 64. (d) 67. (c) 68. (a) 69. (c) 72. (a) 73. (c) 74. (d) 77. (c) 78. (c) 79. (a) 82. (d) 83. (b) 84. (d) 87. (b) 88. (a) 89. (c) 92. (c) 93. (b) 94. (a) |



| 1. | Bottom most layer of pavement is known | as | (1) |
|----|---|-----------------------|-------------------------|
| | (a) subgrade (b) base course | (c) sub-base cou | rse (d) wearing course |
| | Ans. (a) | | |
| | Sequence of layers from top to bottom | is | |
| | surface course (wearing cours | e) | |
| | base course | | |
| | sub-case course | | |
| | sub grade | | |
| 2. | Desire lines are plotted in | | . 17 |
| | (a) accident studies | (b) Traffic volume | |
| | (c) origin and destination studies | (d) speed studie | S |
| | Ans. (c) | | . I do a dan linut |
| 3. | Who developed the earliest equations to ca | lculative the wheel | load stress at sallent |
| | locations on a rigid pavement? | | |
| | (a) Westergaard (b) Boussinesq | (c) Rankine | (d) Spangler |
| | Ans. (a) | | and to point load |
| 4. | Presented the theory for calculating | ig stresses in soil m | ass due to point toad |
| | on the ground. | () D U - | (d) Coulomb |
| | (a) westergaard (b) Boussinesq | (c) Rankine | (d) Coulonio |
| | Ans. (b) | 3 - 3 40 | - which recombled a |
| 5. | Who developed theory of analysing two- | -layered soil system | ii wiiich resembled a |
| | flexible pavement? | () () () () () | (d) Kelly |
| | (a) Boussinesq (b) Burmister | (c) Spangler | (d) Keny |
| | Ans. (b) | | first suggested by |
| 6. | The basic formula for determination of pa | vement thickness w | (d) Spangler |
| | (a) Kelly (b) Goldbeck | (c) Picket | (u) Spangier |
| | Ans. (b) | to 1 limetion on | woodway that nass a |
| 7. | The number of vehicles moving in a spec | eitied direction on a | a roadway that pass a |
| | given point during specified unit of time | in (a) Pagia cana | city (d) Traffic volume |
| | (a) Traffic density (b) Traffic capac | ity (e) Basic capa | city (d) Traffic volume |
| | Ans. (d) | n | |
| | Traffic volume is expressed as vehicle | es /nr. | |
| | Traffic volume is also called flow | | |
| | Maxi. possible traffic volume is called | d traffic capacity | ided in terms of |
| 8. | | nce (ISD) shall be | provided in terms of |
| | stopping sight distance (SSD) is | () A (10D) | (4) 4 (2) |
| | (a) SSD (b) 3 SSD | (c) 2 SSD | (d) 4 SSD |
| | Ans. (c) | | • |

Objective Civil Eng. \ 2016 \ 37

| | load at 2.5 mm/5 mm penetr | ation | | |
|-----|--|------------|----------------|--|
| | CBR value = standard load | | | |
| 15. | 'STOP' sign on highway is a type of | | | |
| | (a) Cautionary sign | (b) | Information s | ign |
| | (c) Mandatory sign | • • | Warning sign | |
| 16. | Tests conducted to determine flash and fire | e poir | nts of bitume | are known as |
| | (a) Pansky Marten's test (b) IRC test | (c) | Brard test | (d) API test |
| | Ans. (a) | | | |
| 17. | In water bound macadam roads binding m | ateri | al is | |
| | (a) sand (b) broken stones | (c) | cement | (d) brick dust |
| | Ans. (d) | | | |
| 18. | According to IRC, the maximum allowal | ole lir | mit of water | absorption for any |
| | aggregate used for highway pavement cons | struct | ion is | |
| | (a) 2 % (b) 3 % . | (c) | 4 % | (d) 5 % |
| Ans | . (a) | | | |
| 19. | The width of formation of a road means t | | | |
| | (a) carriage way | | Pavement and | |
| | (c) embankment at ground level | | embankment | at top level |
| | Ans. (b) Note: Ans (d) is also corre | ct | | 11 // // // // // // // // // // // // / |
| 20. | IRC has specified safe overtaking sight dista | nce a | ssuming overta | aking time requied as |
| | (a) 9 to 14 sec (b) 9 to 14 minutes | s (c) | 20 to 40 sec | (d) 40 to 50 seconds |
| | Ans. (a) | | | tala aran than ta |
| 21. | The distane travelled by revolving the | e wh | eel of a veh | icle more than its |
| | circumferential movement is known as | | *** | b (d) both a and b |
| | (a) Slip (b) skid | (c) | neither a or | b (d) both a and b |
| | Ans. (b) | | | |
| | Skid – wheel slide without revolving | | | |
| | Slip – wheel revolve without moving | 4 | | |
| 22. | | ept (-) | 22 to 45 am | (d) more than 45 cm |
| | (a) 10 cm (b) 15 to 20 cm | (c) | 23 to 43 cm | (u) more than 45 cm |
| | Ans. (a) | | • | |
| | Low or mountable kerb - 10 cn semi - barrier kerb - 15 to | | m | |
| | | | | |
| 0.2 | Barrier type kerb - 23 to The minimum length of overtaking zone i | | | s ner IRC is |
| 23. | | | $3 \times OSD$ | |
| | | (0) | , 5 . 000 | (4) 5 002 |
| | Ans. (c) | | | • |

| 32. | 2. In the design of filter material for su | bsurface drainage of roads D15 filter/D15 of |
|-----|--|--|
| | foundation should be | |
| | (a) less than 5 (b) greater th | an 5 (c) less than 10 (d) greater than 10 |
| | Ans. (b) | |
| 33. | 3. While planning plantation of trees a | llong road side, minimum distance of trees |
| | should be | |
| | (a) 1.5 m from edge, 10 m from cent | re of road |
| | (b) 1.8 m from edge, 12 m from cent | re of road |
| | (c) 2.0m from edge, 15 m from cent | re of road |
| | (d) 2.5 m from edge, 20 m from cent | re of road |
| | Ans. (b) | |
| 34. | 4. As per IRC, the axle load of vehicle | should not exceed. |
| | (a) 5000 kg (b) 7500 kg | (c) 10,000 kg (d) 12000 kg |
| | Ans. (c) | |
| 35. | | icle is |
| | (a) 2.0 m (b) 2.44 m | (c) 2.75 m (d) 3.0 m |
| | Ans. (b) | |
| 36. | 66. As per IRC, maximum height of sin | gle deck vehicle is |
| | (a) 2.44 m (b) 3.2 m | (c) 3.82 m (d) 4.72 m |
| | Ans. (c) | |
| 37. | 1 4t -62 a | xle vehicle is |
| | (a) 7.5 m (b) 10.67 m | |
| | Ans. (b) | |
| 38. | 38. If I is the wheel base and R is the | adius of the curve, the off tracking will be |
| | 12 | (c) $\frac{l^2}{2R}$ (d) $\frac{2l^2}{R}$ |
| | (a) $\frac{l}{2R}$ (b) $\frac{l}{R}$ | (c) $\frac{1}{2R}$ (u) R |
| | Ans. (c) | Ponnico Innovativo |
| 39 | 39. The hourly volume considered for d | lesign of traffic facilities is |
| | (a) 30 th (b) 85 th | (c) 98^{th} (d) 50^{th} |
| | Ans. (a) | |
| 40 | 40. On highways safe speed limit is tak | en as |
| | (a) 30 th percentile speed | (b) 85 th percentile speed |
| | (c) 98 th percentile speed | (d) 50 th percentile speed |
| | Ans. (b) | |
| 41 | 41. Design speed for highway is taken | as |
| 71 | (a) 30 th percentile speed | (b) 85th percentile speed |
| | (c) 98 th percentile speed | (d) 50th percentile speed |
| | (0) 70 Porcerume above | |
| | | |

Highway Engineering

| | | (c) 15 th percentile speed | | | | | | |
|-----|--|---------------------------------------|--------------------|--|--|--|--|--|
| 42. | For parking the ratio of no. of bays occupied to the number of bays available is | | | | | | | |
| | called | l | | | | | | |
| | (a) I | Parking load | (b) parking volume | | | | | |
| | (c) I | parking accumulation | (d) parking index | | | | | |

43. The width and length of segments of centre line marked on divided highway is (c) 0.15 m, 2 m (d) 0.15 m, 3 m

(a) 0.1 m, 2 m

(b) 0.1 m, 3m

Ans. (b)

Ans. (d)

44. Colours used for kerb markings are

(a) white

(b) yellow

(c) black

(d) alternate bands of white and black

Ans. (d)

45. Colours used for carriage way markings are

(a) white

(b) yellow

(c) black

(d) green

Ans. (a)

46. The Indian practice is to have an amber period of in traffic signal.

(a) 10 sec

(b) 8 sec

(c) 5 sec

(d) 2 sec

Ans. (d)

47. The method for signal cycle time design was suggested by

(a) Webster

(b) Burmister

(c) Westergaard

(d) Newmark

Ans. (a)



Bridge Engineering

| 1. | An ideal bridge sight has | |
|-----|---|--|
| | (a) Stream is well defined and narrow | (b) Square alignment |
| | (c) Firm and permanent river banks | (d) all of the above |
| | Ans. (d) | |
| 2. | For a bridge, $l = clear span$, $n = number$ | r of spans and b = width of pier, then |
| | length of bridge is | |
| | (a) $L = n \times l + (n + 1) b$ | (b) $L = n \times l + (n - 1) b$ |
| | (c) $L = nl$ | (d) $L = nl + nb$ |
| | Ans. (b) No. of piers $= n - 1$ | |
| 3. | Linear waterway for a river at bridge sit | e is equal to |
| | (a) Length of the bridge | (b) Sum of all clear spans |
| | (c) Sum of all clear spans + sum of width | of piers |
| | (d) Sum of all clear spans + width of two | abutments |
| | Ans. (b) | |
| 4. | Lacey's regime formula for linear water | |
| | (a) $L = C$. Q (b) $L = \frac{C}{Q}$ | (c) $L = C\sqrt{Q}$ (d) $L = \sqrt{C}Q$ |
| | Ans. (c) | |
| | Where, $C = constant (4.5 to 6.3)$ | |
| | Q = design flood discharge (| m^3/s) |
| 5. | Increase in water level at the bridge site | due to obstruction to the flow is called |
| | (a) Free board (b) afflux | (c) Scour (d) HFL |
| | Ans. (b) | |
| 6. | | evel bridge is |
| | (a) 300 mm (b) 500 mm | (c) 600 mm (d) 1000 mm |
| . • | Ans. (c) | |
| 7. | If Q is the discharge in m ³ /s, the regime | width of stream is given by |
| | (a) 4.8 Q (b) 4.8 \sqrt{Q} | (c) 1.76 Q (d) 1.76 \sqrt{Q} |
| | Ans. (b) | |
| 8. | The normal scour depth is given by, | |
| | (a) $d = 0.473 \left(\frac{Q}{f}\right)^{\frac{1}{3}}$ (b) $d = 0.473 \left(\frac{Q}{f}\right)^{\frac{1}{3}}$ | $\frac{Q}{f}$ $\int_{1}^{\frac{1}{2}} (c) d = 4.8 \left(\frac{Q}{f}\right)^{\frac{1}{3}} (d) d = 4.8 \left(\frac{Q}{f}\right)^{\frac{1}{2}}$ |

f = Lacey's silt factor

Ans. (a)

If P = cost of one pier with its foundation and $a_1 = constant$ for a super structure 9. then the economic span of the bridge is given by:

(a)
$$l = \frac{2P}{a_1}$$

(b)
$$l = \sqrt{\frac{2P}{a_1}}$$
 (c) $l = \frac{P}{a_1}$ (d) $l = \sqrt{\frac{P}{a_1}}$

(c)
$$l = \frac{P}{a_1}$$

(d)
$$l = \sqrt{\frac{P}{a_1}}$$

Ans. (d)

10. In a suspension bridge, dip is usually taken as

(a)
$$\frac{1}{5}^{th}$$
 of span

(b)
$$\frac{1}{10}^{th}$$
 of span

(a)
$$\frac{1}{5}$$
 of span (b) $\frac{1}{10}$ of span (c) $\frac{1}{15}$ of span (d) $\frac{1}{20}$ of span

(d)
$$\frac{1}{20}$$
 of span

Ans. (b)

11. In case of a balanced cantilever bridge, the cantilever span is usually taken as

- (a) 5 to 10% of the supported span
- (b) 10 ti 15% of the supported span
- (c) 15 to 20% of the supported span
- (d) 20 to 25% of the supported span

Ans. (d)

12. 'Viaduct' is a

- (a) Small bridge constructed over small stream
- long continuous structure supported on trestle bents over a dry valley
- bridge structure constructed at the crossing of railway line and road.
- (d) tunnel for a roadway

Ans. (b)

13. The bearing which permit horizontal movement as well as rotation is

(a) Rocker bearing

(b) Rocker - roller bearing

(c) shallow plate bearing

(d) Tar paper bearing

14. The width of expansion joint in highway bridges is usually

- (a) 10 mm
- (b) 25mm
- (c) 40 mm
- (d) 50 mm

Ans. (b)

15. The minimum width of carriage way for two lane bridge is

- (a) 4.25 m
- (b) 7.5 m
- (c) 8.4 m
- (d) 10.0 m

Ans. (b)

16. Which of the following loading class for bridges is the highest loading

- (a) class AA
- (b) class A
- (c) close B
- (d) class 70 R

Ans. (d)

Live loads for various loadings are:

class 70 R = tracked vehicle 700 kN and wheeled vehicle 1000 kN

class AA = tracked vehicle 700 kN and wheeled vehicle 400 kN

| 17. | For IRC loading 70 R | and AA, the impact | factor is taken as | (up to 9m span) |
|-----|-----------------------------------|---|---------------------------------|-------------------------|
| | (a) 10% (b) | 20 % | (c) 25 % | (d) 30% |
| | Ans. (c) | | | _ |
| 18. | The wind load acting | on moving live load | will be assumed t | to act at a height of |
| | 1.5 m above the roady | vay, its value for ord | linary bridge is | |
| | (a) 1.5 kN/m | (b) 2.0 kN/m | (c) 3.0 kN/m | (d) 4.0 kN/m |
| | Ans. (c) | | | |
| 19. | Bridge railings and pa | rapets should be des | igned to resist a lat | teral horizontal force |
| | and vertical force eac | h of acting sin | multaneously. | |
| | (a) 1.0 kN/m | (b) 1.5 kN/m | (c) 2.0 kN/m | (d) 2.5 kN/m |
| | Ans. (b) | | | |
| 20. | Dicken's formula to d | etermine discharge i | is 1/4 | (n) a = 0.1/3 |
| | (a) $Q = CA^{3/4}$ | (b) $Q = CA^{2/3}$ | (c) $Q = CA^{1/4}$ | (d) $Q = CA^{ng}$ |
| | Ans. (a) | | | |
| | Ryve's formula is | | | |
| | $Q = CA^{2/3}$ where, | $Q = discharge (m^3)$ | | |
| | | A = area of catchm | | |
| 21. | The length of the d/s | portion of the guide | bank should be | 1 |
| | (a) $\frac{1}{2}$ × bridge length | h | (b) $\frac{1}{3}$ × bridge | |
| | (c) — x nringe lengt | | (d) $\frac{3}{5}$ × bridge | |
| | Ans. (c) The | top width of guide ba | ank should not be is | ess than 3.0 iii |
| 22. | Which of the following | | work | (d) all of the above |
| | (a) guide banks | (b) spurs | (c) cutoffs | (d) all of theabove |
| | Ans. (d) | | 11 | and an angle with the |
| 23. | | | ks generally subte | nu an angle with the |
| | centre line of bridge | equal to | (2) 1209 1459 | (d) 145° = 180° |
| | (a) $0 - 45^{\circ}$ | (b) $45^{\circ} - 90^{\circ}$ | (c) $120^{\circ} - 143^{\circ}$ | (u) 143 - 100 |
| | Ans. (c) | 1 1 -1-450 4 | a 000 with the cent | re line of bridge |
| n n | The d/s curved h | ead make angle 45° to | rant the intensity | of water pressue is. |
| 24. | On piers, parallel to | the direction of curves -0.5 kg^2 | (c) $p = 1.5 \text{ kv}^2$ | 2 (d) $p = kv^3$ |
| | (a) $p = kv^2$ | (b) $p = 0.5 \text{ KV}$ | .(с) р 1.5 к. | (m) P |
| | Ans. (b) | | | . (|
| | p in kN/m ² | | | |
| | v in m/s | 1 | | • |
| | k = constan | l Luiden niove mey be t | aken parallel to the | e carriage way equal to |
| 25 | | bridge piers may be t | (c) 750 kN | (d) 1000 kN |
| | (a) 250 kN | (b) 500 kN load perpendicular to | | * * |
| | Ans. (d) Collision | load perpendicular o | | |
| · | • | | • | |
| | | | | |

Railway Engineering

| | | <u>SE 1-1</u> | | | |
|------|-----------------------------|-------------------------------------|---------|-----------------|---------------------|
| 1. | In India, the first tra | ain was run between | | | |
| | (a) Bombay and Calc | utta | (b) | Delhi and Bon | nbay |
| | (c) Bombay and Than | ne | (d) | Delhi and Cul | cutta |
| 2. | Total number of zone | es in w <mark>hi</mark> ch Indian R | ailwa | ys is distribut | ed |
| | (a) 5 | (b) 11 | (c) | 9 | (d) 16 |
| 3. | The Broad Gauge is | wide. | | | |
| | (a) 1.676 m | (b) 1.00 m | (c) | 0.762 m | (d) 0.610 m |
| 4. | Railways originated in | n | | | |
| | (a) U.S.A. | (b) England | (c) | U.S.S.R. | (d) Germany |
| 5. | The first Indian rails | vay was laid in | | | |
| | (a) 1825 | (b) 1876 | (c) | 1853 | (d) 1804 |
| 6. | Gauge of a permanen | it way, is | | | |
| | (a) Minimum distance | between inner faces | of rai | ils. | |
| | (b) Minimum distance | between outer faces | of rai | ils . | |
| | (c) Distance between | centres of rails | | | |
| | (d) Width of formatio | n | | | |
| 7. | Now a days, the rail | section on Indian Ra | ailway | s is | nnvatiz |
| | (a) Double headed | (b) Bull headed | (c) 1 | Dumb bell type | e (d)Flat footed |
| 8. | Generally, in India, th | ie axle load on rail is | take | n | the weight of rail |
| | per metre. | | - | | |
| | | • • | | 350 times | • • |
| 9. | On Indian Railways, | standard length of ra | ails fo | or BG track i | S |
| | | (b) 11.89 m (39 ft) | | | (d) 10.06 m (33 ft) |
| 10. | On wooden sleepers a | dzing is done to give | e a sl | ope of | |
| | (a) 1 in 10 | (b) 1 in 25 | (c) 1 | l in 20 | (d) 1 in 35 |
| 11. | The height of 60 kg l | FF rail is | | | |
| | (a) 128 mm (| (b) 172 mm | (c) 1 | 156 mm | (d) 150 mm |
| Obje | ctive Civil Eng. \ 2016 \ 3 | 88 | | | |
| - | <i>-</i> . , | | | | |

| 12. | On B.G. track the standard size of wooden sleeper used is |
|-----|---|
| | (a) $150 \text{ cm} \times 18 \text{ cm} \times 11.5 \text{ cm}$ (b) $180 \text{ cm} \times 20 \text{ cm} \times 11.5 \text{ cm}$ |
| | (c) $275 \text{ cm} \times 25 \text{ cm} \times 12.5 \text{ cm}$ (d) none of the above |
| 13. | The name of railway station where all four gauges exist is |
| | (a) Sasaram (b) Giridih (c) Shimla (d) Banglore |
| 14. | The trade of wheel is provided an outward slope of |
| | (a) 1 in 10 (b) 1 in 15 (c) 1 in 20 (d) 1 in 25 |
| 15. | · · · · · · · · · · · · · · · · · · · |
| | (a) on outer side (b) on inner side (c) on both sides (d) on neither side |
| 16. | The head quarter of central railway is at |
| | (a) Mumbai Central (b) Mumbai VT (c) Maligaon (d) Gorakhpur |
| 17. | The head quarter of Eastern Railway is at |
| | (a) Chennai (b) Kolkata (c) Maligaon (d) Secunderabad |
| 18. | The longest platform of the world on B.G. is at |
| | (a) Sonepur (b) Allahabad (c) Kharagpur (d) Sasaram |
| 19. | The standard depth of ballast for BG track on Indian Railway is |
| | (a) 22 cm (b) 25 cm (c) 30 cm (d) 35 cm |
| 20. | Fish bolts are made up of |
| | (a) High carbon steel (b) Low carbon steel (c) Cast Iron (d) Stainless steel |
| 21. | Wear of rail is maximum in |
| • | (a) Tangent curve (b) Sharp curve (c) Tunnels (d) Coastal area |
| 22. | The tracks having gradients more than come under mountain |
| | railways. |
| 22 | (a) 10% (b) 8% (c) 5% (d) 3% |
| 45. | To connect one pair of fish plates, the number of bolts used are |
| 24 | (a) 2 (b) 4 (c) 6 (d) 8 The maximum quadient for P.C. is station would be |
| 44. | The maximum gradient for B.G. in station yards is |
| 25 | (a) 1 in 1000 (b) 1 in 400 (c) 1 in 200 (d) 1 in 100 |
| 23. | The minimum gradient permitted for good drainage of water in station yards on B.G. should be |
| | |
| 26 | (a) 1 in 1000 (b) 1 in 500 (c) 1 in 400 (d) 1 in 200 The relation between the radius of aures (D) and its degree of aures (D) is |
| au. | The relation between the radius of curve (R) and its degree of curve (D) is given by |
| | · · · · · · · · · · · · · · · · · · · |
| | (a) $D = \frac{1720}{R}$ (b) $D = \frac{1580}{R}$ (c) $D = \frac{1850}{R}$ (d) $D = \frac{1786}{R}$ |
| 27. | For fixing rails on wooden sleepers, the commonly used spikes are |
| | (a) Screw spike (b) Elastic spike (c) Dog spike (d) Round spike |

| 43. Advance starter | signal is used for | | |
|------------------------|-------------------------|----------|----------------------------------|
| (a) Shunting ope | | (b) | Goods trains |
| (c) Loco sheds | | (d) | None of the above |
| 4. Rails are general | ly discarded if | | · |
| (a) Web or head | | (b) | Gauge side is worn by 3 mm |
| (c) wear of rail | | (d) | Any of the above takes place |
| ` ' | concrete sleeper by | | |
| (a) Screw spike | (b) Pandrol clip | (c) | lug and key (d) none of the abov |
| • , | v the sleeper is called | | |
| (a) Ballast cushi | on (b) Crib ballast (| c) Sho | ulder bailast (d) Heavy ballas |
| 47. The total friction | al resistance independe | nt of th | e speed can be obtained by |
| (a) 0.0016 W | | (b) | |
| (c) 0.000017 AV | 2 | (d) | none of the above |
| 48. The curve resista | nce for 50 tonne train | on BG | track on a 4° curve will be |
| (a) 0.05 tonnes | (b) 0.06 tonnes | (c) | 0.08 tonnes (d) 0.10 tonnes |
| | | in pla | aces where the track is provide |
| with a | | | |
| (a) Ruling gradi | | (b) | momentum gradient |
| (c) Pusher gradi | | (d) | Limiting gradient |
| | signal seen by the d | river is | known as |
| (a) Home signal | | (c) | Routing signal (d) Outer signal |
| 51. Generally, when | a train passes throug | h a sta | tion without stopping, the drive |
| | signals in the sequen | | |
| (a) Warner, oute | | (b) | |
| • • | r, starter, home | (d) | outer, warner, starter, home |
| | ed ballast width for B | G is | |
| (a) 3650 mm | (b) 2290 mm | (c) | 3350 mm (d) 2750 mm |
| \ <i>\</i> | ed as a ballast for | | |
| (a) wooden slee | pers (b) Steel sleeper | s . (c) | CI sleepers (d) all the abov |
| 54. The sleepers wh | ich satisfy the require | ments | of an ideal sleeper, are |
| (a) CI sleeper | (b) RCC sleeper | | Steel sleeper (d) Wooden sleeper |
| | tion on curve on BG | | |
| (a) 0.05% | (b) 0.04 % | | 0.03 % (d) 0.02 % |
| • • | urvature allowed is m | , , | • |
| (a) BG | (b) MG | (c) | NG (d) All the above |
| (4) 10 | (0) | (1) | • • |

Railway Engineering

57. The negative superelevation is provided on a

- (a) branch line
- (b) main line
- (c) transition curve (d) None of the above

58. Rack railway is provided on

(a) Steep gradient

(b) Mild gradient

(c) Vertical gradient

Horizontal surface (d)

59. Bending of rails for laying curvatures is resorted when the degree of curvature is more than

- (a) 3°
- (b) 4°
- (c)
- (d) 6°

60. A hump yard is a type of

(a) Passenger yard

Marshalling yard (b)

(c) Goods yard

Retarder (d)

: ANSWERS:

| 1.) | С | 2. | d | 3. | a | 4. | ь | 5. c |
|-----|---|-------------------|---|-------|-----|--------------------|---|---------------------------------------|
| 6. | a | 7. | d | 8. | b | 9. | a | 10. c |
| 11. | b | 12. | С | 13. | b | 14. | c | 15. b |
| 16. | b | 17. | b | 18. | c | 19. | b | 20. a |
| 21. | b | 22. | d | 23. | b | 24. | b | 25. a |
| 26. | a | 27. | С | 28. | d | 29. | С | 30. a |
| 31. | d | 32. | С | 33. | a | 34. | a | 35. b |
| 36. | a | 37. | ь | 38. | С | 39. | a | 40. c |
| 41. | С | <mark>42</mark> . | a | 43. | a | 44. | d | 45. b |
| 46. | a | 47. | a | . 48. | c · | 49. | b | 50. d |
| 51. | a | 52. | С | 53. | c | 54. | d | 55. b |
| 56. | С | 57. | a | 58. | a | 59. | a | 60. b |
| | | | | | | ~~~~~~~ | | · · · · · · · · · · · · · · · · · · · |



301

302

<u>SET-2</u>

| 1. | | toes of both the | toun | ge rails are con | inected | 1 together n | y mean | S 01 A | piate Tech |
|---|---|---|---|---|---|---|---|--|---|
| | whic | h is called | | | | | (45.74) | ` | (ES) |
| | (a) | Stretcher bar | (b) | gauge tie bar | (c) | tie bar | ` ' | ounge ra | |
| 2. | | idth of sleeper is | | | | | | _ | (IES) |
| | (a) | $\frac{S-w}{2}$ | (b) | $\underline{\mathbf{w} - \mathbf{S}}$ | (c) | $S - \frac{W}{2}$ | (d) \ | $v - \frac{S}{2}$ | |
| | (a) | 2 | (0) | 2 | ` ' | 2 | | | (TTICL) |
| 3. | | rn table on railw | | | | | | | (IES) |
| | | Preventing the lat | | | els | | | | |
| | (b) | Reversing the dir | ection | of the engine | | | | | |
| | (c) | reduing the dama | ge to | the rails | | | | | |
| | | reducing the acci | | | | | | | (TEC) |
| 4. | Can | t deficiency is th | e diff | erence between | | | | | (IES) |
| | (a) | actual cant provi | ded at | the time of cons | tructio | on and at the | time of | renewa | |
| | (b) | equilibrium cant | neces | sary for the maxi | mum s | peed and act | ual cant | provide | ed |
| | (c) | Cant required at | maxir | num speed and m | inimu | m speed | | | |
| | (d) | two parallel rails | after | 10 years | | | | | (==== G) |
| 5. | Peri | missible limit of | cant o | deficiency for B. | G. is | | | | (IES) |
| | | | | | | | | | |
| | (a) | 50 mm | (b) | 60 mm | (c) | 75 mm | • • | 88 mm | _ |
| 6. | (a) | 50 mm | (b) | 60 mm | (c) | | • • | | .G. on |
| | (a) As p | 50 mm per practice of Inves is | (b) ıdian | 60 mm Railways the gra | (c) ade co | mpensation | provide | d for B | (IES) |
| | (a) As p curv (a) | 50 mm per practice of Inves is 0.5 % per degree | (b) (dian (b) | 60 mm Railways the gra 0.2 % per degree | (c) ade co (c) 0.4 | mpensation 4 % per degree | provided | d for B | (IES) legree |
| | (a) As p curv (a) | 50 mm per practice of Inves is 0.5 % per degree | (b) (dian (b) | 60 mm Railways the gra 0.2 % per degree | (c) ade co (c) 0.4 | mpensation 4 % per degree | provided | d for B | (IES) iegree med is |
| 6. | (a) As p curv (a) | 50 mm per practice of Inves is 0.5 % per degree ailway yard in v | (b) Idian (b) Which | 60 mm Railways the gra 0.2 % per degree wagons are rec | (c) ade co (c) 0.4 eived, | mpensation 4 % per degree sorted and | provided e (d) 0.15 trains a | d for B % per c | degree med is (IES) |
| 6. | (a) As p curv (a) A r call (a) | 50 mm per practice of Inves is 0.5 % per degree ailway yard in ved goods yard | (b) Idian (b) Which (b) | 60 mm Railways the gra 0.2 % per degree wagons are rec Station yard | (c) ade co (c) 0.4 eived, | mpensation 4 % per degree sorted and Marshalling y | provided (d) 0.15 trains a | d for B % per c are for Shuntin | degree med is (IES) g yard |
| 6. | (a) As p curv (a) A r call (a) | 50 mm per practice of Inves is 0.5 % per degree ailway yard in ved goods yard | (b) Idian (b) Which (b) | 60 mm Railways the gra 0.2 % per degree wagons are rec Station yard | (c) ade co (c) 0.4 eived, | mpensation 4 % per degree sorted and Marshalling y | provided (d) 0.15 trains a | d for B % per c are for Shuntin | diegree med is (IES) g yard vitches |
| 6.7. | (a) As positive (a) A recall (a) When in a | 50 mm per practice of Inves is 0.5 % per degree ailway yard in ved goods yard ich of the following railway yard? | (b) dian (b) which (b) ng sig | 60 mm Railways the gra 0.2 % per degree wagons are rec Station yard gnals is provided | (c) 0.4 (c) 0.4 eived, (c) 1 beyon | mpensation 4 % per degree sorted and Marshalling y d the trailin | provided e (d) 0.15 trains a eard (d) g points | d for B % per c nre for Shuntin and sv | iegree med is (IES) g yard vitches (IES) |
| 6.7. | (a) As positive (a) A recall (a) When in a (a) | 50 mm per practice of Inves is 0.5 % per degree ailway yard in ved goods yard ich of the following railway yard? Repeater signal | (b) adian (b) which (b) ng sig | 60 mm Railways the gra 0.2 % per degree wagons are rec Station yard gnals is provided Departure signa | (c) ade co (c) 0.4 eived, (c) 1 beyon | mpensation 4 % per degree sorted and Marshalling y 1d the trailin advance sta | provided e (d) 0.15 trains a eard (d) g points | d for B % per c nre for Shuntin and sv | degree med is (IES) g yard vitches (IES) g signal |
| 6.7. | (a) As positive (a) A recall (a) When in a (a) | 50 mm per practice of Inves is 0.5 % per degree ailway yard in ved goods yard ich of the following railway yard? Repeater signal | (b) adian (b) which (b) ng sig | 60 mm Railways the gra 0.2 % per degree wagons are rec Station yard gnals is provided Departure signa | (c) ade co (c) 0.4 eived, (c) 1 beyon | mpensation 4 % per degree sorted and Marshalling y 1 the trailin advance statallings is | provided e (d) 0.15 trains a eard (d) g points | d for B % per c nre for Shuntin and sv | iegree med is (IES) g yard vitches (IES) |
| 6.7.8. | (a) As positive (a) A recall (a) When in a (a) | 50 mm per practice of Inves is 0.5 % per degree ailway yard in ved goods yard ich of the following railway yard? | (b) adian (b) which (b) ng sig | 60 mm Railways the gra 0.2 % per degree wagons are rec Station yard gnals is provided Departure signa | (c) ade co (c) 0.4 eived, (c) 1 beyon | mpensation 4 % per degree sorted and Marshalling y 1d the trailin advance state ailways is Spiral | provided (d) 0.15 trains a rard (d) g points | d for B % per correction Shunting and sv Routing | degree med is (IES) g yard vitches (IES) g signal |
| 6.7.8. | (a) As p curv (a) A r call (a) Wh in a (a) The (a) (c) | per practice of Inves is 0.5 % per degree railway yard in ved goods yard ich of the following railway yard? Repeater signal e shape of transit cubic parabola Sine curve | (b) dian (b) which (b) ng sig (b) tion c | 60 mm Railways the gra 0.2 % per degree wagons are rec Station yard gnals is provided Departure signa urve used by Inc | (c) ade co (c) 0.4 eived, (c) 1 beyon (c) 1 (d) | mpensation 4 % per degree sorted and Marshalling y d the trailin advance state ailways is Spiral Lemniscate | provided e (d) 0.15 trains a rard (d) g points rter (d) | d for B % per correction Shunting and sv Routing | degree med is (IES) g yard vitches (IES) g signal |
| 6.7.8. | (a) As p curv (a) A r call (a) Wh in a (a) The (a) (c) | 50 mm per practice of Inves is 0.5 % per degree railway yard in ved goods yard ich of the following railway yard? Repeater signal e shape of transit cubic parabola | (b) dian (b) which (b) ng sig (b) tion c | 60 mm Railways the gra 0.2 % per degree wagons are rec Station yard gnals is provided Departure signa urve used by Inc | (c) ade co (c) 0.4 eived, (c) 1 beyon (c) 1 (d) | mpensation 4 % per degree sorted and Marshalling y d the trailin advance state ailways is Spiral Lemniscate with respec | provided (d) 0.15 trains a eard (d) g points reter (d) of Bern t to | d for B % per correction Shunting and sv Routing | degree med is (IES) g yard vitches (IES) g signal (IES) |
| 6.7.8.9. | (a) As positive (a) A recall (a) When (a) The (a) (c) Me | per practice of Inves is 0.5 % per degree railway yard in ved goods yard ich of the following railway yard? Repeater signal e shape of transit cubic parabola Sine curve | (b) Idian (b) Which (b) Ing signification continues | 60 mm Railways the gra 0.2 % per degree wagons are rec Station yard gnals is provided Departure signa urve used by Inc | (c) ade co (c) 0.4 eived, (c) 1 beyon (c) 1 (d) | mpensation 4 % per degree sorted and Marshalling y d the trailin advance state ailways is Spiral Lemniscate | provided (d) 0.15 trains a eard (d) g points reter (d) of Bern t to | d for B % per correction Shunting and sv Routing | degree med is (IES) g yard vitches (IES) g signal (IES) |
| 6.7.8.9. | (a) As p curv (a) A r call (a) Wh in a (a) The (a) (c) Me (a) | per practice of Inves is 0.5 % per degree railway yard in ved goods yard ich of the following railway yard? Repeater signal e shape of transit cubic parabola. Sine curve tal sleepers are s | (b) adian (b) which (b) ng sig (b) cion c | 60 mm Railways the gra 0.2 % per degree wagons are rec Station yard gnals is provided Departure signa urve used by Inco | (c) 0.4 eived, (c) 1 beyon (b) (d) | mpensation 4 % per degree sorted and Marshalling y d the trailin advance state ailways is Spiral Lemniscate with respec | provided (d) 0.15 trains a rard (d) g points reter (d) of Bern t to ting (d) | d for B % per correction Shunting and sv Routing | degree med is (IES) g yard vitches (IES) g signal (IES) |

| 12. | For a sleeper density of $(n + 5)$, the number of sleepers required for | constructing |
|-----|--|--------------|
| | a B.G. track of length 650 m is | (IES) |

(a) 975

(b) 918

(c) 900

(d) 880

Hint: Length of B.G. = 12.8 m ∴ No. of rails required = $\frac{650}{12.8} \approx 50$ Sleeper density = n + 5

$$= 12.8 + 5 \approx 18$$

total no. of sleepers = $50 \times 18 = 900$ nos

13. On railway tracks, corrugations normally occur on stretches where

(a) trains stop or start

(b) steel sleepers are used

(c) there are horizontal curves

(d) there are vertical curves

14. Match List I and List II and select the correct answer using the codes given below the lists: (IES)

List I List II To lift rails 1. Shovel A. To remove dog spikes out of sleepers 2. B. Crow bars To correct track alignment 3. C. Rail tongs Claw bars To handle ballast D.

Codes:

- 15. The distance between running faces of the stock rail and gauge face of tongue rail measured at the heel of the switch is called?
 - (a) flange way clearance

(b) throw of switch

(c) heel divergence

(d) flare

16. Rail gauge is the distance between

(a) Outer faces of rails

- (b) running faces of rails
- (c) centre to centre of rails
- (d) none of these
- 17. Top of rails of a track are placed at an inward slope of
 - (a) 1 in 10
- (b) 1 in 15
- (c) 1 in 20
- (d) 1 in 25

18. Coning of wheels is provided

- (a) to check the lateral movement of rail, (b) to avoid damage to inner faces of rails
- (c) to avoid discomfort to passengers
- (d) all of these

| 19. | Bull headed rails are generally provided | at | | | |
|-------------|--|---------|--------------------------------------|--|--|
| | (a) Curved tracks | (b) | Points and crossings | | |
| | (c) Bifurcation of tracks | (d) | bridges | | |
| 20. | The creep in rails is measured in | | · | | |
| | (a) kg/cm ² (b) kg/cm | (c) | kg-cm (d) cm | | |
| 21. | Creep is the | | | | |
| | (a) longitudinal movement of rail | (b) | lateral movement of rail | | |
| | (c) Vertical movement of rail | (d) | None of these | | |
| 22. | Creep is greater | | | | |
| | (a) on curves | (b) | in new rails than old rails | | |
| | (c) both (a) and (b) | (d) | None of these | | |
| 23. | The extra rails provided over a bridge to | prev | ent damage due to derailment on | | |
| | the bridge are called | | | | |
| | (a) Check rails (b) guard rails | ` ' | torque rails (d) wing rails | | |
| 24. | When rails go out of their position due to | | | | |
| | (a) hogging (b) creeping | | buckling (d) none of these | | |
| 25. | Due to battering action of wheels, over | | | | |
| | down and deflected at ends, these rails a | | | | |
| | (a) hogged rails (b) guard rails | (c) | torque rails (d) none of these | | |
| 26. | Rails are welded by | | C | | |
| | (a) gas welding (b) Arc welding | | | | |
| 27. | The misalignment of rails due to tempera | | | | |
| | (a) hogging (b) Creeping | | buckling (d) bulging | | |
| 28. | The Indian practice is to weld maximum | | | | |
| | (a) 2 rail lengths (b) 3 rail lengths | (c) | 4 rail lengths (d) 5 rail lengths | | |
| 29. | enice L'ducietièm II | (1.) | | | |
| | (a) Supported on sleeper | (b) | | | |
| | (c) left suspended | (d) | Supported on ballast | | |
| 30. | When two different rail sections are joint | ted tog | gether by means of itsh plates, the | | |
| | joint is called | · (-) | Communication to (d) Staggard ignit | | |
| | | | Compromise joint (d) Staggered joint | | |
| 31. | When rail ends rest on a joint sleeper jo | | | | |
| 20 | | (c) | suspended rail joint(d) Base joint | | |
| <i>5</i> 2. | Ideal sleepers are | | CI alamana (4) Wiladan alaanana | | |
| | (a) RCC sleepers (b) Steel sleepers | (c) | CI sleepers (d) Wooden sleepers | | |
| 33. | Best wood for wooden sleeper is | | T-1 (N C-sahan- | | |
| | (a) Chir (b) Sal | (c) | Teak (d) Seasham | | |

| 34. | Sleepers preferred on joints are | | | |
|-----|--|--------------|--------------------------------------|------|
| | (a) Wooden sleepers (b) Concrete sleepers | (c) | Steel sleepers (d) CST - 9 Sleepers | |
| 35. | Standard size of wooden sleeper for B.G. t | track | is | |
| | (a) $274 \times 25 \times 13$ cm | (b) | $183 \times 20 \times 11$ cm | |
| | (c) $152 \times 15 \times 10 \text{ cm}$ | (d) | $250 \times 26 \times 12$ cm | |
| 36. | Spacing of sleepers is kept | | | |
| | (a) same throughout the length of rail | (b) | closer near the joints | |
| | (c) closer at the middle of rails | (d) | none of these | |
| 37. | Sleepers providing best elasticity is | | | |
| | (a) wooden (b) RCC | (c) | Steel (c) CI | |
| 38. | Sleepers providing best rigidity of track is | | | |
| | (a) Wooden (b) RCC | (c) | Steel (d) CI | |
| 39. | The best suited material for the ballast is | | | |
| | (a) broken stone | (b) | gravel or river pebbles | |
| | (c) ashes or cinders | (d) | brick ballast | |
| 40. | Pandrol clip can not be used with | | | |
| | (a) wooden sleepers (b) Concrete sleeper | | | |
| 41. | Arrangement made to divert the trains fro | m or | | |
| | (a) railway crossing (b) railway junction | (c) | turnout (d) none of these | |
| 42. | The switch giving best performance is | | | |
| | (a) undercut switch | (b) | | |
| | (c) Over riding switch | (d) | none of these | |
| 43. | In a diamond crossing, number of noses a | | | |
| | (a) 3 (b) 4 | (c) | X(// ()) | |
| 44. | A track adopted when a double track is to | be 1 | narrowed over a short distance o | f |
| | the track is called | 5 H I | | |
| | (a) Gauntleted track (b) ladder track | | | |
| 45. | When a number of parallel tracks are bra | nche | d off from the straight track, it is | \$. |
| | called | | Dealth alia tarada (d) Multi tarad | |
| | (a) Gauntleted track (b) Ladder track | ` ' | Double slip track (d) Multi track | • |
| 46. | Distance between adjacent faces of the sto | | | |
| | | | flangeway clearance | |
| | (a) heel clearance | ` ′ | | |
| | (c) throw of switch | (d) | flare | |
| 47. | (c) throw of switch | (d) an ra | flare | |

Objective Civil Eng. \ 2016 \ 39

| | |
|---|--|
| Holes for fish bolts should be | |
| (a) drilled (b) hot-punched (c) Cold-punched (d) Screwed | |
| In station the gradient provided is | |
| (a) zero per cent (b) equal to ruling gradient | |
| (c) Steeper than ruling gradient (d) Flatter than ruling gradient | |
| Advance starter signal is used for | |
| (a) Shunting (b) goods trains (c) locosheds (d) all of these | |
| The reception signal is | |
| (a) outer signal (b) home signal (c) Starter signal (d) both (a) and (b) |) |
| During foggy and cloudy weather, the signal used is | |
| | l |
| A station at which a railway line or one of its branches terminates is called | |
| (a) junction station (b) terminal station (c) flag station (d) Halts | |
| When semaphore signal is in horizontal position, it is said to be in | |
| (a) 'on' position which indicates 'stop' | |
| (b) 'on' position which indicates 'proceed' | |
| `` | |
| | |
| | |
| | |
| (c) 'off' position which indicates 'stop' (d) 'off' position which indicates 'proceed | , |
| Pandrol is an example of | |
| (a) Rail spike (b) fish bolt (c) Elastic fastening (d) bearing plate | |
| Bearing plate is used below | |
| (4) 11 1411 | ; |
| | |
| | |
| | IJ , |
| • | (|
| | |
| | |
| | |
| | |
| The test conducted on rails is | |
| (a) hammer test (b) tensile test (c) Falling weight test (d) both (a) and (b) | ١. |
| | (a) drilled (b) hot-punched (c) Cold-punched (d) Screwed In station the gradient provided is (a) zero per cent (b) equal to ruling gradient (b) Steeper than ruling gradient (c) Steeper than ruling gradient (d) Flatter than ruling gradient Advance starter signal is 'used for (a) Shunting (b) goods trains (c) locosheds (d) all of these The reception signal is (a) outer signal (b) home signal (c) Starter signal (d) both (a) and (b) During foggy and cloudy weather, the signal used is (a) Semaphore signal (b) warner signal (c) detonating signal (d) colour light signal A station at which a railway line or one of its branches terminates is called (a) junction station (b) terminal station (c) flag station (d) Halts When semaphore signal is in horizontal position, it is said to be in (a) 'on' position which indicates 'stop' (b) 'on' position which indicates 'stop' (d) 'off' position which indicates 'proceed' When semaphore signal is inclined at 45° to 60° below horizontal, it is said to be in (a) 'on' position which indicates 'stop' (b) 'on' position which indicates 'stop' (c) 'off' position which indicates 'stop' (d) 'off' position which indicates 'stop' (d) 'off' position which indicates 'stop' (e) 'off' position which indicates 'stop' (b) 'on' position which indicates 'stop' (c) 'off' position which indicates 'stop' (d) 'off' position which indicates 'stop' (e) 'off' position which indicates 'stop' (b) 'on' position which indicates 'stop' (b) 'on' position which indicates 'proceed' (c) 'off' position which indicates 'stop' (d) 'off' position which indicates 'stop' (e) 'off' position which indicates 'stop' (b) 'on' position which indicates 'proceed' (c) 'off' position which indicates 'stop' (e) 'off' position which indicates 'stop' (f) 'off' position which indicates 'proceed' (g) 'off' position which indicates 'stop' (h) 'off' position which indicates 'proceed' (g) 'off' position which indicates 'stop' (h) 'off' position which indicates 'proceed' (g) 'off' position which indic |

Railway Engineering

| 63. | 63. Number of MS keys used in CST - 9 sleepers are | | | | | | |
|-----|--|--------------------|-------|---------------------------|-------|----------------|--|
| | (a) 4 (b) 3 | 3 (| (c) | 2 | (d) | 5 | |
| 64. | Slope of platform across its | width is | | | | | |
| | (a) 1 in 40 (b) | 1 in 60 (| (c) | 1 in 30 | (d) | 1 in 45 | |
| 65. | The platform height above | rail level in BG i | is pr | ovided as | | aF | |
| | (a) 0.76 to 0.84 m (b) | 0.90 to 1.2 m (| (c) | 0.65 to 0.75 m | (d) | 1.05 to 1.15 m | |
| 66. | Two important constituents | in the compositi | on o | of steel used fo | r rai | ils are | |
| | (a) Carbon and sulphur | . (| (b) | Carbon and ma | ngar | iese | |
| | (c) Carbon and silicon | (| (d) | Manganese and phosphorous | | | |
| 67. | The reception signal is | | | | | | |
| | (a) Outer signal | (| b) | home signal | | | |
| | (c) both (a) and (b) | | (d) | advance starter | | | |
| 68. | Maximum value of 'throw of | of switch' for B.C | 3. tr | ack is | | | |
| | (a) 89 mm (b) 9 | 95 mm (| c) | 100 mm | (d) | 115 mm | |
| 69. | Generally side slope of emb | oankments for a r | railw | vay track is tal | ken : | as | |
| | (a) 1.5:1 (b) 2 | 2:1 | c) | 1:2 | (d) | 1:1.5 | |
| 70. | Limitations of superhigh sp | eed is/are | | | | | |
| | (a) wave formation | | b) | adhesion betwe | en w | heel and rails | |
| · | (c) vibrations | | d) | all of these | | | |
| | | | | | | | |

: ANSWERS:

| | | | | | | | 1 7 57 | | | |
|-----|-----|-----|-----|------|-----|-----|--------|-----|-----|---|
| 1. | (a) | 2. | (a) | 3. | (b) | 4. | (b) | 5. | (c) | |
| 6. | (b) | 7. | (c) | 8. | (c) | 9. | (a) | 10. | (b) | |
| 11. | (b) | 12. | (c) | 13. | (a) | 14. | (c) | 15. | (c) | |
| 16. | (b) | 17. | (c) | 18. | (d) | 19. | (b) | 20. | (d) | |
| 21. | (a) | 22. | (a) | 23. | (b) | 24. | (c) | 25. | (a) | |
| 26. | (d) | 27. | (c) | 28. | (d) | 29. | (c) | 30. | (c) | - |
| 31. | (b) | 32. | (d) | 33. | (c) | 34. | (a) | 35. | (a) | |
| 36. | (c) | 37. | (a) | 38. | (b) | 39. | (a) | 40. | (c) | ٠ |
| 41. | (c) | 42. | (c) | 43. | (d) | 44. | (a) | 45. | (b) | |
| 46. | (b) | 47. | (b) | 4,8. | (a) | 49. | (d) | 50. | (a) | |
| 51. | (d) | 52. | (c) | 53. | (b) | 54. | (a) | 55. | (d) | |
| 56. | (c) | 57. | (a) | 58. | (b) | 59. | (c) | 60. | (a) | |
| 61. | (c) | 62. | (c) | 63. | (c) | 64. | (c) | 65. | (a) | |
| 66. | (b) | 67. | (c) | 68. | (d) | 69. | (b) | 70. | (d) | |

Airport Engineering

MCO'S According to ICAO, markings on the runways are (d) red (c) black (b) yellow (a) white Which one of the following is used for servicing and repairs of the aircraft? (c) Terminal building (d) Holding apron (b) hanger Where the aircraft is standing (parking), the facility for loading and unloading 3. (IES) operation in front of terminal building is called (d) hanger (a) holding apron (b) apron The wind rose diagram (WRD) for orientation of airport runway give (Civil Services) (b) direction and duration of wind direction of wind direction, duration and intensity of wind (d) none of these Which one of the following imaginary surfaces in airport is circular in plan with centre located at an elevation of 150 m above the airport reference point? (IES) (b) Transitional surface Conical surface (d) Outer horizontal surface Inner horizontal surface ICAO has recommended that the basic runway length should be increased at the rate of per 300 m rise in elevation above mean sea level.(IES) (d) 15% (c) 10% (b) 7% 5% At a certain station, mean of the average temperature is 25°C and mean of the maximum daily temperature is 40°C. What is the airport reference temperature (ART)? (d) 30°C (b) 45°C (c) 35°C 46.6°C **Hint**: $T = T_1 + \frac{(T_2 - T_1)}{3}$ $= 25 + \frac{(40 - 25)}{3} = 30^{\circ}C$

8. A surface longitudinally centred on the extended runway centre line and extending outward and upward is called (IES)

(a) Primary surface (b) conical surface (c) Horizontal surface (d) approach surface

| 9 | The total correction percentage for altitude and temperature in calculating the |
|------------|--|
| <i>)</i> . | runway length from basic runway length, normally does not exceed (22%) |
| | (b) 14 (c) 28 (d) 33 |
| 1Λ | The best direction of a runway is (IES) |
| 10. | A largest line on the windrose diagram |
| | (h) along the direction perpendicular to the longest line on the whitelose diagram |
| | the longest line on the Wind rose diagram |
| | A NIVI CIVI line. |
| | (d) along the NW - SW line Type I windrose diagram used for the orientation of runway shows |
| 11. | (b) direction and duration of wind |
| | (a) direction of wind |
| | (IES) |
| 12. | Consider the following factors 2. Aircraft traffic composition |
| | 1. Air traine control incusares |
| | VERTER ODCIATION |
| | Which of these factors affect the capacity of a runway? (a) 1, 2 and 3 (b) 2 and 3 (c) 1 and 4 (d) 1, 2, 3 and 4 |
| | (a) 1, 2 and 3 (b) 2 and 3 (c) 1 and 4 (d) 1, 2, 4 and gives an |
| 13 | (a) 1, 2 and 3 (b) 2 and 3 (c) A VOR radio transmitter which emits beam in a vertical plane and gives an indication to the pilot whether he is to the left or right of the correct alignment (IES) |
| | indication to the pilot whether he is to the left of right. (IES) |
| | for approach to the runway is called (a) outer marker (b) localiser antena (c) Glide slope antena (d) Marker beacon |
| | (a) outer marker (b) localiser afficial to the basic length of runway |
| 14 | (a) Outer market (b) recently applied to the basic length of runway |
| | 1. Elevation correction 2. Gradient correction 3. Temperature correction (IES) |
| | The correct order of aplying the corrections to obtain running to the corrections to the corrections to the correction of the correction |
| | (a) $1 - 3 - 3$ (b) $1 - 3 - 2$ (c) $2 - 3 - 1$ (d) $3 - 1 - 2$ |

15. From list I and list II, select the correct answer from the codes:

| Lint I | List II |
|---|--|
| List I A. Localiser antena B. Glide slope antena C. PAR (Precision approach radar) | Gives controller a picture of the descending aircraft both in plan and elevation Indicates to the pilot whether he is to the left or right of the correct alignment indicates to the pilot the correct angle |
| D. ASR (Airport surveillance rada | 4 Provide the control tower operator with |

| 310 | | | * | | | | | Ol | bjective (| Civil Eng | ineering |
|-----|-------|-----------------------|---------|------------------|------------|-------------------|---------|------------|------------|-----------|--------------|
| | Code | es: | | | | | | | | | |
| | | A | В . | C | . D | | | | | | |
| | (a) | 3 | 2 | 1 | 4 | | | | | | |
| | (b) | 3 | 2 | 4 | 1 | | | | | | |
| | (c) | 2 | 3 | 1 | 4 | | | • | | | |
| | (d) | 2 | 3 | 4 | 1 | | | | | | 4:!!! |
| 16. | As p | er ICA(| O, the | mini | mum b | asic runw | | | | pe of al | rport wni |
| | (a) | 1500 m | and 6 | 500 m | L | | • • | 00 m and | | | · |
| | (c) | 1500 m a | and 75 | 0 m | | | (d) 2 | .100 m an | d 600 m | | |
| 17. | Mat | tch List | I and | List | II corr | ectly: | | | | | |
| | | List I | | | | List II | | | | | |
| | A. | Elevato | or | | 1. | It controls | yawii | ng | | | |
| | В. | Rudder | | | 2. | It controls | rollin | ng | | | |
| | C. | Ailero | ı | | 3. | It controls | pitch | ing | | | |
| | Co | des : | | | | | | | | | |
| | | Α | В | C | | | | | | | |
| | (a) | 1 | 2 | 3 | | | | | | | |
| | (b) | 2 | 3 | 1 | | | | | | | |
| | (c) | 2 | 1 | 3 | | | | | | | |
| | (d) | 3 | 1 | 2 | | | | | | | |
| 18 | . Mi | nimum | width | of cl | earway | is | | | | 0.50 | imne é |
| | (a) | -50 m | | (| b) 100 | m | ` ' | 150 m | | (d) 250 | |
| 19 |). W | ithin 4.5 structio | km (| distan s heig | ice from | m the run eeds | way e | end, an ol | bject shal | | nsidered an |
| | (a` | 20 m | | (| (b) 30 | m | ` . | 40 m | | (d) 50 | |
| 20 | 0. TI | , ie engin | e failu | ire ca | se for | determini | ng the | basic ru | nway leng | gth requ | ire |
| | (a | | clearw | | | | (b) | only stop | way | | |
| | - (0 | \ aithar | clean | พลบ ดเ | a stop | way | (d) | either a c | learway o | r a stopw | ay or both |
| 2 | 1. If | TOD = | Take | of di | stance | , LOD = l | ift off | distance, | , the cori | ect rela | tionship for |
| | | or Lomes | ndina | 0956 | is | | | | | | |
| | (a |) TOD | = 1.15 | LOI | (b) LC | D = 1.15 T | OD (c | e) TOD = | 1.25 LOD | (d) LOD | = 1.25 TOD |
| | . ` | • | | | | | | | | | |

(d) L + 120 m

| 35. | The | landing an | d take off | area of | heliports | are marked | with |
|-----|-----|--------------|-------------|-----------|------------|---------------|-----------------------------|
| | (a) | Letter H | (b) | Letter P | (c) | Number 3 | (d) None of these |
| 36. | The | pilot norm | ally takes | decision | about lan | ding when l | e is about m above |
| | the | runway. | •• | | | | |
| | (a) | 100 m | (b) | 60 m | (c) | 40 m | (d) 20 m |
| 37. | | ich of the f | ollowing is | s not ma | tched cor | rectly? | |
| | (a) | runway lig | | | (b) | Threshold l | ighting - yellow |
| | (c) | | | | (d) | End thresho | old lighting - red |
| 38. | The | width of t | hreshold r | narkings | is | | |
| | | 4 m | | 10 m | | 15 m | (d) 10 m |
| 30 | As | ner ICAO | recommen | dations a | ıll markin | gs on taxiwa | ays are painted |
| ٠,٠ | | white | | blue | (c) | black | (d) yellow |
| 40 | A o | nor ICAO | the maxin | num long | itudinal s | gradient aloi | ng the runway is limited to |
| 40. | | 1.5 % | | 3 % | (c) | 5 % | (d) 7 % |
| | | | | | | ~ ~ | |

: ANSWERS:

| . () | 2. (b) | 3. (b) | 4. (c) | 5. (d) |
|---------|---------|---------|---------|---------|
| 1. (a) | | 8. (d) | 9. (d) | 10. (a) |
| 6. (b) | 7. (d) | | | 15. (c) |
| 11. (b) | 12. (d) | 13. (b) | 14. (b) | |
| 16. (d) | 17. (d) | 18. (c) | 19. (b) | 20. (d) |
| | 22. (b) | 23. (c) | 24. (c) | 25. (c) |
| 21. (a) | | 28. (b) | 29. (a) | 30. (c) |
| 26. (b) | 27. (b) | • | | 35. (a) |
| 31. (d) | 32. (c) | 33. (a) | 34. (d) | |
| 36. (b) | 37. (b) | 38. (a) | 39. (d) | 40. (a) |



12.

Docks and Harbour

MCQ'S A platform built parallel to the shore for ship to come close to the shore is known as (d) port (c) lock (b) Wharf (a) Jetty A wharf is a platform having berth on one side only (a) built parallel to the shore (b) None of these (d) (a) and (b) both The 'breakwater' is provided (a) in artificial harbour (b) with arm (c) (a) and (b) both (d) None of these The platform constructed perpendicular to the shore having berth on both sides is called (d) lock (b) wharf (c) quay (a) Jetty The impact of ship while docking is taken up by (b) bresting dolphins (a) mooring dolphins fixed mooring berth (c) bulkhead The lock gates are always in (d) dozen (c) quadruple Single number (b) pair The tidal range is maximum in India at 7. (d) Bhavnagar Paradeep (b) Madras (a) Bombay Lowest tide of the month is known as 8. (d) diurnal tide (c) spring tide (b) tidal bore (a) neap tide Which is not a type of signal used in harbours? 9. (d) Moorings (c) Buoys (b) Semaphore (a) Beacon 10. The depth of sea at harbour used in handling of cargo should normally be (b) not less than 15 m (a) not more than 15 m (d) not less than 7.5 m not more than 7.5 m 11. The lowest tide which occurs in half lunar month is called (d) tidal bore (c) lunar tide (b) neap tide Spring tide 12. The phenomenon of movement and deposition of sand in a zig-zag way due to drifting of sand in the vicinity of coast is called

Objective Civil Eng. \ 2016 \ 40

314

| (a) Beac | h drift |
|----------|---------|
|----------|---------|

littoral drift (b) through action (c)

(d) Sedimentation

13. Why are moorings provided?

- (a) For anchoring of ships
- (b) For towing of ships to the sea
- For repair of ships
- (d) For washing of ships

14. Which one of the following is the best method for locating soundings to estimate the dredged material from the harbours?

- Two angles from shore
- (b) Two angles from boat
- One angle from shore and other from the boat (c)
- (d) Fixed intersecting ranges

15. Match List I and List II and select correct answer from the codes:

| \Box | List I | List II |
|--------|---------------------------------|---|
| B. | Break water Wharf Fender system | It protects the land from wave erosion Protects a seashore Lays vessels alongside, receives and discharges cargo and passengers |
| D. | Revetments | 4. Absorbs the energy of mooring vessel |

Codes:

| | Α | \mathbf{B} | С | D |
|-----|---|--------------|---|---|
| (a) | 2 | 3 | 1 | 4 |
| (b) | 2 | 3 | 4 | 1 |
| (c) | 3 | 2 | 1 | 4 |
| (d) | 3 | 2 | 4 | 1 |

16. The percentage of time in a year during which the cross wind component remains within the limit is

- (a) wind coverage (b) Head wind
- (c) Cross wind
- (d) Prevailing wind

17. What is use of station pointer?

- (a) For making soundings in water bodies
- (b) For making tidal observations
- (c) For plotting of soundings in harbour area
- (d) For marking sunken shipping hazards

18. The approximate height of wave for a given fetch (F) is given by

- (a) F
- (b) √F
- (c) $0.34 \sqrt{F}$
- (d) $0.25 \sqrt{F}$

19. In maritime works, speed of wind is expressed in

- (a) km/hr
- (b) knots
- (b) miles/hr
- (d) none of these

| 20. | 'Beaufort scale' is related to | | | | | | |
|-----|--|--|--|--|--|--|--|
| | (a) earthquake magnitude (b) Velocity of water waves | | | | | | |
| | (c) Wind speed (d) None of these | | | | | | |
| 21. | The mean low water/mean high water is the average of low/high water over a | | | | | | |
| | period of | | | | | | |
| | (a) 10 years (b) 15 years (c) 19 years (d) 25 years | | | | | | |
| 22. | Dolphins are used | | | | | | |
| | (a) to absorb the impact force of the ships | | | | | | |
| | (b) to provide mooring facility | | | | | | |
| | (c) to shorten the length of piers and wharves | | | | | | |
| | (d) all of the above | | | | | | |
| 23. | Horizontal or vertical wooden members or rubber strips fastended to the deck or | | | | | | |
| | face of the dock to absorb the impact of the ship are called (a) Dolphins (b) fenders (c) moles (d) trestle | | | | | | |
| | (a) Doiphins | | | | | | |
| 24. | Rockfills extending out from shores, used for roadways, railway track, side wall, | | | | | | |
| | etc. are called (a) Piers (b) breakwater (c) moles (d) trestle | | | | | | |
| 25 | the land and acquiring protection of the entrance | | | | | | |
| 25. | only is known as | | | | | | |
| | (a) Natural harbour (b) Semi-natural harbour | | | | | | |
| | (c) Artificial harbour (d) natural roadstead | | | | | | |
| 26 | Bilge blocks are used | | | | | | |
| 20. | (a) to afford sufficient bearing to the ship keel without being crushed | | | | | | |
| | (b) to provide level seat for ship and lateral stability | | | | | | |
| | (c) to keep the harbour water undisturbed | | | | | | |
| | (d) none of these | | | | | | |
| 27. | Dolphins are | | | | | | |
| | (a) cluster of closely spaced piles for mooring vessels | | | | | | |
| | (b) light pier designed to withstand vertical force | | | | | | |
| | (c) Constructed at the tip of breakwater near the harbour entrance | | | | | | |
| | (d) heavy pier designed to withstand water pressure | | | | | | |
| 28. | The location of a harbour may be guessed by | | | | | | |
| | (a) wave direction (b) wave height (c) fetch (d) all of the above | | | | | | |
| 29. | The floating navigational aid is | | | | | | |
| | (a) light ship (b) light house (c) beacon light (d) all of these | | | | | | |
| 30. | Which is not a mooring accessory? | | | | | | |
| | (a) bollard (b) capston (c) fender (d) cleat | | | | | | |

| 31. | The concrete armour units used in the construction of | | | | |
|---|---|--|--|--|--|
| | a) break water (b) pier (c) wharves (d) dry docks | | | | |
| 32. | The width of entrances of harbours is restricted to | | | | |
| | a) 100 m (b) 125 m (c) 150 m (d) 180 m | | | | |
| 33. | dock and unloading corgo and | | | | |
| | a) is a marine structure for berthing of vessels for loading and unloading cargo and passengers | | | | |
| | b) is provided with a dock gate | | | | |
| | c) is provided with an arrangement to pump out water when required | | | | |
| | (d) all the above | | | | |
| 34. | The dock wall is designed as a gravity retaining wall and is tested for | | | | |
| | (a) backfill pressure when the dock is empty | | | | |
| (b) Maximum water pressure from the dock without backfill | | | | | |
| | (c) The load transmitted to the dock by the movement of loaded vehicles on the way | | | | |
| | (d) all of the above | | | | |
| 35. | Which one of the following lines is used for tying a ship with a dock? | | | | |
| | (a) bow line (b) stern line (c) spring line (d) all the above | | | | |
| 36. | A low wall built into the sea more or less perpendicular to the coast line, to resist | | | | |
| | travel of sand and shingle along a beach is called | | | | |
| | (a) break water (b) break wall (c) groynes (d) shore wall | | | | |
| | : ANSWERS : | | | | |
| | (b) | | | | |

| 1. (b) 6. (b) 11. (a) 16. (a) | 2. (c) 7. (d) 12. (c) 17. (b) | 3. (c) 8. (a) 13. (a) 18. (c) | 4. (a) 9. (b) 14. (c) 19. (b) 24. (c) | 5. (b) 10. (b) 15. (b) 20. (b) 25. (b) | |
|--|--|--|---------------------------------------|--|--|
| 21. (c) 26. (b) 31. (a) 36. (c) | 22. (d) 27. (a) 32. (d) | 23. (b) 28. (d) 33. (d) | 29. (a) 34. (d) | 30. (c) 35. (d) | |



Estimating and Costing

Useful Information

Length, Area, Volume

$$1m = 3.28$$
 ft.
 $1m^2 = 3.28 \times 3.28 = 10.75$ Sq. ft.
 $1m^3 = 3.28 \times 3.28 \times 3.28 = 35.28$ cu.ft

Area 1 brass = 100 Sq. ft.
=
$$\frac{100}{10.75} = 9.3 \text{ m}^2$$

Volume 1 brass = 100 cu. ft.
= $\frac{100}{35.28} = 2.83 \text{ m}^3$
1 bag of cement = 50 kg mass
= 0.035 m³ volume
= 0.035 × 35.28
= 1.23 cu. ft. volume

1 Gallon = 4.546 Litre
1 Pound = 0.4536 kg
1 Tonne = 1000 kg
1 Quintal = 100 kg

| 1 Var | = | 1 Yard |
|----------|---|-----------------------------|
| | = | 3 ft. = 90 cm = 0.9 cm |
| 1 Guntha | = | 33 ft. × 33 ft |
| | = | 1089 Sq. ft = 101.30 Sq.m |
| 1 Vigha | = | 16 Guntha |
| 1 Acre | = | 2.5 Vigha |
| | = | 2.5 × 16 |
| | = | 40 Guntha |
| 1 Acre | = | 4047 Sq. m |

318

| 100 Sq.m | = | 1 Are |
|-----------|-----|-------------|
| 100 Are | = | 1 Hectare |
| | = | 10000 Sq. m |
| 1 Hectare | _ = | 2.471 Acre |

Density of Materials

| Material | Density |
|---------------|--|
| Cement | 1430 kg/m ³ |
| Sand | 1500 kg/m3 |
| Aggregates | 1600 kg/m3 |
| Steel | 7850 kg/m ³ |
| Water | $1000 \text{ kg/m}^3 (10 \text{ kN/m}^3)$ |
| Soil | $1600 - 1800 \text{ kg/m}^3$ |
| RCC | 2500 kg/m ³ (25 kN/m ³) |
| PCC | 2400 kg/m ³ |
| Brick Masonry | 1800 kg/m ³ |
| Wood | 800 to 1600 kg/m ³ |

Nominal Mixes for Concrete:

| Grade of Concrete | Cement : Sand : Aggregate |
|-------------------|---------------------------|
| M 7.5 | 1:4:8 |
| M 10 | 1:3:6 |
| M 15 | 1:2:4 |
| M 20 | 1:1.5:3 |
| | (minimum grade for RCC) |
| M 25 | 1:1:2 |

SHORT ANSWER QUESTIONS

1. What is weight and volume of 1 bag of cement?

weight = 50 kg

volume = 0.035 m^3 (1.23 cu.ft) or 35 litres.

2. How many bricks required for 1 m³ brickwork? 500 nos.

3. What is actual and nominal size of bricks?

Actual size of brick = $19 \times 9 \times 9$ cm

Nominal size of brick = $20 \times 10 \times 10$ cm (with mortar joints)

Estimating and Costing

4. What is volume of bricks and mortar in 1 m³ of brickwork?

In 1 m³ brickwork 500 bricks are required

.. Volume of bricks =
$$500 \times (0.19 \times 0.09 \times 0.09)$$

= 0.77 m³

∴ Volume of mortar =
$$1 - 0.77$$

= $0.23 \text{ m}^3 \dots$ wet volume
= $0.33 \text{ m}^3 \dots$ dry volume

5. Give quantities of cement, sand and aggregate required for 1 m³ of M 20 concrete.

Volume of wet concrete 1m3

Volume of dry concrete = 1.52 m³ (52 % more)

$$1:1.5:3=5.5$$

$$\therefore \text{ cement} = \frac{1}{5.5} \times 1.52 = 0.2763 \text{ m}^3 = \frac{0.2763}{0.035} = 7.89 \text{ say 8 bags}$$

$$\text{Sand} = \frac{1.5}{5.5} \times 1.52 = 0.414 \text{ m}^3$$

Aggregate =
$$\frac{3}{5.5}$$
 × 1.52 = 0.829 m³

6. Give current market rates for following materials

7. Give current market rates for following items.

Excavation up to 1.5 m = 85 Rs./m^3

B.B.C.C.
$$(1:4:8) = 2700 \text{ Rs./m}^3$$

P.C.C.
$$(1:4:8) = 3000 \text{ Rs./m}^3$$

R.C.C.
$$(1:1.5:3) = 10,000 \text{ Rs./m}^3$$

Brickwork in foundation
$$(1:6) = 3200 \text{ Rs./m}^3$$

Brickwork in superstructure
$$(1:6) = 3500 \text{ Rs./m}^3$$

12 mm thick cement plaster (1 : 4) =
$$150 \text{ Rs./m}^2$$

8. Give task work for labour and mason.

Labour
$$\longrightarrow$$
 excavation in ordinary soil = 3 m³

Delivary of concrete = 3 m³

320

Mason
$$\longrightarrow$$
 Brickwork in foundation = 1.25 m³

Brickwork in superstructure = 1.0 m³

12 mm thick Plaster (1 : 3) = 10 m²

P.C.C. (1 : 4 : 8) = 5 m³

R.C.C. (1 : 1.5 : 3) = 3 m³

9. For earthwork, what is standard lead and lift?

Lead = 30 m

Lift = 1.5 m

10. What is surface excavation?

Excavation exceeding 1.5 m in width as well as 10 sq.m on plan with a depth not axceeding 30 cm shall be described as surface excavation.

It is measured in m².

11. If it is not possible to take the measurements from borrow pit or cutting, excavation shall be worked out from filling. How deductions are made for voids?

10% deduction for normally consolidated fills.

5% deduction for consolidation by heavy machinery.

32. Give deduction rules for brickwork as per IS: 1200

No deduction shall be made for:

- Opening up to 0.1 Sq.m area
- Pipes up to 30 cm diameter
- ends of beams, lintels, joists, etc.

13. Give deduction rules for concrete as per IS: 1200.

No deduction shall be made for:

- Opening up to 0.1 Sq.m area
- Volume of steel reinforcement
- Volume of pipes not exceeding 100 Sq.cm in cross sectional area.
- Ends of beams, joists, girders, etc. up to 500 sq.cm in cross sectional area.

14. Give deduction rules for plastering as per IS: 1200.

No duduction shall be made for:

- Opening up to 0.5 m² area
- ends of beams, joists, steps not exceeding 0.5 m² in area.
- For openings exceeding 0.5 m² and less than 3 m² deduction shall be made for one face only if both faces are plastered with the same plaster.

15. In centre-line method how net centre line length is calculated?

Net centre line length

= total centre line length – $(\frac{1}{2} \times \text{width of wall} \times \text{number of junctions}).$

- At the corner of a building, no junction is formed.
- At T-joint there will be one junction.
- At cross walls, there will be two junctions.
- 16. Give formula to calculate weight of 1m long steel bar.

Weight of reinforcement bar for 1 m length

$$= \frac{d^2}{162} \text{ kg}.$$

where d = diameter of bar in mm.

For example,

If
$$d = 16 \text{ mm}$$

wt. of bar per m =
$$\frac{d^2}{162} = \frac{16^2}{162} = 1.58 \text{ kg/m}$$

17. How number of bars can be calculated if distance and spacing of bars is given.

No. of bars =
$$\frac{(L-2 \times \text{end cov er})}{c/c \text{ spacing of bars}} + 1$$

where,

L = distance within which bars are to be provided.

18. What is length of hook provided at the end of bar?

hook length = $9 \times \text{diameter of bar}$.

19. What is minimum hook length for stirrups?

Minimum hook length = $12 \times dia$. of bar

or

75 mm which ever is more.

For 2-hooks,

hook length = 24ϕ or 0.15 m

20. For 45° bent up bar, extra length of bar is taken as,

extra length = 0.45x

where, x = vertical distance between centres of top and bottom portion of bent up bar.

21. What is size of standard gauge box (Farma) for mixing sand and aggregate?

Vol. of 1 bag of cement = 35 litres.

Size of standard gauge box

$$= 33.3 \text{ cm} \times 30 \text{ cm} \times 35 \text{ cm} = 0.035 \text{ m}^3$$

= 35 litres

22. For 1:1.5:3 mix proportion, if bulking of sand is 20%, actual volume of moist sand required is ?

For 1:1.5:3

Objective Civil Eng. \ 2016 \ 41

35 lit: 52.5 lit: 105 lit

Volume of moist sand required

 $= 52.5 + 0.20 \times 52.5 = 63$ litres.

23. What is EMD?

The purpose of Earnest money Deposit (EMD) is to punish the contractor who refuse to take up the work or to prevent the financially weak contractor from entering into the contract.

The EMD amount is about 1 to 2% of the estimated cost of the work.

The EMD of the tenderer whose tender has not been accepted is refunded. The EMD of the successful contractor is converted into security deposit.

24. What is security deposit?

The security amout is required to be deposited by a successful contractor whose tender has been accepted.

The SD amount is about 10% of the tendered amount.

In case of delay in work or incomplete work or poor quality work, the SD is forfeited fully or partially.

After satisfactory completion of work, usually after one rainy season, SD is refunded to the contractor.

25. What is technical sanction?

Technical sanction means the sanction of the detailed estimate, design calculations, quantities of works, rates and cost of the work by the competent authority of the engineering department (PWD).

Powers for technical Sanction

| | P.W.D. officer | Power for technical sanction |
|----|-------------------------|------------------------------|
| 1. | Chief engineer | Full powe |
| 2. | Superintending engineer | up to Rs.15 lakhs |
| 3. | Executive engineer | up to Rs.5 lakhs |

26. What is liquidated damages?

Liquidated damages is an amount of compensation payable by a contractor to the owner or Government due to delayed construction. It has no relation with the real damage.

27. What is arbitration?

The process by which the parties under a contract get their disputes and differences settled through the intervention of an impartial person or a committee of experts in a judicial manner is known as arbitration.

The impartial person chosen by the parties themselves to whom the disputes and differences are referred to, is called an arbitrator.

Estimating and Costing

- 28. List methods of calculating depreciation.
 - (i) Straight line method $D = \frac{C-S}{n}$
 - (ii) Constant percentage method, $p = 1 \left(\frac{S}{C}\right)^{\frac{1}{n}}$ where p = % rate of annual depriciation
 - (iii) Sinking fund method

$$p = \frac{i}{(1+i)^n - 1}, \qquad q = \frac{(1+i)^n - 1}{i}$$

Rate of depreciation in 'n' years = $p \times q$ %

29. If, i is the rate of interest, annual sinking fund installment (p) to accumulate 1 Rs. in n years is?

$$p = \frac{i}{\left(1 + i\right)^{n} - 1}$$

30. If i is the rate of interest, and 1 Rs. is deposited every year, total sinking fund accumulated at the end of n years will be?

$$q = \frac{\left(1+i\right)^n - 1}{i}$$



Meq's

| 1. | The volume of 1 bag of cement is | | | | | |
|-----|---|---------|------------------------------------|--------|-------------------------------|--|
| | (a) 0.025 m^3 (b) 0.035 m^3 | (c) | 0.045 m^3 | (d) | 0.055 m^3 | |
| 2. | The actual size of standard brick is | | | | | |
| | (a) $19 \times 9 \times 9$ cm | (b) | $20 \times 10 \times 10$ | em | | |
| | (c) $20 \times 12 \times 8$ cm | (d) | $20 \times 10 \times 8$ ci | n | | |
| 3. | The nominal size of standard brick is | | | | | |
| | (a) $19 \times 9 \times 9$ cm | (b) | | | | |
| | (c) $20 \times 12 \times 8 \text{ cm}$ | (d) | $20 \times 10 \times 8 \text{ cm}$ | | . 3 0 | |
| 4. | Number of bricks of size 20 cm × 10 wall is | em × | 10 cm require | d for | c 1 m ³ of masonry | |
| | (a) 300 (b) 400 | (c) | 450 | (d) | 500 | |
| ź. | In analysis of rates, contractor's profit i | s take | en as | | | |
| | (a) 5 % (b) 10 % | | 15 <mark>%</mark> | (d) | 20 % | |
| 6. | In analysis of rates, water charges are | adde | d on those iter | ns w | hich require water, | |
| | these charges are added at the rate of | | | | | |
| | (a) 1 % (b) 1.5 % | (c) | 2 % | (d) | 2.5 % | |
| 7. | In measuring plastering, no deduction is | s mad | e for opening ı | ip to | | |
| | (a) 0.25 m^2 (b) 0.50 m^2 | (c) | 1.0 m ² | (d) | 1.5 m ² | |
| 8. | The ratio of cost of labour to the total | cost of | f the building i | | | |
| | (a) 1:10 (b) 1:1 | , , | 1:4 | | 1:2 | |
| 9. | In preparing cement concrete by volume | e the s | size of gauge bo | ox (fa | rma) for measuring | |
| | sand and aggregate is | | | | | |
| | (a) $35 \times 30 \times 40 \text{ cm}$ | ` ′ | 33.3 × 30 × 35 cm | | | |
| | (c) 30 × 30 × 30 cm | , , | 35 × 25 × 30 | | | |
| 10. | - | tion, l | ift and lead sp | ecifie | d are, respectively | |
| | (a) 1.0 m, 20 m (b) 1.5 m, 20 m | (c) | 1.5 m, 30 m | (d) | 1.0 m, 25 m | |
| 11. | Which estimate is least accurate? | | | | · | |
| | (a) Detailed estimate | ` ' | Plinth area es | | | |
| | (c) Supplementary estimate | (d) | Revised estim | ate | | |
| 12. | Thickness of plastering is usually | | | | 10 | |
| | (a) 6 mm (b) 12 mm | (c) | 25 mm | (d) | 40 mm | |
| 13. | 1 m ³ of mild steel weighs about | | , | 4.15 | 105501 | |
| | (a) 3800 kg (b) 1000 kg | (c) | | | 12560 kg | |
| 14. | For 1 m ³ of M 20 (1:1.5:3) concrete | | | | | |
| | (a) 6.4 bags (b) 8 bags | (c) | 10 bags | '(d) | 12 bags | |
| | | | | | | |

| 15. | The weight of 16 | mm dia. steel bar pe | r m length is | | |
|-----|------------------------|------------------------------------|------------------------|----------|-----------------------|
| | (a) 0.89 kg | (b) 1.58 kg | (c) 2.0 kg | (d) | 2.46 kg |
| 16. | In analysis of rate, | , for 1 m ³ of brick wo | rk the quantity of dry | | |
| | (a) 0.20 m^3 | (b) 0.25 m^3 | (c) 0.30 m^3 | (d) | 0.35 m ³ |
| 17. | In a detailed estin | nate the provision for | r contingencies is us | ually | |
| | (a0 1 % | (b) 3 to 5 % | (c) 10 % | (d) | 15 % |
| 18. | Number of bricks | s usually carried by a | truck is | | |
| | (a) 2000 | · / | (c) 5000 | ` ' | 7500 |
| 19. | The expected task | kwork for earthwork | in equation in ordin | nary so | oil per mazdoor per |
| | day is | | | | |
| | | (b) 2.0 cu.m | | | |
| 20. | The expected task | kwork (turnout) of br | ick work in C:M in | founda | ation and plinth per |
| | mason per day is | | | | |
| | (a) 1.25 m^3 | (b) 1.50 m ³ | (c) 2.0 m^3 | (d) | 2.5 m ³ |
| 21. | The expected task | work of RCC (1:1. | 5:3) per mason pe | r day i | s |
| | | (b) 2.5 m^3 | | | 4 m ³ |
| 22. | - | kwork of 12 mm c.m. | | | |
| | (a) 6 m^2 | (b) 8 m ² | (c) 10 m^2 | (d) | 15 m ² |
| 23. | Length of hook p | provided at the end of | | | |
| | (a) 7.5 × dia | (b) 9 × dia | (c) 10 × dia | (d) | 12 × dia |
| 24. | Length of hook fo | | | | |
| | | (b) 9 × dia . | | (d) | 12 × dia |
| 25. | Volume of sand of | carried in a truck is a | approximately | | |
| | (a) 4 cu.m | (b) 6 cu.m | (c) 8 cu.m | (d) | 10 cu.m |
| 26. | The unit of meas | urement of brick par | tition wall is | | |
| | (a) per m length | \ / 1 | (c) cu.m | (d) | |
| 27. | While doing eart | hwork in filling in fo | oundation trenches t | the thic | ckness of each layer |
| | should not exceed | | | | |
| | | (b) 20 cm | | (d) | |
| 28. | As per IS: 1200, | while doing measure | ements, the dimensio | n shall | be measured to the |
| | nearest | | | | |
| | (a) 0.01 m | (b) 0.01 mm | · | | 0.1 m |
| 29. | Number of bricks | s required for constru | icting a partition wa | ill 10 m | long, 1 m high and |
| | 10 cm thick is | | | | |
| | (a) 200 | (b) 300 | (c) 500 | | 600 |
| 30. | The salvage valu | e of a building is us | ually taken as | of t | he total cost of con- |
| | struction. | | | | |

| 326 | · | Objective Civil Engineering |
|-----|--|---|
| | (a) 5 % (b) 10 % | (c) 15 % (d) 20 % |
| 31. | The EMD amount to be deposite | ed with the tender is about of the |
| | estimated cost of the work. | |
| | (a) 2 to 3 % (b) 1 to 2 % | (c) 5 % (d) 10 % |
| 32. | The security deposit amount to be de | eposited by a successful contractor whose tender |
| | has been accepted is of the te | ndered amount. |
| | (a) 2 to 3 % (b) 5 % | (c) 10 % (d) 15 % |
| 33. | The amount of compensation to be | paid by the contractor to the owner due to de- |
| | layed work is called | |
| | (a) Penalty (b) Compensation | n (c) liquidated damages (d) Fine |
| 34. | The process by which the parties und | der a contract get their disputes settled through |
| | the impartial person is called | |
| | (a) discussion (b) arbitration | (c) court case (d) technical sanction |
| 35. | The value of a property building afte | r its working tenure without being dismentled is |
| | known as | |
| | (a) Scrap value (b) book value | (c) Salvage value (d) market value |
| 36. | The unit of measurement for steel w | orks in trusses and its parts is in |
| | (a) quintal (b) cm | (c) numbers (d) kg |
| 37. | The unit of payment for fixing glass | panels is in |
| | (a) sq.m (b) number | (c) cu.m (d) kg |
| 38. | Thickness of 25 gauge sheet is | |
| | (a) less than 1 mm (b) | 1 mm |
| | (c) between 1 mm and 1.5 mm | (d) 2 mm |
| 39. | Damp proof course (D.P.C.) is measu | red in |
| | (a) running meter (b) sq.m | (c) cu.m (d) None of these |
| 40. | While mixing cement mortar by volu | me, the volume of a cement bag is specified as |
| | (a) 50 litres (b) 35 litres | (c) 0.050 m^3 (d) 0.35 m^3 |
| 41. | As a thumb rule, the percentage of s | teel in RCC column is taken as |
| | (a) 1 to 5 % (b) 5 to 10 % | (c) 10 to 15 % (d) 15 to 20 % |
| 42. | Generally for analysis of rates, the re | duction in volume of wet mixed mortar over the |
| | sum total volume of ingredients is ta | ken as |
| | (a) 5 % (b) 10 % | (c) 25 % (d) 50 % |
| 43. | The useful part of liveable area of a | building is called |
| | (a) carpet area (b) circulation ar | ea (c) plinth area (d) built up area |
| 44. | The expected taskwork of half brick | partition wall per mason per day is |
| | (a) 1.5 m^3 (b) 1.25 m^3 | (c) 3 m^2 (d) 5 m^2 |
| | | |

| 45. | Ori | Original cost of property minus depriciation is | | | | | | | |
|-----|-----|---|--------|---|---------|--------------------|--------|---------------------|--|
| | (a) | book value | (b) | Salvage value | (c) | rateable value | (d) | market value | |
| 46. | The | total estimated | d cost | t of a building el | ectrifi | ication usually | accou | ints for | |
| | (a) | 1 % | (b) | 2 % | (c) | 8 % | (d) | 12 % | |
| 47. | The | standard widt | h of | asbestos cement | corru | gated sheet is | | | |
| | (a) | 0.9 m | (b) | 1.0 m | (c) | 1.05 m | (d) | 1.25 m | |
| 48. | The | number of con | ruga | tions per sheet in | n case | of asbestos ce | ment | corrugated sheet is | |
| | (a) | 3 | (b) | 5 | (c) | 7 | (d) | 10 | |
| 49. | For | 100 sq.m of ro | of su | rface, the area o | f A.C | . sheets require | ed wi | ll be | |
| | (a) | 80 m ² | (b) | 100 m ² | (c) | 115 m ² | (d) | 128 m ² | |
| 50. | | | | y that can be ob is put for sale, is | | | ular t | ime from the open | |
| | (a) | book value | (b) | market value | (c) | current value | (d) | obsolence value | |
| 51. | | tiplying facto | | | | | | window for each | |
| | (a) | 1.2 | (b) | 1.0 | (c) | 0.5 | (d) | 0.25 | |
| 52. | Mul | tiplying factor | for p | ainting of flush | door f | for each side is | | | |
| | (a) | 1.8 | (b) | 1.2 | (c) | 1.0 | (d) | 0.8 | |
| | | | | | | | | | |

: ANSWERS:

| | 1. (b) | 2. (a) | 3. (b) | 4. (d) | 5. (b) |
|---|---------|---------|---------|---------|---------|
| | 6. (b) | 7. (b) | 8. (c) | 9. (b) | 10. (c) |
| | 11. (b) | 12. (b) | 13. (c) | 14. (b) | 15. (b) |
| | 16. (c) | 17. (b) | 18. (b) | 19. (c) | 20. (a) |
| | 21. (c) | 22. (c) | 23. (b) | 24. (d) | 25. (a) |
| | 26. (b) | 27. (b) | 28. (a) | 29. (c) | 30. (b) |
| | 31. (b) | 32. (c) | 33. (c) | 34. (b) | 35. (c) |
| | 36. (a) | 37. (b) | 38. (a) | 39. (b) | 40. (b) |
| | 41. (a) | 42. (c) | 43. (a) | 44. (d) | 45. (a) |
| ı | 46. (c) | 47. (c) | 48. (c) | 49. (c) | 50. (b) |
| | 51. (c) | 52. (b) | | | |
| İ | | | | 1 | |

Irrigation Engineering

| | | ع پار | | | | | | |
|----|---|---------|---|--|--|--|--|--|
| 1. | The relation between duty (D) in he | ectares | /cumec, delta (Δ) in metres and base | | | | | |
| | period (B) in days is | | | | | | | |
| | (a) $D = 8.64 \frac{B}{\Delta}$ (b) $D = 86.4 \frac{B}{\Delta}$ | (c) | $D = 864 \frac{B}{\Delta} \qquad (d) D = 8640 \frac{B}{\Delta}$ | | | | | |
| 2. | The total depth of water required by | a cro | p during entire base period of crop is | | | | | |
| | known as | | , | | | | | |
| | (a) Duty (b) Delta | (c) | base period (d) Crop period | | | | | |
| 3. | The duty is largest | | | | | | | |
| | (a) at the head of main canal | (b) | at the head of water course | | | | | |
| | (c) on the field | (d) | at all place | | | | | |
| 4. | The irrigating capacity of a unit of v | vater i | s called | | | | | |
| | (a) duty (b) delta | (c) | cumec-day (d) capacity factor | | | | | |
| 5. | The useful soil moisture for plant gr | owth i | s | | | | | |
| | (a) gravitational water | (b) | Capillary water | | | | | |
| | (c) hygroscopic water | (d) | all of these | | | | | |
| 6. | 7 | _ | the pore spaces in soil particles by | | | | | |
| | replacing all air held in pore spaces | is kno | wn as | | | | | |
| | (a) field capacity | (b) | available moisture | | | | | |
| | (c) saturation capacity | (d) | Permanent wilting coefficient | | | | | |
| 7. | The moisture content of the soil, af | ter fr | ee drainage has removed most of the | | | | | |
| | gravity water is known as, | | | | | | | |
| | (a) Field capacity | (b) | Saturation capacity | | | | | |
| | (c) Wilting coefficient | (d) | Available moisture | | | | | |
| 8. | - | n no-le | onger extract sufficient water from the | | | | | |
| | soil for its growth is called | | • | | | | | |
| | (a) Field capacity | (b) | Permanent wilting point | | | | | |
| | (c) Saturation capacity | (d) | Available moisture | | | | | |
| 9. | Available moisture may be defined a | S | | | | | | |
| | (a) Moisture content at permanent wilting point | | | | | | | |

Crop ratio is defined as

area irrigated during rabi season area irrigated during kharif season area irrigated during kharif season

(a) Total area of land total area under crop

(b) area irrigated during kharif season area irrigated during rabi season

(d) None of these

18. Total quantity of water flowing for one day at the rate of 1 cumec is known as

(a) time factor (b) outlet factor (c) capacity factor (d) cumec-day

19. The time in days, that a crop takes from the instant of its sowing to that of its harvesting is known as

(a) Crop period (b) base period (c) time factor (d) none of these

Objective Civil Eng. \ 2016 \ 42

| 20. | The | period between | the | first wateri | ng and | the last wateri | ng supplie | d to the land |
|-------------|-------|---------------------------------|--------------|--------------------------|------------------|------------------|--------------|----------------------|
| | is ca | | | | | | | |
| | | Crop period | | | (c) | time factor | (d) N | lone of these |
| 21. | Grav | vitational water | is al | lso called | | | | |
| | (a) | Capillary water | | | (b) | • | ater | |
| | . , | hygroscopic war | | | (d) | all of these | | |
| 22. | The | water requirem | ient i | in terms of | | | | |
| | (a) | | | tobacco | | potatoes | (d) s | ugarcane |
| 23. | The | average delta o | f ric | | | | | |
| | (a) | 30 cm | | | , , | 120 cm | • • | 50 cm |
| 24. | | uty (D) is 1428 h | | | | period (B) is 12 | 20 days, for | r an irrigated |
| | crop | , then delta (Δ) | in r | | | | | (GATE) |
| | (a) | 102.8 | (b) | 0.73 | (c) | 1.38 | (d) (| 10.0 |
| | | | _] | B 8.64×12 | 20 _ 0.7 | 26 m | | |
| | - | Hint: $\Delta = 8$ | .64 j | $D = \frac{1428}{1}$ | — = 0.7 <i>.</i> | 20 iu | | |
| 25. | A SI | orinkler irrigati | on sy | ystem is sui | table wl | ien | | (GATE) |
| 20. | (a) | land gradient is | s stee | p and the so | oil is eas | sily erodible | | |
| | (b) | Soil is having | | | | | | |
| | (c) | water table is I | | | | | | |
| | (d) | Crops to be gro | | have deep ro | oots | | | |
| 26. | On | which of the | canal | l systems, l | R.G. Ke | ennedy executi | ve engine | er in Punjab |
| | irri | gation departme | ent n | nade <mark>his</mark> ob | servatio | ns for proposi | ng his the | ory on stable (GATE) |
| | | nnels ? Krishna Weste | Da | alta agnale | (6) | Lower Bari De | oab canals | , , |
| | (a) | Lower Chenab | | | (d) | < | | |
| 4 4 | ` ' | | | | (0) | Орриг 2 шт 2 | | |
| 27. | | ter logging may | | | | (b) Inadequat | e drainage | |
| | (a) | Over irrigation Seepage from | | ning recervo | irs etc | (d) all of the | | |
| • | (c) | io of the total | | | | • • | | which it has |
| 28. | | n spread is call | | ille of water | delive | to it orop | | |
| | | _ | | delta | (c) | critical depth | (d) | None of these |
| 20 | (a) | duty e operation, whi | | | | | • • | |
| 29. | | | | leaching | | Separation | (d) | none of these |
| 20 | (a) | washing the diversion o | | | ` ' | - | , , | |
| <i>5</i> 0. | | ridge canal | 71 IU (4) | nerennial | canal (c) | inundation can | al (d) | canal |
| | (a) | Huge canai | (0) | , porominar (| - unui (0) | | () | |

| 31. | The calle | | el be | tween the top | of t | he bank and supply | | |
|-----|-----------|----------------------------|---------|-----------------|--------|--------------------------------|-------|---------------------------------------|
| | (a) | berm | (b) | free board | (c) | height of bank | (d) | none of these |
| 32. | Borr | ow pits should | prefe | rably be locat | ed in | 1 | | |
| | (a) | field on the left | | | - | | | • |
| | (b) | field on the righ | nt side | e of the canal | | | | |
| | (c) | fields on both s | | | | | | |
| | (d) | central half wid | | | | | | |
| 33. | Disp | osal of extra ex | cavat | ed earth of ca | anals | is utilised to provi | | |
| | (a) | left side | (b) | right side | (c) | both sides | (d) | all the above |
| 34. | A w | atershed canal | | | | | | _ |
| | (a) | is most suitable | in hi | lly areas | (b) | | ainag | ge work |
| | (c) | irrigates only or | n one | side | (d) | all of these | | |
| 35. | A co | entour canal | | | | | | |
| | (a) | irrigates on both | | | | | | |
| | (b) | is most suitable | | | | | | |
| | (c) | is generally alig | gned p | parallel to the | conto | our of the area | | |
| | (d) | all of these | | | | | | |
| 36. | The | most desirable | align | ment of an ir. | rigat | ion canal is along | | 0.4 |
| | (a) | | | | | the valley line | | none of these |
| 37. | A c | | | | | the contour is call | | 0.1 |
| | (a) | | | | | side slope canal | (d) | none of these |
| 38. | The | most serious ty | | | om a | | 200 | 6.1 |
| | (a) | evaporation | (b) | Seepage | (c) | absorption | (d) | none of these |
| 39. | Ber | ms are provided | | | | | | |
| | (a) | Strengthen the | | | 1 1 | Check the seepage | | |
| | (c) | remove silt from | m the | canal | (d) | both (a) and (b) | | |
| 40. | A li | ned canal is in | | | | | (1) | |
| | (a) | initial regime | (b) | permanent reg | gime | (c) final regime | (d) | none of these |
| 41. | | | | y, assumed the | at the | e silt is kept in susp | ensic | on due to eddies |
| | gen | erated from the | | | | | 15 | · · · · · · · · · · · · · · · · · · · |
| | (a) | bed only | ` ' | sides only | • • | whole perimeter (| ı) an | y one of these |
| 42. | | e relation given | | ennedy for cr | itical | velocity is | | |
| | | $V = 0.55 \text{ m } D^0$ | | | (b) | $V = 0.64 \text{ mD}^{0.55}$ | | |
| | (c) | $V = 0.74 \text{ mD}^{0.}$ | .84 | | (d) | $V = 0.84 \text{ m } D^{0.74}$ | | |

| 43. | If m _r is the mean particle diameter of the silt in mm, the Lacey's silt factor (f) is | | | | | | |
|-----|---|--|--|--|--|--|--|
| | given by | | | | | | |
| | (a) $f = 1.76 \text{ m}_r^{3/2}$ (b) $f = 1.76 \text{ m}_r^{1/2}$ (c) $f = 1.76 \text{ m}_r^2$ (d) $f = 1.76 \text{ m}_r^{5/2}$ | | | | | | |
| 44. | The perimeter - discharge (P - Q) relationship is given by | | | | | | |
| | (a) $P = 2.25 \sqrt{Q}$ (b) $P = 4.75 \sqrt{Q}$ (c) $P = 3.75 \sqrt{Q}$ (d) $P = 1.75 \sqrt{Q}$ | | | | | | |
| 45. | Distributary head regulator is provided | | | | | | |
| | (a) to control the supplies to the off-taking canal | | | | | | |
| | (b) to control the silt entry in the off-taking canal | | | | | | |
| | (c) both (a) and (b) | | | | | | |
| | (d) none of these | | | | | | |
| 46. | A structure constructed in an irrigation canal for the purpose of wasting some of | | | | | | |
| | its water is known as | | | | | | |
| | (a) fall (b) escape (c) regulator (d) none of these | | | | | | |
| 47. | are known as safety valves for canals. | | | | | | |
| | (a) escapes (b) regulators (c) falls (d) outlets | | | | | | |
| 48. | The Sarda canal has | | | | | | |
| | (a) glacis type fall (b) vertical drop fall (c) ogee fall (d) rapid fall | | | | | | |
| 49. | A parabolic glacis type fall is commonly known as | | | | | | |
| | (a) Inglis fall (b) Sarda fall (c) Montague fall (d) Vertical type fall | | | | | | |
| 50. | Average normal size of a rain drop may be of the order of | | | | | | |
| | (a) 0.5 - 4 mm (b) 5 - 10 mm (c) 10 - 50 mm (d) None of these | | | | | | |
| 51. | | | | | | | |
| | (a) main canal (b) branch canal (c) Water course (d) distributary | | | | | | |
| 52. | As per Lacey's theory, the silt factor is | | | | | | |
| | (a) Directly proportional to average particle size | | | | | | |
| | (b) directly proportional to square root of average particle size | | | | | | |
| | (c) inversely proportional to average particle size | | | | | | |
| | (d) not related to average particle size | | | | | | |
| 53. | Garret's diagrams are based on | | | | | | |
| | (a) Lacey's theory (b) Kennedy's theory (c) Khosla's theory (d) Bligh's theory | | | | | | |
| 54. | According to Bligh's Creep theory the percolation water creeps | | | | | | |
| | (a) in a straight line under the floor | | | | | | |
| | (b) along the contact of the base profile of the apron with the subsoil | | | | | | |
| ٠ | (c) in a straight path under the foundation | | | | | | |
| | (d) none of the above | | | | | | |
| | | | | | | | |

| 31. | | | vel between the top | of t | he bank and supply | lev | el in a canal is |
|-----|-------|-------------------------------|------------------------------------|--------|--------------------------------|-------|------------------|
| | calle | | a> 0 1 1 | | 1 . 14 . 6 1 1. | (4) | none of these |
| | • • | berm | (b) free board | | | (u) | none of these |
| 32. | Bor | _ | preferably be locat | ted in | 1 | | |
| | (a) | | side of the canal | • | | | |
| | (b) | - | nt side of the canal | | | | |
| | (c) | | ides of the canal | _ | | | |
| | (d) | | th of the section of | | | | |
| 33. | Dist | oosal of extra ex | | | is utilised to provi | | |
| | (a) | | (b) right side | (c) | both sides | (d) | all the above |
| 34. | A w | atershed canal | | | | | • |
| | (a) | is most suitable | · | (b) | | aınaş | ge work |
| | (c) | irrigates only o | n one side | (d) | all of these | | |
| 35. | A c | ontour canal | | | | | |
| | (a) | irrigates on bot | | | | | |
| | (b) | is most suitable | | | | | |
| | (c) | is generally alig | gned parallel to the | conto | our of the area | | |
| | (d) | all of these | | | | | |
| 36. | The | | alignment of an ir | | | | 0.4 |
| | (a) | | (b) the ridge line | | | | none of these |
| 37. | A c | anal <mark>which is</mark> al | igned at right a <mark>ng</mark> l | es to | the contour is call | ed | |
| | (a) | | | | side slope canal | (d) | none of these |
| 38. | The | e most serious ty | ype of water loss fi | om a | canal is | 3/7 | |
| | (a) | evaporation | (b) Seepage | (c) | absorption | (d) | none of these |
| 39. | Ber | ms are provided | d to | | | | |
| | (a) | Strengthen the | canal bank | (b) | Check the seepage | | |
| | (c) | remove silt fro | m the canal | (d) | both (a) and (b) | | |
| 40. | | ined canal is in | | | | | 4. |
| | (a) | initial regime | (b) permanent reg | gime | (c) final regime | (d) | none of these |
| 41. | Ke | nnedy in his silt | theory, assumed th | at the | e silt is kept in susp | ensi | on due to eddies |
| | gen | erated from the | • | | | | |
| | (a) | bed only | (b) sides only | (c) | whole perimeter (c | d) an | y one of these |
| 42. | Th | e relation given | by Kennedy for cr | itical | velocity is | | |
| | (a) | V = 0.55 m D | 0.64 | (b) | $V = 0.64 \text{ mD}^{0.55}$ | | |
| | | $V = 0.74 \text{ mD}^0$ | | (d) | $V = 0.84 \text{ m } D^{0.74}$ | | |
| | | | | | • | | |

| 43. | If m _r is the mean particle diameter of the s | ilt in mm, the Lace | y's silt factor (f) is |
|-----|--|------------------------|--------------------------------|
| | given by (a) $f = 1.76 \text{ m}_r^{3/2}$ (b) $f = 1.76 \text{ m}_r^{1/2}$ (c) | $f = 1.76 \text{ m}^2$ | (d) $f = 1.76 \text{ m}^{5/2}$ |
| 44 | The perimeter - discharge (P - Q) relations | | (d) 1 = 1.70 m _f |
| 77. | (a) $P = 2.25 \sqrt{Q}$ (b) $P = 4.75 \sqrt{Q}$ (c) | | (d) $P = 1.75 \sqrt{Q}$ |
| 45. | Distributary head regulator is provided | | (-) |
| | (a) to control the supplies to the off-taking | canal | |
| | (b) to control the silt entry in the off-taking | | |
| | (c) both (a) and (b) | | |
| | (d) none of these | | |
| 46. | A structure constructed in an irrigation can | al for the purpose | of wasting some of |
| | its water is known as | | _ |
| | (a) fall (b) escape (c) | regulator | (d) none of these |
| 47. | are known as safety valves for cana | ls. | |
| | (a) escapes (b) regulators (c) | falls | (d) outlets |
| 48. | The Sarda canal has | | |
| | (a) glacis type fall (b) vertical drop fall | (c) ogee fall | (d) rapid fall |
| 49. | A parabolic glacis type fall is commonly kn | own as | |
| | (a) Inglis fall (b) Sarda fall (c) | Montague fall (d) | Vertical type fall |
| 50. | Average normal size of a rain drop may be | | |
| | (a) 0.5 - 4 mm (b) 5 - 10 mm (c) | | (d) None of these |
| 51. | Outlet discharge factor is the duty at the h | | |
| | (a) main canal (b) branch canal (c) | Water course | (d) distributary |
| 52. | As per Lacey's theory, the silt factor is | | |
| | (a) Directly proportional to average particle | | |
| | (b) directly proportional to square root of av | | |
| | (c) inversely proportional to average particle | size | |
| | (d) not related to average particle size | | |
| 53. | Garret's diagrams are based on | / \ TZ1 | (1) 1011 11 11 |
| ~ 4 | (a) Lacey's theory (b) Kennedy's theory | • | ., . |
| 54. | According to Bligh's Creep theory the perce | olation water creep | S |
| | (a) in a straight line under the floor | | .1 *1 |
| | (b) along the contact of the base profile of t | ne apron with the si | 108011 |
| • | (c) in a straight path under the foundation | | |
| | (d) none of the above | | |
| | | | • |

| 55. | Wei | rs designed and constructed on Blig | h's | theory failed due to undermining of |
|------------|-------|---|-------|--|
| | (a) | 4 • 4 • 4 • 4 • • • • • • • • • • • • • | (c) | |
| 56. | A c | heck dam is a | | |
| | (a) | Flood control structure | (b) | Soil conservation structure |
| | (c) | river training structure | (d) | Water storage structure |
| 57. | In a | canal siphon the flow is under | | |
| | (a) | atmospheric pressure | (b) | negative pressure |
| | (c) | positive pressure | | |
| 58. | Sup | erpassage is a structure in which | | |
| | (a) | Canal flows over a drainage channe | :1 | → Canal |
| | (b) | drainage channel flows over a canal | l | ↑ j |
| | (0) | both flow at the same level | | River |
| | (c) | both flow at the same level | | AquaductSyphoa aquaduct |
| 59. | A st | ructure in which canal flows over | the | |
| | drai | n and bed of the canal is above H. | F.L. | ↑ River |
| | of th | ne drain, it is called | | |
| | (a) | aqueduct | | |
| | (b) | syphon aquaduct | | → Canal |
| | (c) | Super passage | | Super Press |
| | (d) | canal syphon | | - Super Passage - Canal syphon |
| 60. | | ructure in which canal flows over | | |
| | and | HFL of the drain is above the bed | of | canal, it is called |
| | • • | aqueduct (b) syphon aquaduc | | (c) Super passage (d) canal syption |
| 61. | | cutoff in the earthdam is provided | | |
| | (a) | reduce seepage (b) Prevent piping | (c) | both (a) and (b) (d) none of these |
| 62. | | stability of the downstream slope of | of ar | |
| | | | (b) | steady seepage |
| <i>(</i> 2 | | | (d) | both (b) and (c) |
| 63. | | the design of lined canal, the form | | |
| | | 3.6 | | Lacey's formula |
| <i>.</i> | | | (d) | Lindley's formula |
| 64. | | bmerged pipe outlet is a | | |
| | | | (b) | Semi-modular outlet |
| | (c) | non-modular outlet | (d) | both (b) and (c) |

- 65. A cross regulator on a canal is used to
 - increase the water level u/s of the off take canal
 - control the sediment entering the off take canal (b)
 - dispose off excess supply in the parent channel (c)
 - (d) increase discharge on the downstream
- 66. In a Sarda type fall, the energy dissipation is mainly due to
 - Water cushion (b) hydraulic jump (c) baffle blocks
- 67. For silt whose average size is 0.18 mm, Lacey's site factor will be about
 - 0.65
- (b) 0.85
- (c) 0.75
- (d) 0.95

(d) Chute blocks

Hint:

$$f = 1.76 \text{ m}_r^{1/2} = 1.76 \times (0.18)^{1/2} = 0.746$$

68. Match list I and list II correctly.

List I

- Arch dam A.
- Gravity dam B.
- C. Earth dam
- Buttress dam D.
- - Rock foundation 1.
 - Strong abutment 2.
 - 3. Any type of foundation
 - 4. Multiple arch

List II

Codes:

| A | 1 | В | C | D |
|---|---|---|---|---|
| 2 | | 1 | 3 | 4 |

- (a) 3 (b) 2
- 3 2 (c) 1
- 3 2 (d)
- 69. Match list I and list II correctly.

List I

- Dams A.
- Weirs В.
- Spillways C.
- Barrage D.

- List II
- Flood control 1.
- 2. Large gates
- 3. Storage of water
- Diversion of water 4.

Codes:

- (a)
- 2 1 (b) 4 3
- 3 2 1 (c)
- 3 2 1 (d)

| 70. | Mat | ch li | st I and lis | t II and | l select | the co | rrect answer using the codes given below: |
|-----|------|-------|--------------|----------|----------|---------|---|
| | | | List I | | | | List II |
| | | A. | Divide wa | all | | 1. | Flood control |
| | | B. | Wing wal | 1 . | | 2. | Diversion headworks |
| | • | C. | Breast wa | III | | 3. | Cross drainage works |
| | | D. | Flood wa | 11 | | 4. | Head regulator |
| | Cod | es: | | | | | |
| | | Α | В | C 1 | D | | |
| | (a) | 2 | 3 | Į 4 | 4 | | |
| | ` ' | 3 | | 1 : | 1 | | |
| | (c) | 2 | 3 | 1 | 1 | | |
| | (d) | 3 | 2 | i 4 | 1 | | |
| 71. | The | | II/AT >> | | | an ir | rigation project : |
| | 1. | | ersion head | d works | | | 2. Head regulator |
| | 3. | Silt | extractor | | | | 4. Cross regulator |
| | Writ | | e correct se | 76 | | | |
| | (a) | | | | | | (c) 1, 3, 4, 2 (d) 1, 2, 3, 4 |
| 72. | Cav | | | | | | following type of spillway. |
| | (a) | _ | • | ` ' | - | spillwa | y (c) shaft spillway (d) chute spillway |
| 73. | _ | | eter is used | | | | |
| | (a) | | | | _ | | (c) evapotranspiration (d) surface run-off |
| 74. | | | | | can be | releas | ed by gravitational flow from a unit volume |
| | | _ | er is called | | | | |
| | (a) | - | osity | _ | | | (b) specific yield |
| L | (c) | | cific retent | | | | (d) specific capacity |
| 75. | | | | st II an | id selec | t the | correct answer using the codes given below |
| | the | lists | | | | | X * 4 XX |
| | | | List I | | | | List II |
| | | | Aquifer | | | 1. | Stores and yields sufficient water |
| | | В. | Aquiclude | • | | 2. | Stores but yields very little water |
| | | C. | Aquitard | | | 3. | Neither stores nor yields water |
| | ~ 1 | D. | Aquifuge | | | 4. | Stores but does not yield water |
| | Cod | | 'n | 0 | ъ | | |
| | (-) | A | | C | D | | et e |
| | (a) | 1 | 4 | 2 | 3 | | |
| | (b) | 1 | 3 | 4 | 2 | | |
| | (c) | 1 | 2 | 4 | | | |
| | (d) | 1 | 2 | 3 | 4 | | |

| 77. For an earthdam, least suited spillway is (a) Ogee spillway (b) Chute spillway (c) Shaft spillway (d) none of these 78. The overfall of a spillway in the shape of a double or S-curve, which is convex at the top and concave at the bottom is called (a) Ogee spillway (b) S-spillway (c) oval spillway (d) zig-zag spillway 79. Due to topography, if space is not available, the most suitable spillway is (a) ogee spillway (b) chute spillway (c) shaft spillway (d) stratight drop spill way 80. According to Khosla's theory, the undermining of the floor starts from the (a) tail end (b) starting end (c) intermediate point (d) foundation bed 81. A balancing reservoir is one which (a) balances the peak and minimum flows (b) balances the flow rates of supply and demand (d) Stores water for emergencies 82. Specific yield for an unconfied aquifer is (a) greater than porosity (b) less than porosity (c) equal to porosity (d) unrelated to porosity 83. Guide banks are provided to (a) train the flow of river along a specified course (b) reduce the peak flood discharge (c) confine the width of river (d) increase the water way 84. In a river, silt excluder and silt ejector are constructed at (a) a location after the head regulator and at the head of the canal, respectively (b) the head of the canal and at a location after the head regulator, respectively (c) the location of which is independent of the command to be served (d) designed to secure raising of water surface on its upstream 85. Balancing depth of cutting of canal is (a) half the total depth of canal (b) half of full supply depth (c) Maximum cut that an excavator can take | 76. | The discharge per unit drawdown at a well is called | | | | | | | |
|--|------------|---|--|--|--|--|--|--|--|
| (a) Ogee spillway (b) Chute spillway (c) Shaft spillway (d) none of these 78. The overfall of a spillway in the shape of a double or S-curve, which is convex at the top and concave at the bottom is called (a) Ogee spillway (b) S-spillway (c) oval spillway (d) zig-zag spillway 79. Due to topography, if space is not available, the most suitable spillway is (a) ogee spillway (b) chute spillway (c) shaft spillway (d) straight drop spill way 80. According to Khosla's theory, the undermining of the floor starts from the (a) tail end (b) starting end (c) intermediate point (d) foundation bed 81. A balancing reservoir is one which (a) balances the peak and minimum flows (b) balances the distribution (c) Balances the flow rates of supply and demand (d) Stores water for emergencies 82. Specific yield for an unconfied aquifer is (a) greater than porosity (d) unrelated to porosity (c) equal to porosity (d) unrelated to porosity 83. Guide banks are provided to (a) train the flow of river along a specified course (b) reduce the peak flood discharge (c) confine the width of river (d) increase the water way 84. In a river, silt excluder and silt ejector are constructed at (a) a location after the head regulator and at the head of the canal, respectively (b) the head of the canal and at a location after the head regulator, respectively (c) the location of which is independent of the command to be served (d) designed to secure raising of water surface on its upstream 85. Balancing depth of cutting of canal is (a) half the total depth of canal (b) half of full supply depth (c) Maximum cut that an excavator can take | | (a) specific yield (b) specific capacity (c) storage coefficient (d) transmissibility | | | | | | | |
| The overfall of a spillway in the shape of a double or S-curve, which is convex at the top and concave at the bottom is called (a) Ogee spillway (b) S-spillway (c) oval spillway (d) zig-zag spillway 79. Due to topography, if space is not available, the most suitable spillway is (a) ogee spillway (b) chute spillway (c) shaft spillway (d) straight drop spill way 80. According to Khosla's theory, the undermining of the floor starts from the (a) tail end (b) starting end (c) intermediate point (d) foundation bed 81. A balancing reservoir is one which (a) balances the peak and minimum flows (b) balances the flow rates of supply and demand (d) Stores water for emergencies 82. Specific yield for an unconfied aquifer is (a) greater than porosity (b) less than porosity (c) equal to porosity (d) unrelated to porosity 83. Guide banks are provided to (a) train the flow of river along a specified course (b) reduce the peak flood discharge (c) confine the width of river (d) increase the water way 84. In a river, silt excluder and silt ejector are constructed at (a) a location after the head regulator and at the head of the canal, respectively (b) the head of the canal and at a location after the head regulator, respectively (b) the head of the canal and at a location after the head regulator, respectively (c) the location of which is independent of the command to be served (d) designed to secure raising of water surface on its upstream 85. Balancing depth of cutting of canal is (a) half the total depth of canal (b) half of full supply depth (c) Maximum cut that an excavator can take | 77. | For an earthdam, least suited spillway is | | | | | | | |
| the top and concave at the bottom is called (a) Ogee spillway (b) S-spillway (c) oval spillway (d) zig-zag spillway 79. Due to topography, if space is not available, the most suitable spillway is (a) ogee spillway (b) chute spillway (c) shaft spillway (d) straight drop spill way 80. According to Khosla's theory, the undermining of the floor starts from the (a) tail end (b) starting end (c) intermediate point (d) foundation bed 81. A balancing reservoir is one which (a) balances the peak and minimum flows (b) balances the flow rates of supply and demand (d) Stores water for emergencies 82. Specific yield for an unconfied aquifer is (a) greater than porosity (b) less than porosity (c) equal to porosity (d) unrelated to porosity 83. Guide banks are provided to (a) train the flow of river along a specified course (b) reduce the peak flood discharge (c) confine the width of river (d) increase the water way 84. In a river, silt excluder and silt ejector are constructed at (a) a location after the head regulator and at the head of the canal, respectively (b) the head of the canal and at a location after the head regulator, respectively (c) the location of which is independent of the command to be served (d) designed to secure raising of water surface on its upstream 85. Balancing depth of cutting of canal is (a) half the total depth of canal (b) half of full supply depth (c) Maximum cut that an excavator can take | | (a) Ogee spillway (b) Chute spillway (c) Shaft spillway (d) none of these | | | | | | | |
| (a) Ogee spillway (b) S-spillway (c) oval spillway (d) zig-zag spillway 79. Due to topography, if space is not available, the most suitable spillway is (a) ogee spillway (b) chute spillway (c) shaft spillway (d) straight drop spill way 80. According to Khosla's theory, the undermining of the floor starts from the (a) tail end (b) starting end (c) intermediate point (d) foundation bed 81. A balancing reservoir is one which (a) balances the peak and minimum flows (b) balances the flow rates of supply and demand (d) Stores water for emergencies 82. Specific yield for an unconfied aquifer is (a) greater than porosity (b) less than porosity (c) equal to porosity (d) unrelated to porosity 83. Guide banks are provided to (a) train the flow of river along a specified course (b) reduce the peak flood discharge (c) confine the width of river (d) increase the water way 84. In a river, silt excluder and silt ejector are constructed at (a) a location after the head regulator and at the head of the canal, respectively (b) the head of the canal and at a location after the head regulator, respectively (c) the location of which is independent of the command to be served (d) designed to secure raising of water surface on its upstream 85. Balancing depth of cutting of canal is (a) half the total depth of canal (b) half of full supply depth (c) Maximum cut that an excavator can take | 78. | The overfall of a spillway in the shape of a double or S-curve, which is convex at | | | | | | | |
| 79. Due to topography, if space is not available, the most suitable spillway is (a) ogee spillway (b) chute spillway (c) shaft spillway (d) straight drop spill way 80. According to Khosla's theory, the undermining of the floor starts from the (a) tail end (b) starting end (c) intermediate point (d) foundation bed 81. A balancing reservoir is one which (a) balances the peak and minimum flows (b) balances the flow rates of supply and demand (d) Stores water for emergencies 82. Specific yield for an unconfied aquifer is (a) greater than porosity (b) less than porosity (c) equal to porosity (d) unrelated to porosity 83. Guide banks are provided to (a) train the flow of river along a specified course (b) reduce the peak flood discharge (c) confine the width of river (d) increase the water way 84. In a river, silt excluder and silt ejector are constructed at (a) a location after the head regulator and at the head of the canal, respectively (b) the head of the canal and at a location after the head regulator, respectively (c) the location of which is independent of the command to be served (d) designed to secure raising of water surface on its upstream 85. Balancing depth of cutting of canal is (a) half the total depth of canal (b) half of full supply depth (c) Maximum cut that an excavator can take | | the top and concave at the bottom is called | | | | | | | |
| (a) ogee spillway (b) chute spillway (c) shaft spillway (d) straight drop spill way 80. According to Khosla's theory, the undermining of the floor starts from the (a) tail end (b) starting end (c) intermediate point (d) foundation bed 81. A balancing reservoir is one which (a) balances the peak and minimum flows (b) balances the distribution (c) Balances the flow rates of supply and demand (d) Stores water for emergencies 82. Specific yield for an unconfied aquifer is (a) greater than porosity (b) less than porosity (c) equal to porosity (d) unrelated to porosity 83. Guide banks are provided to (a) train the flow of river along a specified course (b) reduce the peak flood discharge (c) confine the width of river (d) increase the water way 84. In a river, silt excluder and silt ejector are constructed at (a) a location after the head regulator and at the head of the canal, respectively (b) the head of the canal and at a location after the head regulator, respectively (c) the location of which is independent of the command to be served (d) designed to secure raising of water surface on its upstream 85. Balancing depth of cutting of canal is (a) half the total depth of canal (b) half of full supply depth (c) Maximum cut that an excavator can take | | (a) Ogee spillway (b) S-spillway (c) oval spillway (d) zig-zag spillway | | | | | | | |
| (c) shaft spillway (d) straight drop spill way 80. According to Khosla's theory, the undermining of the floor starts from the (a) tail end (b) starting end (c) intermediate point (d) foundation bed 81. A balancing reservoir is one which (a) balances the peak and minimum flows (b) balances the distribution (c) Balances the flow rates of supply and demand (d) Stores water for emergencies 82. Specific yield for an unconfied aquifer is (a) greater than porosity (b) less than porosity (c) equal to porosity (d) unrelated to porosity 83. Guide banks are provided to (a) train the flow of river along a specified course (b) reduce the peak flood discharge (c) confine the width of river (d) increase the water way 84. In a river, silt excluder and silt ejector are constructed at (a) a location after the head regulator and at the head of the canal, respectively (b) the head of the canal and at a location after the head regulator, respectively (c) the location of which is independent of the command to be served (d) designed to secure raising of water surface on its upstream 85. Balancing depth of cutting of canal is (a) half the total depth of canal (b) half of full supply depth (c) Maximum cut that an excavator can take | 79. | Due to topography, if space is not available, the most suitable spillway is | | | | | | | |
| 80. According to Khosla's theory, the undermining of the floor starts from the (a) tail end (b) starting end (c) intermediate point (d) foundation bed 81. A balancing reservoir is one which (a) balances the peak and minimum flows (b) balances the flow rates of supply and demand (d) Stores water for emergencies 82. Specific yield for an unconfied aquifer is (a) greater than porosity (b) less than porosity (c) equal to porosity (d) unrelated to porosity 83. Guide banks are provided to (a) train the flow of river along a specified course (b) reduce the peak flood discharge (c) confine the width of river (d) increase the water way 84. In a river, silt excluder and silt ejector are constructed at (a) a location after the head regulator and at the head of the canal, respectively (b) the head of the canal and at a location after the head regulator, respectively (c) the location of which is independent of the command to be served (d) designed to secure raising of water surface on its upstream 85. Balancing depth of cutting of canal is (a) half the total depth of canal (b) half of full supply depth (c) Maximum cut that an excavator can take | | | | | | | | | |
| (a) tail end (b) starting end (c) intermediate point (d) foundation bed 81. A balancing reservoir is one which (a) balances the peak and minimum flows (b) balances the distribution (c) Balances the flow rates of supply and demand (d) Stores water for emergencies 82. Specific yield for an unconfied aquifer is (a) greater than porosity (b) less than porosity (c) equal to porosity (d) unrelated to porosity 83. Guide banks are provided to (a) train the flow of river along a specified course (b) reduce the peak flood discharge (c) confine the width of river (d) increase the water way 84. In a river, silt excluder and silt ejector are constructed at (a) a location after the head regulator and at the head of the canal, respectively (b) the head of the canal and at a location after the head regulator, respectively (c) the location of which is independent of the command to be served (d) designed to secure raising of water surface on its upstream 85. Balancing depth of cutting of canal (b) half of full supply depth (c) Maximum cut that an excavator can take | | | | | | | | | |
| 81. A balancing reservoir is one which (a) balances the peak and minimum flows (b) balances the distribution (c) Balances the flow rates of supply and demand (d) Stores water for emergencies 82. Specific yield for an unconfied aquifer is (a) greater than porosity (b) less than porosity (c) equal to porosity (d) unrelated to porosity 83. Guide banks are provided to (a) train the flow of river along a specified course (b) reduce the peak flood discharge (c) confine the width of river (d) increase the water way 84. In a river, silt excluder and silt ejector are constructed at (a) a location after the head regulator and at the head of the canal, respectively (b) the head of the canal and at a location after the head regulator, respectively (c) the location of which is independent of the command to be served (d) designed to secure raising of water surface on its upstream 85. Balancing depth of cutting of canal is (a) half the total depth of canal (b) half of full supply depth (c) Maximum cut that an excavator can take | 80. | | | | | | | | |
| (a) balances the peak and minimum flows (b) balances the distribution (c) Balances the flow rates of supply and demand (d) Stores water for emergencies 82. Specific yield for an unconfied aquifer is (a) greater than porosity (b) less than porosity (c) equal to porosity (d) unrelated to porosity 83. Guide banks are provided to (a) train the flow of river along a specified course (b) reduce the peak flood discharge (c) confine the width of river (d) increase the water way 84. In a river, silt excluder and silt ejector are constructed at (a) a location after the head regulator and at the head of the canal, respectively (b) the head of the canal and at a location after the head regulator, respectively (c) the location of which is independent of the command to be served (d) designed to secure raising of water surface on its upstream 85. Balancing depth of cutting of canal is (a) half the total depth of canal (b) half of full supply depth (c) Maximum cut that an excavator can take | | | | | | | | | |
| (b) balances the distribution (c) Balances the flow rates of supply and demand (d) Stores water for emergencies 82. Specific yield for an unconfied aquifer is (a) greater than porosity (b) less than porosity (c) equal to porosity (d) unrelated to porosity 83. Guide banks are provided to (a) train the flow of river along a specified course (b) reduce the peak flood discharge (c) confine the width of river (d) increase the water way 84. In a river, silt excluder and silt ejector are constructed at (a) a location after the head regulator and at the head of the canal, respectively (b) the head of the canal and at a location after the head regulator, respectively (c) the location of which is independent of the command to be served (d) designed to secure raising of water surface on its upstream 85. Balancing depth of cutting of canal is (a) half the total depth of canal (b) half of full supply depth (c) Maximum cut that an excavator can take | 81. | | | | | | | | |
| (c) Balances the flow rates of supply and demand (d) Stores water for emergencies 82. Specific yield for an unconfied aquifer is (a) greater than porosity (b) less than porosity (c) equal to porosity (d) unrelated to porosity 83. Guide banks are provided to (a) train the flow of river along a specified course (b) reduce the peak flood discharge (c) confine the width of river (d) increase the water way 84. In a river, silt excluder and silt ejector are constructed at (a) a location after the head regulator and at the head of the canal, respectively (b) the head of the canal and at a location after the head regulator, respectively (c) the location of which is independent of the command to be served (d) designed to secure raising of water surface on its upstream 85. Balancing depth of cutting of canal is (a) half the total depth of canal (b) half of full supply depth (c) Maximum cut that an excavator can take | | | | | | | | | |
| (d) Stores water for emergencies 82. Specific yield for an unconfied aquifer is (a) greater than porosity (b) less than porosity (c) equal to porosity (d) unrelated to porosity 83. Guide banks are provided to (a) train the flow of river along a specified course (b) reduce the peak flood discharge (c) confine the width of river (d) increase the water way 84. In a river, silt excluder and silt ejector are constructed at (a) a location after the head regulator and at the head of the canal, respectively (b) the head of the canal and at a location after the head regulator, respectively (c) the location of which is independent of the command to be served (d) designed to secure raising of water surface on its upstream 85. Balancing depth of cutting of canal is (a) half the total depth of canal (b) half of full supply depth (c) Maximum cut that an excavator can take | | | | | | | | | |
| 82. Specific yield for an unconfied aquifer is (a) greater than porosity (b) less than porosity (c) equal to porosity (d) unrelated to porosity 83. Guide banks are provided to (a) train the flow of river along a specified course (b) reduce the peak flood discharge (c) confine the width of river (d) increase the water way 84. In a river, silt excluder and silt ejector are constructed at (a) a location after the head regulator and at the head of the canal, respectively (b) the head of the canal and at a location after the head regulator, respectively (c) the location of which is independent of the command to be served (d) designed to secure raising of water surface on its upstream 85. Balancing depth of cutting of canal is (a) half the total depth of canal (b) half of full supply depth (c) Maximum cut that an excavator can take | | | | | | | | | |
| (a) greater than porosity (b) less than porosity (c) equal to porosity (d) unrelated to porosity 83. Guide banks are provided to (a) train the flow of river along a specified course (b) reduce the peak flood discharge (c) confine the width of river (d) increase the water way 84. In a river, silt excluder and silt ejector are constructed at (a) a location after the head regulator and at the head of the canal, respectively (b) the head of the canal and at a location after the head regulator, respectively (c) the location of which is independent of the command to be served (d) designed to secure raising of water surface on its upstream 85. Balancing depth of cutting of canal is (a) half the total depth of canal (b) half of full supply depth (c) Maximum cut that an excavator can take | | | | | | | | | |
| (c) equal to porosity (d) unrelated to porosity 83. Guide banks are provided to (a) train the flow of river along a specified course (b) reduce the peak flood discharge (c) confine the width of river (d) increase the water way 84. In a river, silt excluder and silt ejector are constructed at (a) a location after the head regulator and at the head of the canal, respectively (b) the head of the canal and at a location after the head regulator, respectively (c) the location of which is independent of the command to be served (d) designed to secure raising of water surface on its upstream 85. Balancing depth of cutting of canal is (a) half the total depth of canal (b) half of full supply depth (c) Maximum cut that an excavator can take | 82. | · · · · · · · · · · · · · · · · · · · | | | | | | | |
| 83. Guide banks are provided to (a) train the flow of river along a specified course (b) reduce the peak flood discharge (c) confine the width of river (d) increase the water way 84. In a river, silt excluder and silt ejector are constructed at (a) a location after the head regulator and at the head of the canal, respectively (b) the head of the canal and at a location after the head regulator, respectively (c) the location of which is independent of the command to be served (d) designed to secure raising of water surface on its upstream 85. Balancing depth of cutting of canal is (a) half the total depth of canal (b) half of full supply depth (c) Maximum cut that an excavator can take | | | | | | | | | |
| (a) train the flow of river along a specified course (b) reduce the peak flood discharge (c) confine the width of river (d) increase the water way 84. In a river, silt excluder and silt ejector are constructed at (a) a location after the head regulator and at the head of the canal, respectively (b) the head of the canal and at a location after the head regulator, respectively (c) the location of which is independent of the command to be served (d) designed to secure raising of water surface on its upstream 85. Balancing depth of cutting of canal is (a) half the total depth of canal (b) half of full supply depth (c) Maximum cut that an excavator can take | 0.0 | | | | | | | | |
| (b) reduce the peak flood discharge (c) confine the width of river (d) increase the water way 84. In a river, silt excluder and silt ejector are constructed at (a) a location after the head regulator and at the head of the canal, respectively (b) the head of the canal and at a location after the head regulator, respectively (c) the location of which is independent of the command to be served (d) designed to secure raising of water surface on its upstream 85. Balancing depth of cutting of canal is (a) half the total depth of canal (b) half of full supply depth (c) Maximum cut that an excavator can take | 83. | | | | | | | | |
| (c) confine the width of river (d) increase the water way 84. In a river, silt excluder and silt ejector are constructed at (a) a location after the head regulator and at the head of the canal, respectively (b) the head of the canal and at a location after the head regulator, respectively (c) the location of which is independent of the command to be served (d) designed to secure raising of water surface on its upstream 85. Balancing depth of cutting of canal is (a) half the total depth of canal (b) half of full supply depth (c) Maximum cut that an excavator can take | | | | | | | | | |
| (d) increase the water way 84. In a river, silt excluder and silt ejector are constructed at (a) a location after the head regulator and at the head of the canal, respectively (b) the head of the canal and at a location after the head regulator, respectively (c) the location of which is independent of the command to be served (d) designed to secure raising of water surface on its upstream 85. Balancing depth of cutting of canal is (a) half the total depth of canal (b) half of full supply depth (c) Maximum cut that an excavator can take | | | | | | | | | |
| 84. In a river, silt excluder and silt ejector are constructed at (a) a location after the head regulator and at the head of the canal, respectively (b) the head of the canal and at a location after the head regulator, respectively (c) the location of which is independent of the command to be served (d) designed to secure raising of water surface on its upstream 85. Balancing depth of cutting of canal is (a) half the total depth of canal (b) half of full supply depth (c) Maximum cut that an excavator can take | | ining Education Panning Innovations | | | | | | | |
| (a) a location after the head regulator and at the head of the canal, respectively (b) the head of the canal and at a location after the head regulator, respectively (c) the location of which is independent of the command to be served (d) designed to secure raising of water surface on its upstream 85. Balancing depth of cutting of canal is (a) half the total depth of canal (b) half of full supply depth (c) Maximum cut that an excavator can take | 0.4 | (// | | | | | | | |
| (b) the head of the canal and at a location after the head regulator, respectively (c) the location of which is independent of the command to be served (d) designed to secure raising of water surface on its upstream 85. Balancing depth of cutting of canal is (a) half the total depth of canal (b) half of full supply depth (c) Maximum cut that an excavator can take | 04. | · | | | | | | | |
| (c) the location of which is independent of the command to be served (d) designed to secure raising of water surface on its upstream 85. Balancing depth of cutting of canal is (a) half the total depth of canal (b) half of full supply depth (c) Maximum cut that an excavator can take | | | | | | | | | |
| (d) designed to secure raising of water surface on its upstream 85. Balancing depth of cutting of canal is (a) half the total depth of canal (b) half of full supply depth (c) Maximum cut that an excavator can take | | | | | | | | | |
| 85. Balancing depth of cutting of canal is (a) half the total depth of canal (b) half of full supply depth (c) Maximum cut that an excavator can take | | • | | | | | | | |
| (a) half the total depth of canal(b) half of full supply depth(c) Maximum cut that an excavator can take | 85. | | | | | | | | |
| (b) half of full supply depth (c) Maximum cut that an excavator can take | 05. | | | | | | | | |
| (c) Maximum cut that an excavator can take | | * | | | | | | | |
| | | | | | | | | | |
| (d) where volume of cutting is equal to the volume of filling | | (d) where volume of cutting is equal to the volume of filling | | | | | | | |

| 86. | Pen | man's equation for evapotranspir | ration | is based on | | |
|-----|------|------------------------------------|---------|----------------------|--------|----------------|
| | (a) | energy budget only | | | | |
| | (b) | energy budgeting and water budge | eting | | | • |
| | (c) | energy budgeting and mass transfe | er | | | |
| | (d) | Water budgeting and mass transfe | r | | | |
| 87. | Bha | kra dam is built on | | | | |
| | (a) | Beas river (b) Sutlej river | (c) | Ganga river | (d) | Sindhu river |
| 88. | Ove | er irrigation results in | | | | |
| | (a) | water logging (b) wilting | (c) | Fertility | (d) | None of these |
| 89. | In a | urid region with uneven land surfa | ice, th | e most suitable me | | |
| | (a) | | | Sprinkler irrigation | | |
| 90. | Best | t method of assessment of irrigati | | | ` ' | 3 |
| | | | | | (d) \ | olumetric rate |
| 91. | Rab | i crop pertain | | | | |
| | (a) | Winter season (b) Summer seas | on (c) | Monsoon season (c | l) No | ne of these |
| 92. | Inte | nsity of irrigation means | | | | |
| | (a) | Total depth of water applied to a | crop | (b) percent area irr | rigate | d of C.C.A. |
| | (c) | | | (d) Area left uncul | | |
| 93. | Plan | its develop their root and derive | moistu | | | |
| | (a) | upper portion | (b) | middle portion | | |
| | (c) | lower portion of the root zone | (d) | all of them | | |
| 94. | Soil | is called saline when pH value is | | | | |
| | (a) | 10 (b) 7 | (c) | less than 7 | (d) | 8 |
| 95. | Duty | y of canal water is expressed in | | | V// | 5 |
| | (a) | cumec (b) centimeters | (c) | Hectare/cumec | (d) | None of these |
| 96. | Grav | vity dams can be constructed with | ı | orise in | | |
| | (a) | Concrete (b) Soil | (c) | Sand | (d) | soil and sand |
| 97. | Echo | o sounder is used to measure | | | | |
| | (a) | Width of river | (b) | Velocity of flow | | |
| | (c) | depth of flow | (d) | discharge of flow | | |
| 98. | Gall | ery is | | | | |
| | (a) | a passage left in a body of dam | (b) | an observation tow | er | |
| | (c) | water exit from a dam | (d) | None of these | | |
| 99. | Whi | ch is the foundation seepage contr | | | | |
| | (a) | impervious cut off | | D/S seepage berms | | |
| | (c) | relief wells | | all of them | | |
| | | | ` ′ | | | |

| 100. Grav | vity dam is most suitable when fou | ndat | ion is | | | | |
|------------|---|--------------|--|--|--|--|--|
| (a) | Weak | (b) | strong | | | | |
| (c) | with heavy over burden | (d) | rocky but cracked | | | | |
| 101. Arcl | n dams are constructed in | | | | | | |
| (a) | Narrow valley with strong abutmen | ts (b) | narrow valley with sound foundation | | | | |
| (c) | wide valley with weak foundation | ٠, | none of them | | | | |
| 102. Shaj | pe of elementary profile of gravity | dam | is roughly | | | | |
| (a) | triangular (b) square | (c) | Trapezoidal (d) Rectangular | | | | |
| 103. Eco | nomic height of dam is the one for | whi | ch . | | | | |
| (a) | Cost of dam is minimum (b) | | t of dam per unit storage is minimum | | | | |
| (c) | Storage is maximum (d) | Sto | rage is minimum | | | | |
| 104. Silti | ing of reservoir | | | | | | |
| (a) | reduces efficiency of dam | (b) | | | | | |
| ` ' | raises reservoir water | (d) | increase storage capacity | | | | |
| 105. A r | eservoir is constructed for | | | | | | |
| (a) | irrigation | (b) | | | | | |
| (c) | generating hydro power | (d) | all of these | | | | |
| 106. Roc | ck fill dam is | | high dam (d) None of these | | | | |
| (a) | rigid type (b) non-rigid type | e (c |) high dam (d) None of these | | | | |
| 107. Bla | nket in earthdam is provided | | 1 is all the design dom | | | | |
| (a) | at the ground on the u/s side | | at the ground in the body of the dam | | | | |
| (c) | at the ground on the d/s side of dar | n (d) | on the d/s slope | | | | |
| 108. The | e spillway in which water spills ov | er th | le body of the dam are | | | | |
| (a) | solid gravity spillway | ` ' | siphon spillway | | | | |
| (c) | | (d) | chute spillway | | | | |
| 109. Spi | illway performs the function of a | <i>(</i> 1.) | Setumbro for the dam | | | | |
| (a) | | (b) | · . | | | | |
| (c) | observation opening | (d) | | | | | |
| 110. Th | e provision of filter zone in the d/s sic | le of (| earthen dam serves the function of | | | | |
| (a) | to drain off water seeping through | n eart | h dam safely | | | | |
| (b) | | | | | | | |
| (c) | | ody (| of dam | | | | |
| (d) | all of above | | I / D of hose metaviel should be | | | | |
| 111. Fo | or filter in earthen dam, D ₁₅ of filt | er m | aterial / D ₈₅ of base material should be | | | | |
| (a) | _ // \ _ 0 | (c | $) \leq 5 \qquad (d) \leq 8$ | | | | |

| 112. Co | mmon method of protection of strea | am b | ed below spillways : | are |
|----------|--------------------------------------|--------|-----------------------|-------------------------------------|
| (a) | Grouting of stream bed | | (b) Grouting o | f body of dam |
| (c) | Creation of hydraulic jump on the | d/s fa | ce (d) Provision of | of sluices |
| 113. Slt | tices are provided in a dam to | | | |
| (a) | Supply water to d/s area | (b) | reduce flood | |
| (c) | remove sediment from reservoir | (d) | all of these | • |
| 114. In | the earthdams spillways are provid | ed | | |
| (a) | in the body of the dam | (b) | in the foundation of | f the dam |
| (c) | by the side of the dam structure | (d) | all of them | |
| 115. Cr | est of an emergency spillway is fixe | d at | | |
| (a) | F.R.L. (b) M.W.L. | (c) | dead storage | (d) top of dam |
| 116. M | ain canal take off from | | | |
| (a) | a head regulator (b) a cross regulat | or (c) | a reservoir | (d) a well |
| 117. Bh | andara irrigation is | | | |
| (a) | runoff the river scheme | (b) | minor scheme | |
| (c) | unproductive scheme | (d) | the only effective se | cheme |
| 118. Ol | khla weir on river Yamuna in Delhi | is ar | example of | |
| (a) | Vertical drop weir | (b) | dry stone slope wei | r |
| (c) | concrete slope weir | (d) | parabolic weir | |
| 119. In | barrage, crest level is kept | | 5//75 | |
| (a) | low with large gates | (b) | high with large gate | es |
| (c) | high with small gates | (d) | low with small gate | es |
| 120. Ac | cording to Khosla's theory, the crit | tical | hydraulic gradient : | for <mark>alluvial soils i</mark> s |
| ap | proximately | | | |
| (a) | 1 (b) 1.5 | (c) | 2 | (d) 2.5 |
| 121. Ac | cording to Khosla's theory, the exit | grad | lient in the absence | of a d/s cut off is |
| (a) | zero (b) unity | (c) | infinity | (d) none of these |
| 122. Gi | bb's module is a type of | | | |
| (a) | non-modular outlet | (b) | semi-modular outlet | t |
| (c) | rigid-modular outlet | (d) | Open flume outlet | |
| 123. Be | rms are provided to | | | |
| (a) | remove the silt from the canal | (b) | Strengthen the canal | bank |
| (c) | to increase the seepage from canal | (d) | allow traffic along c | anal |
| 124. Do | owla on the canal bank serves the p | urpos | se of | • |
| (a` | a drainage (b) a foot path | (c) | a road curb | (d) none of these |

| 143 | Spo | on dank, is a term used to designa | te | | | | | | |
|------|-------|---|--------|-----------------------------------|-------|--------------|--|--|--|
| | (a) | a canal bank constructed wrongly | | • | | | | | |
| | (b) | damaged canal bank | | | | | | | |
| | (c) | additional embankment constructed | to di | spose of excess exca | avate | d material | | | |
| | (d) | none of these | | | | | | | |
| 126. | . Whe | en excavated soil is in excess it is de | eposit | ted in spoil banks. V | When | extra amount | | | |
| | of ea | arth is required it is obtained fron | 1 | | | | | | |
| | (a) | Berms (b) dowla | (c) | borrow pits | (d) | side strips | | | |
| 127. | Sand | l core is provided in the canal ban | ıks to |) | | | | | |
| | (a) | make the bank porous to allow quic | ek dra | ainage of seepage w | ater | | | | |
| | (b) | plug the rat holes | | | | | | | |
| • | (c) | reduce clay content in the banks | | | | | | | |
| | (d) | all of these | | | | | | | |
| 128. | A he | eadwork is an assemblage of the fo | llow | ing hy <mark>draulic struc</mark> | tures | | | | |
| | (a) | A dam, embankments, spillway | (b) | A weir, guide bank | s, he | ad regulator | | | |
| | (c) | A syphon, spurs, bank revetment | (d) | none of these | | | | | |
| 129. | Min | or irrigation scheme serves | | | | | | | |
| | (a) | CCA less than 2000 ha | (b) | CCA less than 10,0 |)00 h | a | | | |
| | (c) | CCA less than 500 ha | (d) | CCA less than 200 | ha | | | | |
| 130. | Bhal | kr <mark>a Nangal project is</mark> | | | | | | | |
| | (a) | Multi purpose project | (b) | located in Punjab | | | | | |
| | (c) | The highest gravity dam in India | (d) | all of the above | | | | | |
| 131. | The | side slopes of Narmada main cana | l is | | | | | | |
| | (a) | 1:1 (b) 2:1 | (c) | 1.5 : 1 | (d) | 1:2 | | | |
| 132. | The | length of Narmada main canal is | | | | | | | |
| | (a) | 258 km (b) 458 km | (c) | 600 km | (d) | 655 km | | | |
| 133. | Wate | er table is | | | | | | | |
| | (a) | the top of the zone of saturation be | low g | ground | | | | | |
| | (b) | water surface of a reservoir | | | | | | | |
| | (c) | depth of water standing on the grou | ınd | | | | | | |
| | (d) | all of these | | | | | | | |
| 134. | A de | eep well | | | | 4 | | | |
| | (a) | draw water from top pervious layer | | | | | | | |
| | (b) | draw water from pervious layer belo | ow m | iota layer | | | | | |
| | (c) | draw water from mota layer | • | | | | | | |
| | (4) | none of these | | | | | | | |

135. A mota layer is

(a) a pervious layer

(b) an impervious layer of clay

(c) a layer of sand

(d) none of these

136. An open well is called shallow when

- (a) the depth of well is small
- (b) the water table is high
- (c) it does not encounter mota layer
- (d) none of these

137. Chezy's equation gives velocity of flow. Chezy's constant is given by

- (a) Manning's formula
- (b) Kutter's formula

(c) Bazin's formula

(d) Lacey's regime formula

138. Protective project is generally taken up in

- (a) Hilly areas where communications are difficult
- (b) desert areas where water is scanty
- (c) Periods of famine to provide employment to the people
- (d) None of these

139. A 'damstone' is

- (a) A type of stone
- (b) The material with which dam is constructed
- (c) is a stone pillar embeded on top of bund
- (d) All of them

140. Fish ladder is a structure in the body of the weir used for

(a) Catching fish

- (b) Providing free passage to migrating fish
- (c) restricting movement of fish
- (d) all of these

141, 'Divide wall' is a structure constructed

- (a) to separate the floor of the scouring sluices from that of weir proper
- (b) to provide still pocket in front of the canal head regulator
- (c) to prevent cross currents and flow parallel to weir
- (d) all of the above

142. Weir and barrage are both low barriers built across the river to raise the water but

- (a) Barrage has gates to control flow
- (b) Weir has gates to control flow
- (c) In weir no solid construction is required
- (d) none of these

: ANSWERS :

| | • | ANSWERS: | | |
|-----------|----------|----------|----------|----------|
| 1. (a) | 2. (b) | 3. (c) | 4. (a) | 5. (b) |
| 6. (c) | - 7. (a) | 8. (b) | 9. (b) | 10. (c) |
| - 11. (c) | 12. (b) | 13. (b) | 14. (a) | 15. (b) |
| 16. (b) | 17. (a) | 18. (d) | 19. (a) | 20. (b) |
| 21. (b) | 22. (a) | 23. (c) | 24. (b) | 25. (a) |
| 26. (d) | 27. (d) | 28. (b) | 29. (b) | 30. (c) |
| 31. (b) | 32. (d) | 33. (d) | 34. (b) | 35. (c) |
| 36. (b) | 37. (c) | 38. (b) | 39. (d) | 40. (b) |
| 41. (a) | 42. (a) | 43. (b) | 44. (b) | 45. (c) |
| 46. (b) | 47. (a) | 48. (b) | 49. (c) | 50. (a) |
| 51. (c) | 52. (b) | 53. (b) | 54. (b) | 55. (a) |
| 56. (b) | 57. (c) | 58. (b) | 59. (a) | 60. (b) |
| 61. (c) | 62. (d) | 63. (c) | 64. (c) | 65. (a) |
| 66. (a) | 67. (c) | 68. (a) | 69. (a) | 70. (c) |
| 71. (d) | 72. (d) | 73. (c) | 74. (b) | 75. (a) |
| 76. (b) | 77. (a) | 78. (a) | 79. (c) | 80. (a) |
| 81. (c) | 82. (b) | 83. (c) | 84. (b) | 85. (d) |
| 86. (c) | 87. (b) | 88. (a) | 89. (c) | 90. (d) |
| 91. (a) | 92. (b) | 93. (a) | 94. (a) | 95. (c) |
| 96. (a) | 97. (c) | 98. (a) | 99. (d) | 100. (b) |
| 101. (a) | 102. (a) | 103. (b) | 104. (b) | 105. (d) |
| 106. (b) | 107. (a) | 108. (a) | 109. (b) | 110. (d) |
| 111. (c) | 112. (c) | 113. (d) | 114. (c) | 115. (b) |
| 116. (c) | 117. (a) | 118. (b) | 119. (a) | 120. (a) |
| 121 (c) | 122. (c) | 123. (b) | 124. (c) | 125. (c) |
| 126. (c) | 127. (b) | 128. (b) | 129. (a) | 130. (d) |
| 131. (b) | 132. (b) | 133. (a) | 134. (b) | 135. (b) |
| 136. (c) | 137. (c) | 138. (c) | 139. (c) | 140. (b) |
| 141. (d) | 142. (a) | | | |



Hydrology and Water Resources EngineEring

MCQ'S

SET-1

| | | Tr T-T | | | | |
|----|--|-------------------------|---|-----|--|--|
| 1. | Horton equation is used to estimate | . | | | | |
| | (a) Amount of evaporation | (b) | Infiltration | | | |
| | (c) Soil water content | (d) | Runoff | | | |
| 2. | Lysimeter is used to measure | | | | | |
| | (a) Infiltration | (b) | evaporation | | | |
| | (c) evapotranspiration | (d) | surface run-off | | | |
| 3. | A rainfall of 1.5 cm occured in a 6 | hr stor | m. If the \phi-index was 0.2 cm/hr, | the | | |
| | rainfall excess was | | | | | |
| | (a) 0.0 (b) 0.30 cm | (c) | 1.20 cm (d) $-0.30 cm$ | | | |
| 4. | A double mass curve is used to | |)" <u> </u> | | | |
| | (a) check the consistency of rain gar | <mark>ug</mark> e recor | rd. | | | |
| | (b) determine the reservoir capacity | | | | | |
| | (c) determine the number of rain gar | uges requ | uired. | | | |
| | (d) determine the maximum probable | precipit | tation | | | |
| 5. | Evapotranspiration is confined to | | | | | |
| | (a) daylight hours (b) night time or | ıly (c) | land surface only (d) none of thes | e | | |
| 6. | φ-index is always | | | | | |
| | (a) less than W-index | (b) | more than W-index | | | |
| | (c) equal to W-index | (d) | equal to 1 | | | |
| 7. | The highest value of annual poten | tial evap | potranspiration (PET) in India is | at | | |
| | Rajkot, Gujarat. Here the annual P. | | | | | |
| | (a) 150 cm (b) 150 mm | (c) | 210 cm (d) 310 cm | • | | |
| 8. | The infiltration capacity of a soil wa | as measu | ured under fairly identical conditi | ons | | |
| | by a flooding type infiltratometer as | s f _f and l | by a rainfall simulator as f _r , one | can | | |
| | expect | | | | | |
| | (a) $f_f = f_r$ (b) $f_f > f_r$ | (c) | $f_f < f_r$ (d) no fixed pattern | | | |
| | | | | | | |

9. The chemical that is found to be most suitable as water evaporation inhibitor is

- (a) ethyl alcohol (b) methyl alcohol (c) cetyl alcohol

- (d) butyl alcohol

10. Which one is the most accurate method of finding the average depth of rainfall over a catchment area?

- (a) Arithmetic mean method
- (b) Isohyetal method
- (c) Thiessen polygon method
- all the above

11. 1 mm water on an area represents

- (a) $l g/m^2$
- (b) 10 kg
- (c) 1 mm/m^2
- (d) $1 \text{ dm}^3/\text{m}^2$

12. For identical conditions, evaporation from sea water is

- (a) 2 3% more than that from fresh water
- (b) 2 3% less than that from fresh water
- (c) equal to that from fresh water
- (d) None of these

13. Dalton's law of evaporation is given by

(a) $E = C (e_s/e_a)$

where,

(b) $E = C (e_s + e_a)$

e_s = saturation vapour pressure

(c) $E = C (e_s - e_a)$

 $e_a = vapour pressure in the air$

(d) $E = C (e_a - e_s)$

E = evaporation

14. Orographic lifting results in

- (a) Topographical dependence of precipitation
- (b) Lifting of soil surface after freezing
- (c) Higher ocean water level
- (d) A warmer climate

The average pan coefficient for the Standard US Weather Bureau class A pan A is

- (a) 0.85
- (b) 0.70
- (c) 0.90
- (d) 0.20

IS standard Pan Coeff = 0.80

: ANSWERS:

| 1. | (b) | 2 | | (c) | 3. | (b) | 4. | (a) | 5. | (a) | 6. | (b) |
|-----|-----|-----|----|-----|-----|-------|-----|-----|-----|-----|-----|-----|
| 7. | (c) | . 8 | 3. | (b) | 9. | (c) | 10. | (b) | 11. | (c) | 12. | (b) |
| 13. | (c) | I | 4. | (a) | 15. | (b) · | | | ١. | | | |

EXPLANATIONS

3. (b)
$$\phi = \frac{P - R}{t_r} \text{ cm/hr}$$

$$R = 0.3 \text{ cm}$$

$$\phi = \frac{P - R}{t_r} \text{ cm/hr}$$

$$0.2 = \frac{1.5 - R}{6}$$

$$\therefore R = 0.3 \text{ cm}$$
Winder = $\frac{P - R - S_R}{t_r}$ cm/hr
$$S_R = \text{surface retention}$$

$$S_R = surface$$
 retention

<u>SET-2</u>

| 1. | The infection point on the recession l | imb of the hydrograph in | ndicates the end of |
|-----|---|-----------------------------|--------------------------|
| | (a) base flow (b) direct runoff | (c) overland flow | (d) Rainfall |
| 2. | The concept of unit hydrograph was | first introduced by | |
| | (a) Dalton (b) Sherman | (c) Horton | (d) Thiessen |
| 3. | UH is the graphical representation be | etween the time distribut | ions of |
| | (a) Total rainfall and total runoff | (b) Total rainfall and | d direct runoff |
| | (c) effective rainfall and total runoff | (d) effective rainfall | and direct runoff |
| 4. | Unit hydrograph theory cannot be ap | plied to catchment areas | greater than |
| | (a) 1000 km^2 (b) 2000 km^2 | (c) 5000 km^2 | (d) 8000 km ² |
| 5. | The word 'unit' in unit hydrograph r | efers to the | |
| | (a) unit depth of runoff | (b) unit duration of | the storm |
| | (c) unit base period of the hydrograph | (d) unit area of the | basin |
| 6. | The S-curve hydrograph is the sumn | nation of the | |
| | (a) unit hydrograph | (b) total runoff hydr | ograph |
| | (c) effective rainfall hydrograph | (d) base flow recess | ion curve |
| 7. | A unit hydrograph is a hydrograph r | representing | |
| | (a) 1 cm of runoff | (b) 10 cm of runoff | |
| | (c) 100 cm of runoff | (d) .1000 cm of runc | off |
| 8. | A hydrograph is a plot between | | |
| | (a) discharge versus time | (b) intensity versus | time |
| | (c) depth versus time | (d) none of the above | ve |
| 9. | A hyetograph is a plot between | | |
| | (a) Duration of rainfall against time | (b) Intensity of rain | fall against time |
| | (c) frequency of rainfall against time | (d) none of these | |
| 10. | The S-Curve hydrograph is used to | | |
| | (a) estimate peak flood flow of a basi | n resulting from a given st | torm. |
| | (b) develop synthetic unit hydrograph. | | |
| | (c) convert the UH of any given durat | tion into a UH of any othe | r desired duration. |
| | (d) derive the UH from complex storn | ns | |
| 11. | The area under a hyetograph represe | ents | |
| | (a) the total volume of runoff received | d in that period | |
| | (b) the total rainfall received in that p | period | |
| | (c) the total infiltration received in the | at period | |
| | (d) total surface runoff | | |
| | | . ' | |

Objective Civil Eng. \ 2016 \ 44

| 12. | A rainfall of 1.5 cm | 1 occured in 6–hr st | orm. If \$\phi\$ -index was | 0.1 cm/hr, the rainfall |
|-----|----------------------|----------------------|------------------------------------|-------------------------|
| | excess was | | | |
| | (a) 0.0 | (b) 0.90 cm | (c) 1.20 cm | (d) -0.30 cm |
| | (a) 0.0 | ` ' | | 3 - xc |

13. A 6-hr UH of a catchment has a peak discharge of 15 m³/s. If 3 cm rainfall excess occurs in 6-hr and the base flow is 2 m³/s the peak discharge of the storm hydrograph is

(a) $45 \text{ m}^3/\text{s}$ (b) $47 \text{ m}^3/\text{s}$

(c) $43 \text{ m}^3/\text{s}$ (d) $15 \text{ m}^3/\text{s}$

14. In a synthetic unit hydrograph the width of 75% peak is 20 hr. The width of 50% peak value will be approximately

(a) 20 hr

(b) 30 hr

(c) 35 hr

(d) 12 hr

Hint: $W_{75} = \frac{W_{50}}{1.75}$

15. For a catchment of 40 km², the equilibrium discharge of an S-curve obtained by 3-hr unit hydrographs summation is about

(a) $37.0 \text{ m}^3/\text{s}$

(b) $47.0 \text{ m}^3/\text{s}$

(c) $27.0 \text{ m}^3/\text{s}$

(d) $57.0 \text{ m}^3/\text{s}$

Hint: $Q_{eq} = 2.78 \frac{A}{t_r} \text{ m}^3/\text{s}$

16. Match list-I with list-II:

List-I

A. Flow duration

B. Mass inflow curve

C. Frequency curve

D. S-curve

List-II

- 1. Unit hydrograph
- 2. Available water power
- 3. Storage capacity
- 4. Return period

Codes:

| · | Α | В | C | D |
|-----|---|---|---|---|
| (a) | 2 | 3 | 1 | 4 |
| (b) | 3 | 2 | 4 | 1 |
| (c) | 2 | 3 | 4 | 1 |
| (d) | 1 | 4 | 3 | 2 |

17. A 1-hr unit hydrograph of a catchment is triangular in shape with a base width of 48 hr and a peak discharge of 25 m³/s. The catchment area is about

(a) 21.6 sq.km

(b) 216 sq.km

(c) 21600 sq.km

(d) 432 sq.km '

Hint: Area of $UH = A \times 1$ cm

$$\frac{1}{2} \times 48 \times 60 \times 60 \times 25 = A \times \frac{1}{100}$$
 : $A = 216 \times 10^6 \text{ m}^3/\text{s}$

Hydrology and Water Resources Engineering

: ANSWERS:

| 1. (c) 7. (a) | 2. (b) 8. (a) | 3. (d) 9. (b) | 4. (c) 10. (c) | | 12. (b) |
|------------------|------------------|------------------|-------------------|---------|---------|
| 13. (b) | 14. (c) | 15. (a) | 16. (c) | 17. (b) | |

SET-3

Confined aquifer is one in which 1.

- (a) Water surface under the ground is at atmospheric pressure.
- (b) Water is confined under pressure less than atmospheric pressure between impermeable strata.
- (c) Water is confined at atmospheric pressure between impermeable strata.
- (d) Water is confined under pressure greater than atmospheric pressure between impermeable strata.

An unconfined aquifer is one in which

- (a) Water surface under the ground is at atmospheric pressure.
- (b) Water is confined under pressure less than atmospheric pressure between impermeable strata.
- (c) Water is confined at atmospheric pressure between impermeable strata.
- (d) Water is confined under pressure greater than atmospheric pressure between impermeable strata.
- For a well penetrating an unconfined aquifer having permeability k = 4×10^{-4} m/s, the radius of influence for a drawdown of 5 m is about
 - (a) 100 m (b) 200 m
- (c) 300 m
- (d) 400 m

Hint. R = 3000 s \sqrt{k}

The specific capacity of a well is defined as

- (a) Drawdown per unit discharge
- (b) Discharge per unit drawdown
- (c) Drawdown per certain given discharge
- (d) Discharge per certain given drawdown

The ratio of volume of water drained to the volume of soil sample is 5.

(a) Porosity

(b) Specific yield

(c) Specific retention

(d) Storage coefficient

An aquifer is a geological formation which 6.

- (a) does not contain water
- (b) Contains water but does not transmit
- (c) Contains water and also transmit it
- None of these (d)

| 7. Which geological formations neith | er contain wa | ter nor transmits | It i |
|--|-----------------------|---------------------|--------------------|
| (a) Aquifer (b) Aquifuge | (c) A | quiclude | (d) Aquitard |
| 8. An unconfined aquifer is also calle | ed | | |
| (a) Water table aquifer | (b) a | rtesian aquifer | |
| (c) leaky aquifer | (d) p | erched aquifer | |
| a m ' II tha niazar | netric surface | is | |
| 9. In case of flowing well, the prezent | (1 | b) below ground i | V.T. |
| (c) Between ground level and ground | nd W.T. (| d) Coincides with | ground W.T. |
| drawdown | at a well is | called | |
| | (b) S | Specific capacity | |
| (a) Specific yield | | Fransmissibility | |
| (c) Storage coefficient | | | |
| 11. Match list – I and list – II. | List- | -II | |
| List-I A Aguifer 1. Store and | yield sufficie | | , |
| 11, 11-1-1 | it yields very l | ittle water | |
| D. 124" | tore nor yields | water | |
| O. riquitai | it does not yie | lds water | |
| D. 114 | D D | | |
| Coues . A | 3 | | |
| (a) 1 4 2 | 2 | | |
| (b) 1 3 4 | 3 | | |
| (c) 1 2 4 | 4 | | |
| (d) 1 2 3 12. The volume of water that can be | lossed by gr | avitational flow fr | om a unit volume |
| 12. The volume of water that can be | released by gr | | |
| of aquifer is called | (b) | Specific yield | |
| (a) Porosity | | Specific capacity | |
| (c) Specific retention13. If T is transmissivity conductive | (u) ter and h is f | the width of aqui | fer, the hydraulic |
| 13. If T is transmissivity conductiv | ity and b is t | | |
| conductivity (k) is | (c) | k = T + b | (d) k = T - b |
| (a) $k = T \times b$ (b) $k = T/b$ | | • | |
| 14. Transmissibility has the dimens | (c) | m^2/s (d) | dimensionless |
| (a) m/s (b) m/s^2 | (6) | /,s | |
| · | ANSWERS: | | |
| | | 4 (b) | 5. (b) |
| 1. (d) 2. (a) | 3. (c) | 4. (b) | 10. (b) |
| 6. (c) 7. (b) | 8. (a) | 9. (a) | 10. (0) |
| 11 (a) 12. (b) | 13. (b) | 14. (c) | <u> </u> |

SET-4

| 1. | Multipurpose reservoir is the one which is constructed | | | | | | | |
|-----|---|--|--|--|--|--|--|--|
| | (a) to be used in combination of other reservoirs. | | | | | | | |
| | (b) to serve one main purpose and other purposes are served incidentally. | | | | | | | |
| | (c) to serve more than one purpose. | | | | | | | |
| | (d) to supply water for more than one year. | | | | | | | |
| 2. | ordinate of mass-inflow curve at any time is equal to | | | | | | | |
| | (a) the ordinate of the flood hydrograph at that time. | | | | | | | |
| | (b) the area under the flood hydrograph upto that time. | | | | | | | |
| | (c) the sum of the ordinates of the flood hydrograph upto that time. | | | | | | | |
| | (d) the ordinate of the demand curve at that time. | | | | | | | |
| 3. | Due to valley storage the peak discharge of an outflow hydrograph is | | | | | | | |
| | (a) increased (b) reduced | | | | | | | |
| | (c) not affected (d) sometimes increased | | | | | | | |
| 4. | The water stored in the reservoir below the minimum pool level is called | | | | | | | |
| | (a) usefull storage (b) sucharge storage (c) dead storage (d) valley storage | | | | | | | |
| 5. | Trap efficiency of a reservoir is a function of | | | | | | | |
| | (a) capacity/inflow ratio (b) capacity/outflow ratio | | | | | | | |
| | (c) outflow/inflow ratio (d) only outflow and inflow | | | | | | | |
| 6. | The maximum water level which can be stored in a reservoir during normal | | | | | | | |
| | conditions is known as | | | | | | | |
| | (a) Maximum pool level (b) Minimum pool level | | | | | | | |
| | (c) Normal pool level (d) None of these | | | | | | | |
| 7. | The volume of water which flows through the river before the construction of a | | | | | | | |
| | dam is known as | | | | | | | |
| | (a) Live storage (b) dead storage (c) bank storage (d) valley storage | | | | | | | |
| 8. | The percentage of the total flowing sediment load in a stream retained by a | | | | | | | |
| | reservoir is called | | | | | | | |
| | (a) density current (b) trap efficiency | | | | | | | |
| | (c) sediment load (d) capacity/inflow ratio | | | | | | | |
| 9. | With the reduction in the reservoir capacity over the passage of time, the trap | | | | | | | |
| | efficiency | | | | | | | |
| | (a) increases (b) decreases | | | | | | | |
| | (c) remains unaffected (d) may increase or decrease | | | | | | | |
| 10. | · Capacity-inflow ratio for a storage reservoir is defined as | | | | | | | |
| | (a) reservoir capacity/average annual flood inflow | | | | | | | |

- (b) reservoir capacity/average annual sediment inflow
- (c) dead storage of reservoir/average annual sediment deposited
- (d) None of the above
- 11. With the increase in its capacity-inflow ratio, the trap efficiency of a reservoir
 - (a) increases

(b) decreases

(c) remains unchanged

- (d) may increase or decrease
- 12. The capacity-inflow ratio for a reservoir
 - (a) is a constant factor over time
- (b) increases with time

(c) decreases with time

- (d) may increase or decrease with time
- 13. The dead storage zone in a reservoir is provided for the storage of
 - (a) water for firm power

- (b) sand and silt
- (c) water for water supplies
- (d) none of these
- 14. Trap efficiency of a reservoir indicates the
 - (a) sediment volume trapped in the reservoir
 - (b) sediment volume let out from the reservoir
 - (c) sediment volume trapped in relation to the sediment volume entering the reservoir
 - (d) none of these
- 15. With the reduction in the reservoir capacity the quantum of the sediment trapped by it
 - (a) increases
- (b) decreases
- (c) does not get affected

: ANSWERS:

| 1. (c) | 2. (b) | 3. (b) 4 | . (c) | 5. (a) |
|---------|---------|-----------|--------|---------|
| 6. (c) | 7. (d) | 8. (b) 9 |). (b) | 10. (a) |
| 11. (a) | 12. (c) | 13. (b) 1 | 4. (c) | 15. (b) |

SET-5

1. Match list I and List II.

List - I

- A. Load factor
- B. Power factor
- C. Capacity factor
- D. Utilisation factor

List - II

- 1. Actual power/Apparent power
- 2. Average output/Plant capacity
- 3. Peak power/Installed capacity
- 4. Average load/Peak load

Hydrology and Water Resources Engineering

| -, | | ,, | | | | | | | | | |
|----|-----|---------|-----------|---------|--------|---------|--------|-------|----------------|----------|------------------|
| | Cod | es: | | | | | | | | | |
| | | A | B | C | D | • | | | | | |
| | (a) | 1 | 4 | 3 | 2 | | | | | | |
| | (b) | | 1 | 2 | 3 | | | | | | |
| | (c) | 4 | 1 | 3 | 2 | | | | | | |
| | (d) | 1 | 4 | 2 | 3 | | | | | | |
| 2. | | un-of- | river pl | ant is | one w | hich | | | | | |
| | | | limited | | | | | (b) | has no ponda | ige at a | ill |
| | • • | | umped | | | | | (d) | is a high hea | d plant | |
| 3. | | | l storag | | | e whic | h | | | | |
| | | | ed as a l | | | | | (b) | is same as ru | ın-of-ri | ver plant |
| | | | ed as pe | | | | | (d) | is a high hea | id plant | |
| 4. | A 1 | siah he | ead nlar | nt uses | a hea | d | | | | | |
| | (a) | more | than 15 | m (b) | more | than 3 | 0 m | (c) n | nore than 60 i | m (d) m | nore than 100 n |
| 5. | A 1 | forebay | v is an | enlarge | ed bod | ly of w | ater v | which | ı is provided | just in | i front of the |
| ٠. | | | r house | | | tocks | | (c) | draft tube | | (d) turbines |
| 6. | Th | e hvdr | o powe | | | | | ennia | l river is | | |
| •• | | | of-river | | | | | (b) | Storage plan | it | |
| | | | ed stor | | nt | | | (d) | High head p | lant | |
| 7. | | | tank is | | | ided at | the | | | | |
| ′• | | | eam end | | | | | (b) | downstream | end of | the long penstoc |
| | (c) | _ | ning of | | | | | (d) | begining of | tail rac | ee |
| | | | | | | | CWE | DC. | | | |

: ANSWERS

| Γ | 1. | (b) | 2. | (a) | 3. (c) | 4. | (c) | 5. | (b) | |
|---|----|-----|----|-----|--------|----|-----|----|-----|--|
| | 6. | (a) | 7. | (b) | | | | · | | |

<u>SET-6</u>

| | • | | | | |
|----|--------------------|----------------------|---------|-------------|-------------------------|
| 1. | The earth embank | ments constructed p | arallel | to the rive | r for flood protection, |
| | are called: | | | | |
| | (a) Guide banks | (b) Levees | (c) | Terraces | (d) Groynes |
| • | TEL | l flood control meas | ure inc | luded below | is |
| 2. | The only structura | | () | C +-ff-(4) | Flood plain zoning |
| | (a) Dikes | (b) Terraces | (c) | Cutoris (a) | Flood plain zoning |
| 3. | Floodways are | | | | |
| | (a) embankments | | (b) | Concrete w | alls |
| | ` ' | | (4) | Low lying | areas |
| | (c) Ponds | | (a) | TOW IAILE | areas |

SET-7

| ı. | in Dicken's en | ipirical formula, Q = | C.A" where, n is | |
|----|----------------|-----------------------|----------------------|---------------------------------|
| | (a) 0.25 | (b) 0.50 | (c) 0.67 | (d) 0.75 |
| 2. | Ryve's formul | a, to predict maximu | m flood discharge. Q | = C.A ⁿ , where n is |
| | (a) 0.25 | (b) 0.50 | (c) 0.67 | (d) 0.75 |

| 3. | If the frequency of flood (T) | having a ran | k m and, total | record of n | years, is |
|----|--|---------------|----------------|-------------|-----------|
| | given by $T = \frac{n+1}{m}$ then this r | elation is kn | own as | | |

(a) California formula

(b) Weibul formula

(c) Hazen formula

- (d) Beard formula flood
- The probability that a T year flood frequency occurs in any given year is 4.
- (b) $\frac{1}{T}$
- (c) log_eT
- (d) e^{-T}
- According California formula, the return period is 5.
 - (a) $T = \frac{n}{m}$
- (b) $T = \frac{2n}{2m-1}$
- (c) $T = \frac{n+1}{m}$ (d) $T = \frac{n}{m+c-1}$
- The most commonly used formula for computing return period is 6.
 - (a) California formula

(b) Weibul formula

(c) Hazen formula

- (d) Gumbel's method
- The governing equation given by Ven Te Chow is 7.
 - (a) $X = \overline{X} + K\sigma$

(b) $X = \overline{X} + \frac{K}{2} \sigma$

(c) $X = \overline{X} + K^n \cdot \sigma$

- $X = \overline{X} + 2K \cdot \sigma$
- The rational formula is restricted to the catchment area of size less than 8.
 - (a) 50 ha
- (b) 500 ha

(d)

- (c) 5000 ha
- (d) 50,000 ha
- The most commonly adopted probability distribution to fit the flood data is 9.
 - (a) Normal distribution

- (b) Gumbel's extreme value distribution
- (c) Log-Normal distribution
- (d) Log-Pearson distribution
- The continuity equation used for flood routing is
 - (a) $I + O = \Delta S$
- (b) $O I = \Delta S$
- $(c) I O = \Delta S$
- (d) $O + I = \Delta S$

: ANSWERS:

| 1 ' 1 | | | 4. (b) | 5. (a) |
|--------|--------|--------|--------|---------|
| 6. (b) | 7. (a) | 8. (c) | 9. (b) | 10. (c) |

SET-8

Multi-purpose project is the one which is constructed to: 1.

- (a) be used in combination of other reservoirs
- (b) Supply water for more than one year
- (c) Serve more than one purpose
- (d) Serve one main purpose always

Objective Civil Eng. \ 2016 \ 45

Objective Civil Engineering

: ANSWERS:

| 1. (b) 6. (b) 11. (b) | 2. (b) 7. (c) - 12. (d) 17. (d) | 3. (c) 8. (c) 13. (d) 18. (a) | 4. (c) 9. (a) 14. (b) 19. (d) | 5. (d) 10. (c) 15. (c) 20. (e) |
|-----------------------------|--|--|--|---|
| 16. (b) 21. (c) | 17. (d) 22. (b) | 18. (a) ' | 19. (d) | 20. (c) |

EXPLANATIONS

(c) Leaching requirement % 3.

$$= \frac{D_i - C_u}{D_i} \times 100 \% \qquad C_u = consumptive use$$

$$C_{ij} = consumptive use$$

$$10 = \frac{\left(D_i - 90\right)}{D_i} \times 100$$

$$\therefore D_i = 100 \text{ mm}$$

Lacey's silt factor

=
$$1.76 \sqrt{m_r}$$

= $1.76 \sqrt{0.16} = 0.70$

6. **(b)**
$$p = 1 - \left(1 - \frac{1}{T}\right)^n$$

$$= 1 - \left(1 - \frac{1}{10}\right)^4 = 1 - (0.9)^4$$

14. (b)
$$\phi = \frac{P - R}{t_r}$$

$$P = 0.5 + 2.8 + 1.6$$

$$=\frac{4.9-3.2}{6}$$

$$= 4.9 \text{ cm}$$

$$= 2.83$$
 cm/hr

$$t_r = 2 + 2 + 2$$

17. (d) Maxi. intensity for 20 min duration

$$= 2.2 + 1.5 = 3.7 \text{ mm/min}$$

359

Hydrology and Water Resources Engineering

19. (d) For unit hydrograph rainfall depth = 1 cm
=
$$1 \times 10^{-2}$$
 m
total vol. of water = Area under unit hydrograph
= $60 \times 60 \times 1 \times (0 + 2 + 6 + 4 + 2 + 1 + 0)$
= 54000 m³

$$\therefore \text{ Area of catchment} = \frac{\text{volume}}{\text{depth}} = \frac{54000}{1 \times 10^{-2}} = 5.4 \times 10^6 \text{ m}^2$$
$$= 5.4 \text{ km}^2$$

20. (c) For 1h unit hydrograph,
rainfall depth = 1 cm = 1 × 10⁻² m
Total quantity of rainfall = area of unit hydrograph

$$= \frac{1}{2} \times 60 \times 30 \times 60 \times 60$$
$$= 3.24 \times 10^6 \text{ m}^3$$

 $\therefore \text{ Area of water shed} = \frac{\text{Volume}}{\text{depth}}$

$$= \frac{3.24 \times 10^6}{1 \times 10^{-2}}$$

$$= 3.24 \times 10^8 \text{ m}^2$$

$$= 3.24 \times 10^2 \text{ km}^2 = 324 \text{ km}^2$$

21. (c)
$$\eta_c = \frac{0.8}{1.0} \times 100 = 80 \%$$

22. (b) Equilibrium discharge

$$q_e = 2.778 \frac{A}{D} = 2.778 \times \frac{540}{6}$$

= 250 m³/s



Surveying

MCQ'S

SET-1: Introduction

| _ | | | |
|----|--|----------|---|
| 1. | The main principle of surveying i | s to wo | ork from |
| | (a) The centre to the boundary | (b) | The whole to the part |
| | (c) The part to the whole | (d) | Higher to lower level |
| | 2. The curvature of the earth is | s ignore | ed in |
| | (a) Geodetic surveying | (b) | Plane surveying |
| | (c) Hydrographic surveying | (d) | Aerial survey |
| 3. | The curvature of the earth is take | n into : | account when the extent of area is mor |
| | than | | |
| | (a) 50 km^2 (b) 100 km^2 | (c) | 250 km^2 (d) 350 km^2 |
| 4. | If scale of map is 1 cm = 50 m, F | R.F. is | |
| | | No. | 1 (1) |
| | (a) $\frac{1}{50}$ (b) $\frac{1}{500}$ | (c) | $\frac{1}{5000}$ (d) $\frac{1}{50,000}$ |
| 5. | | ures lik | ce mountains, valleys, rivers, forests, etc |
| | are known as | | |
| | (a) cadastal surveys | (b) | Topographical surveys |
| | (c) Engineering surveys | (d) | Mine surveys |
| 6. | The plain scale is used to read | | • |
| | (a) One unit | (b) | Two units |
| | (c) Three consecutive units | (d) | None of above |
| 7. | The diagonal scale is used to read | l | * (|
| | (a) One unit | (b) | Two units |
| | (c) Three consecutive units | (d) | None of above |
| 8. | Vernier scale is used to measure | | |
| | (a) Fractional part of a graduated s | scale | |
| | (b) Fractional part of a plain scale | | |
| | (c) Fractional part of a diagonal so | cale | |
| | (d) None of above | | |
| | | | |

| 9. | The branch of surveying which deals with the measurements in a vertical plane is |
|-----|--|
| | known as (a) Plane tabling (b) Levelling (c) Traversing (d) None of above |
| | (a) Traile tability (b) Developed |
| 10. | In surveying the measurements are taken in (a) Horizontal plane (b) Vertical plane |
| | (a) Horizontal plant |
| | (c) memora plane |
| 11. | The object of surveying is to prepare a (a) Drawing (b) Cross-section (c) Sketch (d) Map |
| | (a) Drawing (b) Cross-section (c) Sketch (d) Map |
| | : ANSWERS : |
| | 1. (b) 2. (b) 3. (c) 4. (c) 5. (b) |
| | 6. (b) 7. (c) 8. (a) 9. (b) 10. (a) |
| | 11. (d) |
| L— | SET-2: Linear Measurement |
| - | SE1-2. Linear Measurement |
| 1. | A 20 m chain is divided into |
| | (a) 60 links (b) 100 links (c) 150 links (d) 200 links |
| 2. | The length of Gunter's chain is (a) 100 ft (b) 50 ft (c) 66 ft (d) 75 ft |
| | (a) 100 It (b) 30 It |
| 3. | The chainman who drags the chain is called the (a) Contain (b) Leader (c) follower (d) Labour |
| | (a) Captain (b) Leader |
| 4. | In chain survey the area is divided into (a) Rectangles (b) Triangles (c) Squares (d) Circles |
| | (a) Rectangles (b) Triangles |
| 5. | Chain survey is recommended when the area is |
| | (a) Crowded (b) Undulating (c) Simple and fairly level. |
| 6. | Cross staff is used for (a) Setting out right angles (b) Measuring horizontal angles |
| | (a) Betting out right angul |
| - | (c) Both (a) and (b) (d) None of above For ranging a line, the number of ranging rods required is |
| 7. | (1) At 1 Atlant four |
| o | (a) Atleast two (b) At least three (c) Atleast four Correction for slope is given by |
| 8. | (a) $h^2/2L$ (b) h/L (c) $h/2L$ (d) $2h^2/L$ |
| 9. | In an optical square, the mirrors are fixed at an angle of |
| ٠. | (a) 30° (b) 60° (c) 45° |
| 10 | Company of the Compan |
| | (a) additive (b) Substractive (c) Zero |
| Ot | ojective Civil Eng. \ 2016 \ 46 |

Objective Civil Engineering 362 11. If θ be the angle of slope and l be the sloping distance, slope correction is given by (a) $l(1-\sin\theta)$ (b) $l(1-\cos\theta)$ (c) $l(1-\sec\theta)$ 12. Which one is used for the measurement of base line? (c) Engineer's chain (b) Steel tape (a) Invar tape 13. Which of the following is an obstacle to chaining but not to ranging? (d) River (c) Forest (b) Building (a) Hill : ANSWERS: 7. (b) (a) 6. 5. (c) (b) (b) 4. 3. (c) 2. (b) 1. 13. (d) 12. (a) 11. (b) 10. (b) 9. (c) 8. (a) **SET-3: Compass Survey** In a prismatic compass, the zero is marked on the (d) West end (c) East end (b) South end (a) North end A triangle is said to be well conditioned when its angles lie between (d) 45° and 90° (a) 30° and 120° (b) 20° and 150° (c) 15° and 135° In a surveyor's compass, the ring is graduated 3. (b) From 0° to 180° (a) From 0° to 360° (d) None (c) In quadrants 0° to 90° The angle of dip at a point on equator is 90° (d) (c) 30° (b) 45° (a) 0°

In the WCB system, a line is said to be free from local attraction if the difference between the FB and BB is

(a) 0°

(b) 90°

(c) 180°

(d) 360°

A line joining points of zero declination is called 6.

(a) agonic line

(b) isogonic line (c) isoclinic line

(d) Survey line

The angular error in a closed traverse should not exceed 7.

(a) $15\sqrt{N}$ min

(b) $30\sqrt{N}$ min

(c) $10\sqrt{N}$ min

(d) \sqrt{N} min

The horizontal angle between the true meridian and the magnetic meridian is called

(a) Dip

(b) Azimuth

Declination (c)

(d) None of above

If FB of a line is zero degree, its BB is 9.

(b) 90°

180°

(d) 360°

10. The BB of a line is S 30° W, its FB is

(a) N 30° E

(b) N 30° W

(c) S 30° W

(d) S 30° E

(c) 30°

(b) 45°

8.

(a) 90°

(d) 60°

| <u> </u> | The | vertical dista | nce l | netween two | diace | nt contour line | e ie co | lled a |
|------------|------|--|--------|-----------------|---------|-------------------------|----------|--------------|
| <i>)</i> . | | • | | | - | alent (| | |
| 10. | In a | ı contour map | , who | en lower valu | es are | inside the loo | p, it in | dicates |
| | (a) | Level ground | (b) | Depression | (c) | Hill | (d) | Ridge |
| 11. | A fi | xed point of r | efere | ence, whose el | evatio | on is known is | called | |
| | (a) | Station | (b) | Bench mark | (c) | reduced level | (d) | Change point |
| 12. | The | height of inst | trum | ent (HI) in le | velling | g is the | | |
| | (a) | Elevation of l | ine o | f sight with re | spect | to a datum | | |
| | (b) | Elevation of line of sight with respect to MSL | | | | | | |
| | (c) | Height of tele | scope | e axis above tl | ne gro | und | | |
| | (d) | None of abov | е | | | | | |
| 13. | Wh | en contours of | f diff | erent elevatio | ns cre | oss each other | it indi | eates |
| | (a) | Vertical cliff | (b) | Saddle | (c) | Overhanging of | liff | |
| 14. | The | surface of sti | II wa | ter is conside | red to | o be | | |
| | (a) | Level | (b) | Horizontal | (c) | Smooth | (d) | Curved |
| 15. | The | correction fo | r cur | vature of ear | th is | giv <mark>e</mark> n by | | · |
| | (a) | $0.0785 d^2$ | (b) | $0.01122 d^2$ | (c) | $0.0673 d^2$ | | |
| | | | | : AN | SWE | RS: | | |
| | | | | | | | | |

| | | <u> </u> | | | \neg |
|---------|---------|----------|----------|---------|--------|
| 1. (b) | 2. (d) | 3. (c) | 7 4. (b) | 5. (a) | |
| 6. (b) | 7. (b) | 8. (a) | 9. (c) | 10. (b) | |
| 11. (b) | 12. (a) | 13. (c) | 14. (a) | 15. (a) | |
| | | | | | |

SET-5: Plane Table Survey

| 1. | rep | resenting a di | _ | ng the plane tal on on the plan is | | | | |
|----|------------|----------------|-------|---------------------------------------|-------|--------------|----------|-------------------|
| | call | | | | | | | |
| | (a) | Centering | (b) | Orientation | (c) | Levelling | (d |) Resection |
| 2. | In s | etting up a pl | lane | table, the operati | on w | hich is done | first is | • |
| | (a) | Levelling | (b) | Centering | (c) | Orientation | (d |) Resection |
| 3. | The | principle of | plane | tabling is | | | ٠ | |
| | (a) | Traversing | (b) | Triangulation | (c) | Parallelism | (d |) None of above |
| 4. | Wh tabl | | owin | g instrument is v | sed : | for marking | north d | irection in plane |
| | (a) | Spirit level | (b) | Trough Compass | (c) | u–fork | (d |) alidade |

| 5. | Inaccessible points may be located by | | | | | | |
|-----|--|---|--|--|--|--|--|
| | (a) Radiation (b) Intersection | (c) Traversing (d) Resection | | | | | |
| 6. | Two point and three point problems a | re methods of | | | | | |
| | (a) Resection only | (b) Orientation only | | | | | |
| | (c) Resection and orientation | (d) Traversing | | | | | |
| 7. | The two point problem as compared t | o three point problem is | | | | | |
| | (a) more accurate (b) quicker | (c) more laborious (d) all of above | | | | | |
| 8. | Three point problem can be solved by | | | | | | |
| | (a) Lehmann's method | (b) Bessel's method | | | | | |
| Pr | (c) Tracing Paper method | (d) All the above | | | | | |
| 9. | The working edge of the alidade is kn | | | | | | |
| | | (c) Drawing edge (d) Straight edge | | | | | |
| 10. | The u-fork and plumb bob are requir | | | | | | |
| | | (c) Orientation (d) Bisecting | | | | | |
| 11. | $\vee (C)_{\overline{i}}$ | | | | | | |
| | (a) By traversing | (b) By magnetic needle | | | | | |
| | (c) by backsighting | (d) None of above | | | | | |
| 12. | | ane table surveying for determining the | | | | | |
| | horizontal distances without tape or c | | | | | | |
| | (a) Plane alidade | (b) Telescopic alidade (d) Clinometer | | | | | |
| 12 | (c) Tacheometer | • | | | | | |
| 13. | the method is called | y drawing rays from the station to objects. | | | | | |
| | (a) Radiation (b) Intersection | (c) Traversing (d) Resection | | | | | |
| 14 | | ed position of the station occupied by the | | | | | |
| 14. | | towards points of known location is called | | | | | |
| | (a) Resection (b) Intersection | (c) Orientation (d) None of the above | | | | | |
| | (a) 1000011011 | | | | | | |
| | : ANSV | VERS: | | | | | |
| | 1. (b) 2. (a) 3. | (c) 4. (b) 5. (b) | | | | | |
| | 6. (c) 7. (c) 8. | (d) 9. (a) 10. (b) | | | | | |
| | 11. (c) 12. (b) 13. | (a) 14. (a) | | | | | |

SET-6: Theodolite Traversing

| 1. | Turning the telescope in the vertical plane about the horizontal axis is called |
|-----------|---|
| | (a) Transiting (b) Plunging (c) Swinging (d) both (a) and (b) |
| 2. | The operation of revolving the telescope in the horizontal plane about its vertical |
| | axis is called |
| | (a) Swinging (b) Transiting (c) Plunging (d) None of the above |
| 3. | Approximate bisection in a theodolite is done by the |
| | (a) Focussing screw (b) Tangent screw (c) Clamp screw (d) Foot screw |
| 4. | Fine adjustment in a theodolite is done by the |
| | (a) Clamp screw (b) Tangent screw (c) Focussing screw (d) Foot screw |
| 5. | The upper plate of a theodolite is fixed to |
| | (a) outer spindle (b) Inner spindle (c) Levelling head (d) None of above |
| 6. | For improved accuracy in the measurement of horizontal angle the method used is |
| | (a) General method (b) Reiteration method |
| | (c) Repetition method (d) Vernier method |
| 1 | If N is the number of sides in a closed traverse, then the sum of interior angles |
| | should be equal to |
| | (a) $(2N + 4) \times 90^{\circ}$ (b) $(2N \times 4) \times 90^{\circ}$ (c) $(2N - 4) \times 90^{\circ}$ (d) $(N - 4) \times 90^{\circ}$ |
| 8. | The direction of closing error is given by |
| | (a) $\tan \theta = \frac{\Sigma D}{\Sigma I}$ (b) $\tan \theta = \frac{\Sigma L}{\Sigma D}$ |
| | (a) $\tan \theta = \frac{\sum D}{\sum L}$ (b) $\tan \theta = \frac{\sum L}{\sum D}$ |
| | (c) $\tan \theta = \Sigma L \times \Sigma D$ (d) $\sin \theta = \frac{\Sigma D}{\Sigma L}$ |
| | |
| 9. | If θ be the RB of a line of length 'l' then the latitude of a line is given by |
| | (a) $l \sin \theta$ (b) $l \cos \theta$ (c) $l \tan \theta$ (d) $l \cot \theta$ |
| 10. | If θ be the RB of a line of length 'l' then the departure of a line is given by |
| | (a) $l \sin \theta$ (b) $l \cos \theta$ (c) $l \tan \theta$ (d) $l \cot \theta$ |
| 11. | While taking a backsight, the screw used is |
| | (a) Lower clamp (b) Upper clamp (c) Upper tangent (d) None of above |
| 12. | The Parallax can be removed by |
| | (a) Focussing the objective (b) Focussing the eye piece |
| | (c) Focussing both the eye piece and objective (d) None of above |
| 13. | - a a a little in Justice and by |
| | (a) The length of the telescope |
| | (b) The diameter of the telescope |
| | · |

368-

- 24. A theodolite in which the telescope can be revolved through a complete revolution in a vertical plane is known as a
 - (a) Vernier theodolite

- (b) Tilting theodolite
- (c) Non-transit theodolite
- (d) Transit theodolite
- 25. When the vertical circle is to the right side of the observer, it is called
 - Telescope inverted

(b) Telescope normal

Telescope reversed

(d) none of above

: ANSWERS:

| 1. (d) 6. (c) | 2. (a) 7. (c) | 3. (c) 8. (a) | 4. (b) 9. (b) | 5. (b) 10. (a) |
|------------------|------------------|------------------|------------------|-------------------|
| 11. (a) | 12. (c) | 13. (d) | 14. (b) | 15. (c) |
| 16. (d) | 17. (c) | 18. (c) | 19. (a) | 20. (c) |
| 21. (a) | 22. (a) | 23. (c) | 24. (d) | 25. (a) |

SET-7: Trigonometric Levelling

- Which corrections are neglected in plane trigonometric levelling?

 - (a) earth's curvature (b) refraction (c) both (a) and (b)
- (d) None of above
- The combined correction for earth's curvature and refraction in linear measurement is given by
 - (a) $0.0673 D^2$
- (b) $0.0785 D^2$
- (c) $0.0112 D^2$
- (d) None of above
- When the base of the objectis accessible, the horizontal distance between the instrument and object is D, the elevation h is given by
 - (a) D sin α
- (b) D cos α
- (c) D tan α
- (d) D cot a
- When the base of the object is inaccessible, the instrument stations in the same vertical plane and instrument axes at the same level, the distance between object and the station nearer to object is

$$\text{(a)} \ \frac{b \tan \alpha_1}{\left(\tan \alpha_1 - \tan \alpha_2\right)} \ \text{(b)} \frac{b \tan \alpha_2}{\left(\tan \alpha_1 - \tan \alpha_2\right)} \ \text{(c)} \frac{b \tan \alpha_1}{\left(\tan \alpha_2 - \tan \alpha_1\right)} \ \text{(d)} \ \frac{b \tan \alpha_2}{\left(\tan \alpha_2 - \tan \alpha_1\right)}$$

For angle of elevation, the correction for earth curvature is

(a)
$$+\frac{\theta}{2}$$
 (b) $-\frac{\theta}{2}$

 $(d) - 2\theta$

: ANSWERS:

| 1. (c) | 2. (a) | 3. (c) | 4. (b) | 5. (a) |
|--------|--------|--------|--------|--------|

SET-8: Curves

| 1. | A circular curve is most suited for co | nnecting | |
|------|---|-------------------------|-------------------------------|
| | (a) Two straights in horizontal plane of | only | |
| | (b) Two straights in vertical plane only | y | |
| | (c) Two straights, one in horizontal pl | ane and the other in ve | ertical plane |
| | (d) Two straights in horizontal plane of | | • |
| 2. | The shortest distance between the point | nt of curve (P.C.) and | the point of tangency |
| | (P.T.) is | | · . |
| | (a) Normal chord (b) Long chord | (c) Sub chord | (d) Half chord |
| 3. | The curve preferred for vertical curve | e is a | |
| | (a) circular (b) spiral | (c) parabola | (d) hyperbola |
| 4. | The degree of a circular curve of rad | ius 1719 m is approxi | mately equal to |
| | (a) 100° (b) 10° | (c) 1° (d) | None of the above |
| 5. | The radius of a 1° curve is (length 30 | (m) | |
| | (a) 1719 m (b) 1917 m | (c) 1918 m | (d) 1819 m |
| 6. | For an ideal transition curve, the rela | ation between radius | r and distance l from |
| | begining is | 1 | 1 |
| _ | (a) $l \alpha r$ (b) $l \alpha r^2$ | | (d) $l \propto \frac{1}{r^2}$ |
| 7. | The length of transition curve is given | | |
| | (a) $L = \frac{n}{e}$ (b) $L = ne$ | (c) $L = \frac{e}{n}$ | (d) $L = n - e$ |
| 8. | The length of long chord and tangent | length of a circular of | curve of radius R will |
| | be equal if the deflection angle is | | |
| | (a) 60° (b) 30° | (c) 150° | (d) 120° |
| 9. | The tangent length of a simple circula | r curve of radius R a | nd deflection angle Δ |
| | is given by | | |
| | (a) R tan $\frac{\Delta}{2}$ (b) R tan Δ | $\frac{R}{R}$ | R |
| 4.0 | (0) R tall 2 | $^{(c)}$ tan Δ | $\tan \frac{\Delta}{\Delta}$ |
| 10. | (a) R tan $\frac{\Delta}{2}$ (b) R tan Δ For a transition curve, the shift of a c | ircular curve is given | by and 2 |
| | (a) $\frac{L}{24R}$ (b) $\frac{L^2}{24R}$ | (c) $\frac{L^3}{L^3}$ | (d) $\frac{L^3}{24R^2}$ |
| | 24R 24R | ^(c) 24R | $24R^2$ |
| 11. | Apex distance is also called | | |
| | (a) Mid ordinate (b) Normal chord | (c) External distanc | e (d) tangent length |
| 12. | | | |
| | (a) $\frac{P}{W}$ (b) $\frac{W}{P}$ | (c) P × W | (d) P – W |
| | W Y P | | (-) - · · · |
| Obie | ctive Civil Eng. \ 2016 \ 47 | | |

| 3 | 7 | O |
|---|---|---|
| _ | , | · |

| 13. | The long chord of a circular curve of ra | dius R and deflection | angle ∆ is given by |
|-----|---|--|--------------------------------|
| | (a) $2R \sin \frac{\Delta}{2}$ (b) $2R \cos \frac{\Delta}{2}$ | (c) $2R \tan \frac{\Delta}{2}$ | (d) $2R \sec \frac{\Delta}{2}$ |
| 14. | The deflection angle from the tangent is | given by | • |
| | (a) $\delta = 1718.9 \frac{R}{C}$ minutes | (b) $\delta = 1817.9 \frac{R}{C} \text{ m}$ (d) $\delta = 1817.9 \frac{C}{R} \text{ m}$ | inutes |
| | (c) $\delta = 1718.9 \frac{C}{R}$ minutes | (d) $\delta = 1817.9 \frac{C}{R} \text{ m}$ | inutes |
| 15. | The superelevation required on curve is | | |
| | (a) $e = \frac{bv^2}{gR}$ (b) $e = \frac{b \cdot R}{gv^2}$ | (c) $e = \frac{bv}{gR^2}$ | (d) $e = \frac{b^2 v^2}{gR}$ |
| 16. | If the chainage of point of tangency (P.7 | Γ.) of a circular curve | for a normal chord |
| | of 20 m is 2303.39 m, the length of the | last subchord is | |
| | (a) 16.61 m (b) 3.39 m | (c) 23.39 m (d) | |
| 17. | If the chainage of point of curve of a sir | | r a normal chord of |
| | 20 m is 2002.48 m the length of the firs | st sub-chord is | |
| | (a) 2.48 m (b) 17.52 m | (c) 20 m | (d) 22.48 m |
| 18. | Overturning of vehicles on a curve can | | |
| | (a) Reverse curve (b) Compound curve | (c) transition curve | (d) Vertical curve |
| 19. | A parabola is used for | | |
| | (a) Summit curves only | (b) Sag curves only | |
| | | (d) None of the abov | |
| 20. | Reverse curve is preferred on high way | | |
| | (a) Low speed (b) High speed (c) | Both (a) & (b) (d) | None of the above |
| 21. | The degree of curve for 20 m arc lengt | h is given by | |
| | (a) $D = \frac{1719}{R}$ (b) $D = \frac{1146}{R}$ | (c) $D = \frac{1917}{1917}$ | (d) D = $\frac{1416}{1}$ |
| | | | (d) B |
| 22. | The radius of a 1° curve for 20 m arc | | |
| | (a) 1146 m (b) 1416 m | (c) 1719 m | (d) 1917 m |
| 23. | The radius of a 4° curve (30 m arc length | | |
| | (a) 719 m (b) 1146 m | (c) 430 m | (d) 287 m |
| 24. | To avoid inconvenience to passengers | on highways the rec | ommended value of |
| | centrifugal ratio is | | |
| | (a) 1 (b) 1/2 | (c) 1/4 | (d) 1/8 |
| 25. | If an upgrade is 2% is followed by a c | | |
| | grade is 0.4% per 100 m, the length of | | |
| | (a) 200 m (b) 100 m | (c) 400 m | (d) 1000 m |

- 26. The following curve has a property that the rate of change of curvature is same as the rate of change of increase in superelevation.
 - (a) Reverse curve (b) Transition curve (c) Compound curve (d) Vertical curve
- 27. The most widely used transition curve for small deflection angles for simplicity in setting out is
 - (a) Cubic parabola
- (b) Lemniscate
- (c) Cubic spiral
- (d) hyperbola

28. The offset from long chord is given by

(a)
$$O_x = \sqrt{R^2 - x^2} - (R - O_0)$$

(b)
$$O_x = \sqrt{R^2 + x^2} - (R - O_0)$$

(c)
$$O_x = \sqrt{R - x^2} + (R - O_0)$$

(d)
$$O_r = \sqrt{R^2 - x^2} + (R + O_0)$$

29. Radial offsets from tangent is given by

(a)
$$O_x = \sqrt{R^2 - x^2} - R$$

(b)
$$O_x = \sqrt{R^2 - x^2} + R$$

(c)
$$O_x = \sqrt{R^2 + x^2} - R$$

(d)
$$O_x = \sqrt{R^2 + \chi^2} + R$$

30. Perpendicular offsets from tangent is given by

(a)
$$O_x = R - \sqrt{R^2 + x^2}$$

(b)
$$O_x = R - \sqrt{R^2 - x^2}$$

(c)
$$O_x = R + \sqrt{R^2 + x^2}$$

(d)
$$O_x = R + \sqrt{R^2 - x^2}$$

: ANSWERS:

| 1. (a) | 2. (b) | 3. (c) | 4. (c) | 5. (a) |
|---------|---------|---------|---------|---------|
| 6. (c) | 7. (b) | 8. (d) | 9. (a) | 10. (b) |
| 11. (c) | 12. (d) | 13. (a) | 14. (c) | 15. (a) |
| 16. (b) | 17. (b) | 18. (c) | 19. (c) | 20. (a) |
| 21. (b) | 22. (a) | 23. (c) | 24. (c) | 25. (d) |
| 26. (b) | 27. (a) | 28. (a) | 29. (c) | 30. (b) |

SET-9: Area and Volume

- The area of zero circle is equal to
 - (a) M
- (c) $M \times C$
- (d) M C
- Simpson's rule can be used for computations of areas when the number of ordinates is
 - (a) even
- (b) odd
- (c) any number
- (d) 3
- The area of any irregular figure can be determined accurately by 3.
- (b) planimeter
- (c) cross-staff (d) Prismatic compass
- 4. The area of a parabolic segment of base b and height h is given by
 - (a) $\frac{1}{2}$ bh
- (b) $\frac{2}{3}$ bh
- (c) $\frac{3}{4}$ bh
- (d) $\frac{1}{2}bh$

| ~ | 7 | ~ |
|---|---|---|
| 3 | , | Z |

| • | | | | | |
|---|---|--|---|--|--|
| | Prismoidal cor | rection is requ | ied to correc | t the volume c | alculated |
| | (a) Using conte | ours | (b) | Using spot leve | els |
| | (c) for a curve | d section | (d) | by end-areas ru | ıle` |
| | In Simpson's ru | ule the line join | ing the top o | f the ordinates i | is considered to |
| | (a) sraight | (b) ellip | otical (c) | parabolic | (d) curved |
| | In trapezoidal | rule, the line jo | ining the top | of the ordinates | s is considered to be |
| | (a) straight | (b) ellip | otical (c) | parabolic | (d) curved |
| | When the anch | or point is out | t the figure t | he area of zero | circle is |
| | (a) Added | (b) Subs | stracted (c) | taken zero | (d) None of above |
| | When the traci | ing point is mo | ved along a c | ircle without re | otation of the wheel th |
| | the circle is kn | own as | | | |
| | (a) Zero circle | (b) Prin | ne circle (c) | circum circle | (d) ortho circle |
| | In trapezoidal | formula, the n | umber of or | dinates must be | • |
| | (a) odd | | (b) | even | |
| | (c) either odd o | or even | (d) | None of the ab | ove |
| | | | | | |
| | | | : ANSWE | RS: | |
| | 1. (c) | 2. (b) | 3. (b) | 4 (1) | 5 (4) |
| | | 2, (0) | J. (b) | 4. (b) | 5. (d) |
| | 6. (c) | 7. (b) | 8. (c) | 9. (a) | 10. (c) |
| | 6. (c) For navigation | 7. (b) SET-10: purposes, gen | 8. (c) Hydrogr erally the da | 9. (a) aphic Surv tum used is | 10. (c) |
| | 6. (c) For navigation (a) M.S.L. | 7. (b) SET-10: purposes, gen (b) T.B. | Hydrogr erally the da .M. (c) | 9. (a) aphic Surv | 10. (c) |
| | 6. (c) For navigation (a) M.S.L. Sounding in hy | 7. (b) SET-10: purposes, gen (b) T.B. ydrography me | 8. (c) Hydrogr erally the da .M. (c) eans | 9. (a) aphic Surv tum used is G.T.S. | 10. (c) |
| | For navigation (a) M.S.L. Sounding in hy (a) distance tra | 7. (b) SET-10: purposes, gen (b) T.B. ydrography me | Hydrogr erally the da .M. (c) eans d waves in w | 9. (a) aphic Survetum used is G.T.S. ater body | 10. (c) |
| | For navigation (a) M.S.L. Sounding in hy (a) distance tra (b) depth of w | 7. (b) SET-10: purposes, gen (b) T.B. ydrography meavelled by soundater body at the | Hydrogr erally the da .M. (c) eans d waves in w | 9. (a) aphic Survetum used is G.T.S. ater body | 10. (c) |
| | For navigation (a) M.S.L. Sounding in hy (a) distance tra (b) depth of w (c) Velocity of | 7. (b) SET-10: purposes, gen (b) T.B. ydrography me avelled by soundater body at the water waves | Hydrogr erally the da .M. (c) eans d waves in w | 9. (a) aphic Survetum used is G.T.S. ater body | 10. (c) |
| | For navigation (a) M.S.L. Sounding in hy (a) distance tra (b) depth of w (c) Velocity of (d) None of th | 7. (b) SET-10: purposes, gen (b) T.B. ydrography me avelled by soundater body at the f water waves e above | Hydrogr erally the da .M. (c) eans d waves in we point of mea | 9. (a) aphic Survetum used is G.T.S. ater body asurement | (d) Tidal datum |
| | For navigation (a) M.S.L. Sounding in hy (a) distance tra (b) depth of w (c) Velocity of (d) None of th The observation | 7. (b) SET-10: purposes, gen (b) T.B. ydrography me avelled by soundater body at the f water waves e above ons to establish | Hydrogr erally the da .M. (c) eans d waves in was point of means | 9. (a) aphic Survetum used is G.T.S. ater body assurement | (d) Tidal datum |
| | For navigation (a) M.S.L. Sounding in hy (a) distance tra (b) depth of w (c) Velocity of (d) None of th The observation (a) 9 years | 7. (b) SET-10: purposes, gen (b) T.B. ydrography me avelled by soundater body at the f water waves e above ons to establish (b) 19 y | Hydrogr erally the da .M. (c) eans d waves in we point of means M.S.L. are | 9. (a) aphic Surv tum used is G.T.S. ater body asurement recorded for all 29 years | (d) Tidal datum oout (d) 39 years |
| | For navigation (a) M.S.L. Sounding in hy (a) distance tra (b) depth of w (c) Velocity of (d) None of th The observation (a) 9 years In ocean where | 7. (b) SET-10: purposes, gen (b) T.B. ydrography me avelled by soundater body at the f water waves e above ons to establish (b) 19 y e depth of wat | Hydrogr erally the da .M. (c) eans d waves in was e point of means (c) erally the da .M. (c) eans (c) erans (d) erans (d) erans (e) erans (e) erans (f) | 9. (a) aphic Surv tum used is G.T.S. ater body asurement recorded for all 29 years he instrument | (d) Tidal datum out (d) 39 years used is |
| | For navigation (a) M.S.L. Sounding in hy (a) distance tra (b) depth of w (c) Velocity of (d) None of th The observation (a) 9 years In ocean where (a) Sounding p | 7. (b) SET-10: purposes, gen (b) T.B. ydrography me avelled by sound atter body at the f water waves e above ons to establish (b) 19 y e depth of wat ooles (b) lead | Hydrogr erally the da .M. (c) ans d waves in w e point of mea | 9. (a) aphic Surv tum used is G.T.S. ater body asurement recorded for all 29 years he instrument Fathometer (d) | (d) Tidal datum out (d) 39 years used is Sounding machine |
| | For navigation (a) M.S.L. Sounding in hy (a) distance tra (b) depth of w (c) Velocity of (d) None of th The observation (a) 9 years In ocean where (a) Sounding p The branch of | 7. (b) SET-10: purposes, gen (b) T.B. ydrography me avelled by sound atter body at the f water waves e above ons to establish (b) 19 y e depth of wat ooles (b) lead | Hydrogr erally the da .M. (c) ans d waves in w e point of mea | 9. (a) aphic Surv tum used is G.T.S. ater body asurement recorded for all 29 years he instrument Fathometer (d) | (d) Tidal datum out (d) 39 years used is |
| | For navigation (a) M.S.L. Sounding in hy (a) distance tra (b) depth of w (c) Velocity of (d) None of th The observation (a) 9 years In ocean where (a) Sounding p | 7. (b) SET-10: purposes, gen (b) T.B. ydrography me avelled by sound atter body at the f water waves e above ons to establish (b) 19 y e depth of wat soles (b) lead surveying whi | Hydrogr erally the da .M. (c) ans d waves in w e point of mea M.S.L. are years (c) er is great, t lines (c) ch deals with | 9. (a) aphic Surv tum used is G.T.S. ater body asurement recorded for all 29 years he instrument Fathometer (d) a the measurem | (d) Tidal datum out (d) 39 years used is Sounding machine |

- 6. In hydrographic surveying
 - (a) Only horizontal control is required
 - (b) Only vertical control is required
 - (c) Both horizontal and vertical controls are required.
 - (d) No such controls are required.
- 7. The purpose of soundings is
 - (a) To determine sea bed profile
 - (b) Preparation of navigation charts
 - (c) to locate areas from where material can be dredged
 - (d) all of the above
- 8. The purpose of hydrographic survey is
 - (a) To make nantical charts for navigation
- (b) To establish M.S.L.

(c) To determine shore lines

(d) all of the above

: ANSWERS:

| 1. (d) | 2. (b) | 3. (b) | 4. (c) | |
|--------|--------|--------|--------|--|
| 5. (d) | 6. (c) | 7. (d) | 8. (d) | |

SET-11: Setting out Works

- 1. The accessory used in setting out works is
 - (a) boning rod
- (b) traveller
- (c) batter board
- (d) all of the above

- 2. Setting out works involves
 - (a) transferring plans onto the actual site.
 - (b) establishing lines and levels of work
 - (c) Making measurements to verify the location of completed parts
 - (d) all of the above
- 3. The accessory used for controlling the side slopes in embankments and in cutting
 - (a) batter board
- (b) traveller
- (c) boning rod
- (d) cross head

- 4. Setting out of bridge involves
 - (a) determination of length of the centre line
 - (b) determination of the location of piers
 - (c) (a) and (b) both
 - (d) determination of height of piers
- 5. Setting out of tunnels involves
 - (a) Surface setting out

- (b) Determination of exact length of the tunnel
- (c) Underground survey
- (d) all of the above

: ANSWERS:

| 1. | (d) | 2. (d) | 3. (a) | 4. (c) | 5. (d) | |
|----|-----|--------|----------|--------|--------|--|
| : | | | | | | |
| | | | CORPOR : | 10 | | |

SET-12

| 1. | What is the angle of intersection of | a cor | itour and a ridg | ge line ? (IES) |
|----|---|-------|------------------|--------------------------|
| | (a) 30° (b) 0° | (c) | 180° | (d) 90° |
| 2. | The process of determining the loca | ation | s of the instrun | nent station by drawing |
| | resectors from the locations of the k | | | |
| | (a) radiation (b) intersection | (c) | resection | (d) traversing |
| 3. | Which of the following methods of | plan | e table surveyi | ng is used to locate the |
| | position of an inaccessible point? | | | (IES) |
| | (a) Radiation (b) Intersection | (c) | Traversing | (d) Resection |
| 4. | Which one of the following methods | esti | mates best the a | rea of an irregular and |
| | curved boundary? | | | (IES) |
| | (a) Trapezoidal method | (b) | Simpson's meth | od |
| | (c) Average ordinate method | (d) | Mid ordinate m | ethod |
| 5. | A 100 m tape is held 1 m out of line | e. Th | e true length is | (Civil Services) |
| | (a) 100.0050 m (b) 99.9950 m | (c) | 100.0100 m | (d) 99.9800 m |
| 6. | To determine the length of a bridge | prop | osed to be built | across a wide river, the |
| | surveying method of choice would be | e | | (Civil Services) |
| | (a) tacheometry | (b) | chain surveying | |
| | (c) hydrographic surveying | (d) | Triangulation | |
| 7. | Heliograph is a type of | | shi ian i | (Civil Services) |
| | (a) Instrument used for recording the | move | ment of sun | |
| | (b) Instrument used for contouring an | area | | |
| | (c) electronic distance measuring devi | ce | | • |
| | (d) sun signal used in triangulation we | ork | | |
| 8. | For air borne application and m | ateri | ialization of G | PS receiver and easy |
| | construction, which is the most frequ | | | (IES) |
| | (a) Microstrip (b) Micropole | (c) | Spiral helix | (d) Chock ring |

| Sur | veying 37! |
|-----|--|
| 9. | Diurnal variation is greater |
| | (a) in winter than in summer (b) at smaller latitudes than at higher latitudes |
| | (c) at magnetic equator points (d) in summer than in winter |
| 10. | Which of the following figures are equal to one acre? (IES |
| | 1. 43560 sq.ft 2. 40 gunthas |
| | 3. 10 sq. Gunter's chain 4. 4850 sq. yards |
| | Select the correct answer using the code given below: |
| | (a) 1, 2 and 3 (b) 2, 3 and 4 (c) 1, 2 and 4 (d) 1, 3 and 4 |
| 11. | What is the minimum number of satellites required from which signals can be |
| | recorded to enable a global positioning system receiver to determine latitude |
| | longitude and altitude? (IES |
| | (a) One (b) two (c) three (d) four |
| 12. | Which of the following pairs are correctly matched? (IES |
| | 1. Telemetermeasurement of distance |
| | 2. Price metermeasurement of difference in elevation |
| | 3. Sounding measurement of sextant horizontal angles |
| | 4. Clinometer measurement of vertical angles |
| | Select the correct answer using codes given below: |
| | (a) 2, 3 and 4 (b) 1, 3 and 4 (c) 1, 2 and 4 (d) 1, 2 and 3 |
| 13. | The standard measurement of the Geodimeter 510 is (IES |
| | (a) ± 1 cm (b) ± 5.1 mm (c) $\sqrt{5.1}$ mm (d) ± 1 mm |
| 14. | On which of the following are the third generation electro-optical instrument |
| | based ? (IES |
| | (a) Microwave (b) infrared (c) uv light (d) He-laserlight |
| 15. | -1 |
| | the length of the vertical curve will be (IES |
| | $(g_1 + g_2)$ $(g_1 + g_2)$ $(g_1 + g_2)$ $(g_1 + g_2)$ |
| | (a) $\frac{(g_1 + g_2)}{r^2}$ (b) $\frac{(g_1 + g_2)}{\sqrt{r}}$ (c) $\frac{(g_1 + g_2)}{r}$ (d) $\frac{(g_1 + g_2)}{r^3}$ |
| 16. | Theory of errors and adjustment deals with minimising the effects of (IES |
| 10. | |
| | (a) instrumental errors (b) mistakes (c) systematic errors (d) personal and accidential errors |
| 17. | Triangulation station selected close to the main station for avoiding intervening |
| 1/, | obstruction is called (IES |
| | (a) Eccentric station (b) Pivot station (c) Satellite station (d) tie station |
| 18. | The method of orienting a plane table with two inaccessible points is called (IES |
| 10. | (a) intersection (b) resection (c) back sighting (d) two-point problem |
| | (a) intersection (b) resection (c) back signing (d) two-point problem |

| 19. | Which one of the following surveys is required in observations of stars? | | | | | | | | | | |
|-----|---|------------|---------|---------------------------|-------------|---|---|--|--|--|--|
| | (a) Astronomical survey | | | | | ` ' | Cadastal survey | | | | |
| | (c) | Aerial su | rvey | | | • / | Photogrammetric survey | | | | |
| 20 | Mat | tch List l | [and] | List II | with res | respect to aerial photogrammetry and select the | | | | | |
| | correct answer using the codes given below the lists. (Civil Service | | | | | | | | | | |
| | List I | | | | | | | | | | |
| | | (Name) | | | | (Explanation) | | | | | |
| | A. Principal point | | | | 1. | The angle formed between flight line | | | | | |
| | | | | | | and edges of photograph in the | | | | | |
| | | | | | | _ | direction of flight | | | | |
| | B. Isocentre | | | | | 2. | Failure of the aeroplane to stay on the | | | | |
| | | | | | | 2 | predetermined flight line The point where a perpendicular | | | | |
| | C. | Crab | | | | 3. | dropped from the front nodal point | | | | |
| | | | | | | | strikes the photograph | | | | |
| | | | | | | 4. | The point at which bisector of the angle | | | | |
| | D. | Drift | | | | | of tilt meets the photograph | | | | |
| | Co | des: | | | | | | | | | |
| | Co | A | В | С | D | | | | | | |
| | (a) | | 3 | 2 | 1 | | | | | | |
| | (b) | | 3 | 1 | 2 | | | | | | |
| | (c) | 3 | 4 | 1 | 2 | | | | | | |
| | (d) | | 4 | 2 | 1 | | | | | | |
| 21. | To to VI I List one of the following magnetic hearing wor | | | | | | | | | | |
| | rej | resent tr | ue bea | ring o | f S 25° 20 |)' E ? | (IES) | | | | |
| | (a) | S 19° 20 | 0'E | S 20° 0' E (d) S 19° 20'W | | | | | | | |
| 22. | (Civil Services) | | | | | | | | | | |
| | D | = Distanc | e in km | 1 | | | | | | | |
| | h = visible horizon from a station of known elevation above the datum (in metres) | | | | | | | | | | |
| | | | | | | | | | | | |
| | If there is no obstruction due to intervening ground, then h is equal to | | | | | | | | | | |
| | (a) | 0.6735 | D^2 | | $6.735 D^2$ | | 0.06735 D ² (d) 0.006735 D ² | | | | |
| 23. | | | | | lace is the | e avera | ge datum of hourly tide height observed (Civil Services) | | | | |
| | over a period of nearly | | | | | 7.3 | | | | | |
| | (a) |) 5 years | • | (b) | 10 years | (c) | 20 years (d) 50 years | | | | |
| | | | | | | | · · | | | | |

| _ | - |
|-------|-------|
| SHIV | prive |
| -u: - | ~ 7 |

| | , | | | | | | *** | | | | | |
|-----|--|--|------------|------------|-----------|---------|---|--------------------------------|--|--|--|--|
| 24. | | How high should a helicopter pilot rise at a point 'A' just to see the horizon at point 'B' if the distance AB is 40 km? (IES) | | | | | | | | | | |
| | (a) | 10.75 m | (| b) 110 | .50 m | (c) | 107.75 m | (d) 105. 50 m | | | | |
| 25. | If two triangulation signals of 6.75 m height each are to be just visible over ground | | | | | | | | | | | |
| | mutually, what is the maximum distance between their locations on the ground surface? | | | | | | | | | | | |
| | | 10 km | (| b) 20 | km | (c) | 30 km | (d) 40 km | | | | |
| 26. | ` ' | | • | - | | | r using the code given below | | | | | |
| | | lists : | and not | ıx ana ı | ,01000 | | (IES) | | | | | |
| | the | List I | | | | | | List II | | | | |
| | 1100 | | | | | | Contour lines of different elevations unit to form one line | | | | | |
| | A. Vertical cliff | | | | | | | | | | | |
| | B. | Steep slo | ope | | | 2. | | ines of different elevations | | | | |
| | C | Hill | | | | 3. | Contour li | nes are closely spaced | | | | |
| | C. | | aina aliff | | | 4. | | ntour lines with higher values | | | | |
| | υ. | Overhan | ging citt | | | | inside ther | | | | | |
| | Cor | des : | | | | | | | | | | |
| | | A | В | С | D | | | | | | | |
| | (a) | 4 | 3 | 1 | 2 | | | | | | | |
| | (b) | 1 | 3 | 4 | 2 | | | | | | | |
| | (c) | 1 | 2 | 4 | 3 | | | | | | | |
| | (d) | 4 | 2 | 1 | 3 | | | | | | | |
| 27. | | | | ollowine | | ods of | levelling | eliminates the error due to | | | | |
| LI. | | vature a | | | , ,,,,,,, | | | (IES) | | | | |
| | | Fly leve | | | | | | SE INNU (SILI/A)) | | | | |
| | | | | alizing tl | he dista | ince of | backsight a | and foresight | | | | |
| | | | | | | | Ü | | | | | |
| | | (c) Check leveling (d) Precise levelling | | | | | | | | | | |
| 28. | ٠, | | _ | ength 1 | km sc | ales 8 | cm on a | vertical photograph. If focal | | | | |
| 20. | A road section of length 1 km scales 8 cm on a vertical photograph. If focal length of camera is 160 mm and the terrain is fairly level, then flying height will | | | | | | | | | | | |
| | be (IES) | | | | | | | | | | | |
| | | 20 m | | (b) 200 | 00 m | (c) | 20 km | (d) 200 km | | | | |
| 29. | • ′ | ven that i | | • • | | ` ' | | (Civil Services) | | | | |
| | L = Length of the tracing arm | | | | | | | | | | | |
| | R = Radius of the anchor arm | | | | | | | | | | | |
| | | | | | | | | | | | | |
| | | , distance | | | | , | | | | | | |
| Obj | ective | : Civil Eng | .\2016\ | 48 | | | | | | | | |

If wheel is beyond the hinge, then area of zero circle will be

(a) $\pi (L^2 - 2aL + R^2)$ (b) $\pi (L^2 + 2aL + R^2)$ (c) $\pi (L^2 + aL + R^2)$ (d) $\pi (a^2 - aL + R^2)$

30. Given that

(Civil Services)

f = focal length of camera

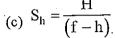
H = Height of exposure station above MSL

h = Height of ground above MSL

The scale of vertical photograph is given by:

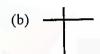


(b) $S_h = \frac{f}{(H-h)}$ (c) $S_h = \frac{H}{(f-h)}$ (d) $S_h = \frac{f}{(H-h)}$



31. Which one of the following is a conventional sign for North line in (Civil Services) surveying?









32. In photogrammetric surveying, the relief displacement

(Civil Services)

- (a) decreases with increase in flying height
- (b) is negative for a point above datum
- (c) decreases as the distance at the object from the principal point increases.
- (d) of the point is not affeted by the tilt of the photograph

If cross section areas of an embankment at 30 m intervals are 20, 40, 60, 50 and 30 m² respectivery, then volume of the embankment on the basis of prismoidal (Civil Services) rule is,

- (a) 5300 m^3
- 8300 m^3 (b)
- (c) 9300 m^3
- (d) 9400 m^3

34. A 3000 m long line lying at elevation of 450 m measures 10 cm on a vertical photograph. The focal length of the camera is 21 cm. The scale of the photograph for the area lying at an elevation of 1000 m will be (Civil Services)

- (a) 1:27381
- (b) 1:25008
- (c) 1:20606
- (d) 1:30421

The representative fraction $\frac{1}{2500}$ means that the scale is 1cm equal to (Civil Services)

- (a) 0.25 m
- (B) 2.5 m
- (c) 25 m
- (d) 2.5 km

36. In photogrammetric surveying, image of the top of the hill is 90 mm from the principal point of the photograph. If elevation of top of the hill is 500 m and flying height is 5000 m above datum, then relief displacement is

(Civil Services)

- (a) 0.9 mm
- 9 mm (b)
- (c) 90 mm
- (d) 900 mm

| 37. | Which one of the following gives the corret distance between light house and a |
|-----|---|
| | ship, when the light house whose height is 100 m is visible just above horizon from ship? (Civil Services) |
| | |
| 20 | |
| 38. | Which one of the following closely represents shape of the earth? (Civil Services) (a) Spheroid (b) Ellipsoid (c) Oblate spheroid (d) Prolate spheroid |
| 20 | |
| 39. | Contour interval on map sheet denotes (Civil Services) (a) vertical distance of contour lines above the datum line |
| | (b) vertical distance between two successive contour lines |
| | (c) slope distance between two successive contour lines |
| | (d) horizontal distance between two successive contour lines |
| 40 | ABCD is a regular parallelogram plot of land, whose angle BAD is 60°. If bearing |
| 40. | of the AB is 30°, then bearing of the line CD is (Civil Services) |
| | (a) 90° (b) 120° (c) 210° (d) 270° |
| 41. | The true length of a line is know to be 200 m, when this is measured with a 20 m |
| 71. | tape, the length is 200.80 m. The correct length of the 20 m tap is (IES) |
| | (a) 19.92 m (b) 19.98 m (c) 20.04 m (d) 20.08 m |
| 42. | It is required to produce a small-scale map of an area in a magnetic zone by |
| | directly plotting and checking the wrok in the field itself. Which one of the |
| | following surveys will be most appropriate for this purpose? (IES) |
| | (a) Chain (b) Theodolite (c) Plane table (d) Compass |
| 43. | A 30 m metric chain is found to be 0.1 m too short throughout the measurement. If |
| | the distance measured is recorded as 300 m, then the actual distance will be (IES) |
| | (a) 301 m (b) 300 m (c) 299 m (d) 298 m |
| 44. | The vertices of an astronomical triangle would include (IES) |
| | (a) zenith, pole and heavenly body (b) azimuth, zenith and pole |
| | (c) azimuth, pole and heavenly body (d) azimuth, zenith and heavenly body |
| 45. | The direction of magnetic meridian is established at each traverse station and |
| | direction of the line is determined with reference to the magnetic meridian. This |
| | method of traversing is called (IES) |
| | (a) Fast needle method (b) Loose needle method |
| | (c) Bearing method (d) Fixed needle method |
| 46. | If fore bearing of a line is S 49° 52' E (assuming there is no local attraction), |
| | then back bearing of the line will be (IES) |
| | (a) S 52° 49' E (b) S 49° 52' E (c) N 49° 08' E (d) N 49° 52' W |
| 47. | <u>-</u> |
| | (Civil Services) |

 $x_A = 1000.0 \text{ m}$

 $y_A = 1000.0 \text{ m}$

 $x_{\rm B} = 2000.0 \text{ m}$

$$y_B = 1000.0 \text{ m}$$

The bearing of the line AB will be

- (a) 0° 0' 00"
- (b) 60° 0' 00"
- (c) 90° 0' 00"
- (d) 180° 0' 00"
- 48. The downhill end of a 20 m tape is held 80 cm too low, then its horizontal length (Civil Services) will be
 - (a) 19.894 m
- (b) 19.984 m
- (c) 20.016 m
- (d) 20.984 m
- 49. The following equipments can be used to lay out horizontal angles in field
 - 1. Microptic theodolite
- Chain and metallic tape 2.
- Vernier theodolite
- Prismatic compass

The correct sequence of the decreasing order of their accuracies is

- (a) 2, 4, 3, 1
- (b) 2, 3, 4, 1
- (c) 1, 4, 3, 2
- (d) 1, 3, 4, 2
- 50. Which of the following are the fundamental lines of a theodolite?
 - (1) The vertical and horizontal axes
 - (2) The diagonally opposite screw lines
 - (3) The line of collimation and axes of the plate levels
 - (4) The bubble line of the altitude level

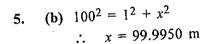
Codes:

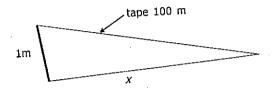
- (a) 1, 2 and 3
- (b) 1, 2 and 4 (c) 2 and 3
- (d) 1, 3 and 4

: ANSWERS :

| 1. (d) | 2. (c) | 3. (b) | 4. (b) | 5. (b) | |
|---------|---------|---------|---------|---------|----|
| 6. (d) | 7. (d) | 8. (c) | 9. (d) | 10. (a) | |
| 11. (d) | 12. (a) | 13. (a) | 14. (d) | 15. (c) | |
| 16. (b) | 17. (c) | 18. (d) | 19. (a) | 20. (c) | |
| 21. (b) | 22. (c) | 23. (c) | 24. (c) | 25. (b) | 40 |
| 26. (b) | 27. (b) | 28. (b) | 29. (b) | 30. (b) | |
| 31. (d) | 32. (a) | 33. (a) | 34. (a) | 35. (c) | |
| 36. (b) | 37. (c) | 38. (c) | 39. (b) | 40. (c) | |
| 41. (a) | 42. (c) | 43. (c) | 44. (a) | 45. (a) | |
| 46. (d) | 47. (c) | 48. (b) | 49. (d) | 50. (d) | |
| ı | 1 | I | | | |

EXPLANATIONS





10. (a) 1 acre = 2.5 vigha = 40 Guntha = 4047 sq.m

21. (b) Since declination is towards west, use - ve sign

:. True bearing = Magnetic bearing - declination
25° 20' = Magnetic bearing - 5° 40'

Magnetic bearing = S 31° 0' E

24. (c)
$$h = 0.06735 D^2$$

= $0.06735 \times (40)^2$
= 107.75 m

25. (b)
$$h = 0.06735 D^2$$

 $6.75 = 0.06735 D^2$
 $D = 10 \text{ km}$

Distance between two stations = $2 \times 10 = 20 \text{ km}$

28. (b) Scale of vertical photograph =
$$\frac{f}{H-h}$$

ground is levelled

$$\therefore \frac{8 \text{ cm}}{1 \text{ km}} = \frac{f}{H - h}$$

$$h=0$$

$$\frac{80 \text{ mm}}{1 \times 10^6 \text{mm}} = \frac{160}{H - 0}$$

∴
$$H = 2 \times 10^6 \text{ mm}$$

= 2 km = 2000 m

33. (a) Prismoidal rule,

$$V = \frac{h}{3} [(A_0 + A_n) + 4 (A_1 + A_3 + ...) + 2 (A_2 + A_4 + ...)]$$

$$= \frac{30}{3} \times [(20 + 30) + 4 (40 + 50) + 2 (60)]$$

$$= 5300 \text{ m}^3$$

Objective Civil Engineering

34. (a) Scale =
$$\frac{f}{H-h}$$

$$\frac{0.1}{3000} = \frac{0.21}{H-450}$$

$$\therefore$$
 H = 6750 m

Now for scale at h = 1000 m

Scale =
$$\frac{f}{H - h}$$

= $\frac{0.21}{6750 - 1000}$
= $\frac{1}{27381}$

35. (c) R.F. =
$$\frac{1 \text{ cm}}{2500 \text{ cm}} = \frac{1 \text{ cm}}{25 \text{ m}}$$

36. (b) relief displacement,

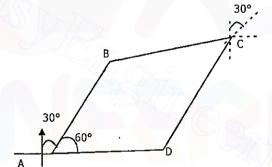
$$d = \frac{rh}{H} = \frac{0.090 \times 500}{5000} = 0.009 \text{ m} = 9 \text{ mm}$$

37. (c)
$$h = 0.06735 D^2$$

 $100 = 0.06735 D^2$

$$\therefore D = 38.54 \text{ km}$$

40. (c)



Bearing of CD = $30^{\circ} + 180^{\circ}$

$$= 210^{\circ}$$

41. (d)
$$l = l' \times \left(\frac{L'}{L}\right)$$
 : $200 = 200.80 \times \left(\frac{L'}{20}\right)$: $L' = 19.92 \text{ m}$

43. (c)
$$l = l' \times \left(\frac{L'}{L}\right)$$

$$l = 300 \times \left(\frac{29.90}{30}\right)$$

$$= 299 \text{ m}$$

48. (b) Horizontal length =
$$\sqrt{(20)^2 - (0.8)^2}$$

= 19.984 m



Fluid Mechanics

| A sa e e e e e e e e e e e e e e e e e e | <u>SEI-1 : Fluids</u> |
|--|-----------------------|
| An ideal fluid is one wh | ial |
| AS OHE WI | lien |

- 1.
 - is compressible (a)

- has no viscosity
- is elastic and viscous
- is non-viscous and incompressible
- Surface tension is a phenomenon due to
 - cohesion only

- (b) viscous force
- difference in magnitude between forces due to cohesion and adhesion (c)
- adhesion only
- Ball pen works on the principle of
 - Viscosity
- (b) Surface tension (c)
- Gravitational force (d) Boyle's law
- The bulk modulus of elasticity of a fluid is

(a)
$$-\frac{\left(\frac{dv}{v}\right)}{dp}$$

(b) $-\frac{dp}{\left(\frac{dv}{dy}\right)}$ (c) $\mu \cdot \frac{dp}{dy}$

- The pressure within a soap bubble is 5.
 - the same as the surrounding atmosphere
 - less than the external pressure
 - greater than the external pressure
 - equal to the vapour pressure
- A fluid which obeys the relation $\tau = \mu \cdot \frac{du}{dv}$ is called the
 - (a) Ideal fluid
- (b) Newtonian fluid (c) Perfect fluid (d)

Pseudo plastic

- The viscosity of water with respect to air is about:
 - (a)
- (b) 55
- (c) 60
- (d) 65 times

- Newton's law of viscosity relates
 - Stress and strain in a fluid
 - Shear stress and rate of angular deformation of a fluid (b)
 - Pressure, velocity and viscosity of a gas
 - Viscosity and rate of angular deformation of a fluid. (d)

| 384 | | | | | | | 00,00 | | - Engineering |
|------|-------------------------------|--------------|----------------------|-----------|--------|--------------------|-----------------|------------|------------------------------|
| 9. | The capillar | y depressio | n in me | ercury i | s on a | ecount (| of | | |
| | (a) adhesion | n being grea | ter than | n cohesie | on | | | | |
| | (b) Surface | tension bei | ng great | ter than | viscos | ity | | | |
| | (c) Cohesia | n being gre | ater tha | n adhesi | on | | | | |
| | (d) Vapour | pressure bei | ng smal | [1 | | | | | |
| 10. | 1 stoke is eq | | _ | | | | 0. | | |
| | (a) $1 \text{ cm}^2/\text{s}$ | (b) |) 1 m ² / | s | (c) | 0.10 cm | ² /s | (d) | $0.10 \text{ m}^2/\text{s}$ |
| 11. | 1 Poise is eq | | | . 2 | | | 2 | . 15 | 700 37 / 2 |
| | (a) 1 N.s/m | | | | | | | ` ' | 100 N.s/m ² |
| 12. | In case of m | | solids, | the law | | | | | cosity is |
| | (a) Hooke's | | | | ` ' | Archime | _ | 7 | mation |
| | (c) Newton | | | | • • | Newton | 's second | a law of | inotion . |
| 13. | The unit of | | | | | | | (4) | Poise |
| | (a) m^2/s | ` ' | | Th | | N.s/m ² | n is cal | • • | i oisc |
| 14. | All liquid su | | | | | ace tensi | | | Cavitation |
| 4 74 | (a) Cohesic | • | | | | | | (4) | Cuvitation. |
| 15. | Falling drop | tension (b | | | | | | (d) | Viscosity |
| 16 | | | | | | | | ` ' | re is give <mark>n by</mark> |
| 10. | | | | | | • | | | |
| | (a) $\frac{8\sigma}{d}$ | (b | $\frac{4\sigma}{1}$ | | (c) | 20 | | (d) | <u>d</u> |
| | u | | u | | | u | | | |
| 17. | | | interta | ce, the c | contac | r angle o | is less | man 90° | . This indicates |
| | that the liqu | ula is | | | (h) | wetting | | | |
| | (a) ideal | attina | | | (d) | _ | | a stable | bubble. |
| 18. | (c) Non-w In the phene | | evitatio | n, the c | ` ' | | | | |
| 10. | | tension | MITTER | , | | Viscosi | | | |
| | ` ' | pressure | | | (d) | | | of elastic | ity |
| | (c) rupou | pressure | | | () | - | | | • |
| | | | | : ANS | WER | S: | | | , |
| | 1. (d) | 2. | (c) | 3. | (b) | 4. | (b) | 5. (| |
| | 6. (b) | 7. | (b) | 8. | (b) | 9. | (c) | 10. (| |
| | 11. (b) | 12. | (a) | 13. | (b) | 14. | (c) | 15. (| (a) |
| | 16. (b) | 17. | (b) | 18. | (c) | ĺ | | | |
| Ŧ | | | | | | | | | |

SET-2: Hydrostatics

| 1. | One | atmospheric pr | essure equals | | | | |
|-----|-------|--------------------|-----------------------|-------|-----------------------|------------------------|--|
| | (a) | 760 mm of mer | cury head | (b) | 10.34 m of water c | olumn | |
| | (c) | 1.01325 bar | | (d) | 101.43 kPa | | |
| | (e) | all of the above | | | | | |
| 2. | A bl | ock of ice floats | on the surface of v | watei | r contained in a ves | sel. How the wate | |
| | level | will change wh | en the snow melts | ? | | | |
| | (a) | rises | | (b) | falls | | |
| | (c) | remains the sam | | (d) | • | = | |
| 3. | Indi | cate the variation | n of hydrostatic pr | ressu | re with depth belov | v the free surface | |
| | (a) | It increases as t | he depth increases. | | | | |
| | (b) | It decreases with | h the increase in de | pth. | | | |
| | (c) | There is no cha | J7 (| | | | |
| | | | period there is no cl | | | | |
| 4. | | | of pressure variat | | | | |
| | (a) | $p = \frac{z}{w}$ | (b) $p = wz$ | (c) | p = wz + constant | (d) $p = constant$ | |
| 5. | | | | | | | |
| | (a) | a manometer | | (b) | a Bourdon gauge | | |
| | (c) | a mercury mano | | , , | a vacuum gauge | | |
| 6. | Stan | dard atmospher | ric pressure in tern | as of | water column is | | |
| | (a) | 9.81 m | (b) 10.34 m | | 8.75 m | (d) 12.35 m | |
| 7. | | | | | point in a liquid is | | |
| | (a) | same in vertical | direction | | same in horizontal | | |
| | (c) | same in all dire | ctions | (d) | different in differen | nt directions | |
| 8. | 1 ba | r = Pa. | | | | - | |
| | (a) | 1 Pa | (b) 10^3 Pa | (c) | 1.01325 Pa | (d) 10 ⁵ Pa | |
| 9. | Pres | sure measured | with the help of pi | ezom | eter tube is | | |
| | (a) | absolute pressur | ·e | (b) | gauge pressure | | |
| | (c) | atmospheric pre | ssure (d) | vacu | ium pressure | | |
| 10. | Mat | h the column A | and column B: | | • | | |
| | | A (Structure) | | | B (hydrostatic for | ce resisted by) | |
| | (i) | Gravity dam | | (a) | Strength of cylindr | ical shell | |
| | (ii) | Circular water t | ank | (b) | Abutment reactions | 3 | |
| | (iii) | Arch dam | | (c) | hinge and side read | ction | |
| | (iv) | Lock gates | | (d) | Weight of concrete | • | |
| | | a ^a | | | | | |

Objective Civil Eng. \setminus 2016 \setminus 49.

11. Absolute pressure in a flow-system:

- (a) is always above local atmospheric pressure.
- (b) is a vacuum pressure.
- (c) may be above, below or equal to the local atmospheric pressure.
- (d) is also called negative pressure.

12. Mercury is generally used in manometers for measuring:

- (a) Low pressures accurately
- (b) Large pressures only
- (c) All pressures except the smaller ones
- (d) Very low pressures

13. 0.3 m head of oil (Sp. gr. 0.8) is equal to

- (a) 0.3 m head of water
- (b) 0.24 m head of water
- (c) 1.3 m head of water
- (d) 10.34 m head of water

14. Differential manometer is used to measure

- (a) absolute pressure at a point
- (b) pressure difference between two points
- (c) atmospheric pressure
- (d) all the above

: ANSWERS:

| 1. (e) | 2. (c) | 3. (a) | 4. (b) | 5. (c) |
|----------|-----------|----------|---------|-------------|
| 6. (b) | 7. (c) | 8. (d) | 9. (b) | 10. (i) – d |
| (ii) – a | (iii) — b | (iv) – c | 11. (c) | 12. (c) |
| 13. (b) | 14. (b) | 0 | | |

SET-3: Total Pressure and Centre of Pressure

- 1. The centre of pressure acts the C.G. of the immersed surface.
 - (a) at
- (b) above
- (c) below
- (d) can't say
- 2. The distance of C.G. and centre of pressure for a vertically immersed surface is equal to
 - (a) $\frac{I_g}{\overline{h}}$
- (b) $\frac{I_g}{A\overline{h}}$
- (c) $\frac{I_g}{A\overline{h}} + \overline{h}$
- (d) $\frac{A\overline{h}}{I_{\sigma}}$
- 3. The total pressure on an immersed surface inclined at an angle θ with the liquid surface is
 - (a) $wA\overline{h}$
- (b) $\frac{wA\overline{h}}{\sin\theta}$
- (c) $\frac{\text{wA}\overline{\text{h}}}{\cos\theta}$
- (d) $\frac{wA\overline{h}}{\tan \theta}$

(a)

Centre of pressure

Centre of pressure

Centre of buoyancy

oscillating about a point. This point is called

Metacentre

(b) Centre of buoyancy

Centre of gravity

(d) all of these

(d) metacentre

When a body floating in a liquid is given a small angular displacement, it starts

| 3. | A | floating | body | is | in | stable | equilibrium | when |
|----|---|----------|------|----|----|--------|-------------|------|
|----|---|----------|------|----|----|--------|-------------|------|

- C.G. is below the centre of buoyancy (b) its metacentric height is zero
- Its metacentric height is positive (d) its metacentric height is negative
- If G is the centre of gravity, B is the centre of buoyancy and M is the metacentre 4. of a floating body then for the body to be in unstable equilibrium
 - (a) GM = 0
- (b) BG = 0
- (c) M is below G
- (d) M is above G

5. For stable equilibrium of a floating body

(a)
$$BG = \frac{I}{V} + MG$$
 (b) $GM = \frac{I}{V} + BG$ (c) $GM = \frac{I/V}{BG}$ (d) $GM = \frac{I}{V} - BG$

- For warships, metacentric height of a ship should vary between
 - (a) 0 1 m
- (b) 1 2 m
- (c) 5 10 m
- (d) More than 10 m
- The metacentric height of two bodies A and B are 1 m and 1.5 m respectively. 7. Select the correct statement.
 - (a) Both A and B have equal stability (b) Both A and B are unstable
 - Body A is more stable than B
- (d) Body B is more stable than A
- During flood water entered an office having wooden tables. Indicate the position of tables, if floating
 - (a) Legs downwards

(b) Legs on sides

(c) Legs upwards

- The tables will not float
- Match list I and list II.

List I

- G is above M
- B. G and M coincide
- C. G is below M
- D. $F_b \ge W$

A.

List II

- Stable equilibrium 1.
- 2. Unstable equilibrium

D

4

2

1

- 3. Floating body
- Neutral equilibrium

- Codes:
- A
- C В
- (a) 1
- 2
- 3 (b)

- (c) 2
- 2 (d)
- 3

: ANSWERS:

1. (b) 2. (d) 3. (c) 4. (c) 5. (d) 6. (b) 7. (d) 8. (c) 9. (d)

SET-5: Fluid Kinematics

- The path traced by a single particle of smoke issuing from a cigarette is a 1.
 - Streamline
- (b) Flow line
- Path line
- (d) Streakline

- Laminar flow is also called 2.
 - Steady flow (b) Uniform flow (c) Unsteady flow (d) Streamline or viscous flow
- A streamline is a line
 - Connecting mid points of a flow cross-section
 - Drawn normal to the velocity vector at any point
 - Connecting points of equal velocity in a flow
 - Tangent to which at any point gives the direction of velocity.
- For a two-dimensional flow field, the equation of streamline is

(a)
$$\frac{u}{dx} = \frac{dy}{y}$$

(a)
$$\frac{u}{dx} = \frac{dy}{v}$$
 (b) $\frac{du}{dx} + \frac{dv}{dy} = 0$ (c) $\frac{dy}{u} = \frac{dx}{v}$ (d) $\frac{dx}{u} = \frac{dy}{v}$

(c)
$$\frac{dy}{u} = \frac{dx}{v}$$

d)
$$\frac{dx}{u} = \frac{dy}{v}$$

- Steady irrotational flow of an incompressible fluid is called 5.
 - Streamline flow (b) Potential flow (c) Shear flow
- Creeping flow
- During the opening of a valve, the flow is
- (b) Unsteady
- Uniform (c)
- (d) Rotational
- If ψ is the stream function, then velocity components u and v are given by :

(a)
$$u = \frac{\partial \Psi}{\partial x}, v = \frac{\partial \Psi}{\partial y}$$

(b)
$$u = \frac{\partial \Psi}{\partial y}, v = \frac{-\partial \Psi}{\partial x}$$

(c)
$$u = \frac{-\partial \Psi}{\partial y}, v = \frac{\partial \Psi}{\partial x}$$

(d)
$$u = \frac{-\partial \Psi}{\partial y}, v = \frac{-\partial \Psi}{\partial x}$$

- The continuity equation $\frac{\partial u}{\partial x} + \frac{\partial u}{\partial y} + \frac{\partial u}{\partial z} = 0$ is valid for
 - ideal fluid flow only (a)
 - incompressible fluids, whether flow is steady or unsteady (b)
 - steady flow, whether compressible or incompressible flow
 - incompressible fluids and steady flow only.
- Flow of water in river is example of
 - One dimensional flow
- (b) Laminar flow
- Two dimensional flow
- (d) Three dimensional flow
- 10. If ϕ is a potential function, then velocity components u and v are given by

(a)
$$u = \frac{-\partial \phi}{\partial x}$$
, $v = \frac{-\partial \phi}{\partial y}$ (b) $u = \frac{\partial \phi}{\partial x}$, $v = \frac{\partial \phi}{\partial y}$ (c) $u = \frac{-\partial \phi}{\partial x}$, $v = \frac{\partial \phi}{\partial y}$ (d) $u = \frac{\partial \phi}{\partial x}$, $v = \frac{-\partial \phi}{\partial y}$

390

11. Vorticity is given by

1.5 times rotation

- (b) Two times rotation
- Three times rotation
- (d) equal to rotation

12. The local acceleration in the direction of x is given by

- $u \frac{\partial u}{\partial x} + \frac{\partial u}{\partial t}$
- (c) $u \frac{\partial u}{\partial x}$
- (d) None of these

13. If velocity in a fluid flow does not change with respect to length of direction of flow, it is called

- (a) Uniform flow
- (b) Steady flow (c) Incompressible flow
- (d) Rotational flow

14. If density of fluid is constant from point to point in a flow region, it is called

- - (a) Unsteady flow (b) Irrotational flow (c) Incompressible flow (d) none of the above
- 15. If the velocity, pressure, density etc. do not change at a point with respect to time, the flow is called
 - (a) Uniform
- (b) Steady
- (c) Non-uniform (d)
- incompressible

: ANSWERS:

| 1. (c) 6. (b) | 7. (c) | 3. (d) 8. (d) 13. (a) | 4. (d) 9. (d) 14. (c) | 5. (b) 10. (a) 15. (b) |
|------------------|---------|-----------------------------|-----------------------------|------------------------------|
| 11. (b) | 12. (b) | 13. (a) | 14. (C) | 15. (0) |

SET-6: Fluid Dynamics

Identify the Bernoulli's equation where each term represents energy per unit mass

(a)
$$z + \frac{v^2}{2g} + \frac{p}{w} = constant$$

(b)
$$z + \frac{v^2}{2} + \frac{gp}{w} = constant$$

(c)
$$z + \frac{pv^2}{2} + \frac{g}{w} = constant$$

(d) None of these

Study of fluid motion with the forces causing the flow is known as

- Kinematics of fluid flow
- (b) Dynamatics of fluid flow
- Statics of fluid flow
- (d) None of above

The term $\frac{\mathbf{v}}{2\mathbf{g}}$ is known as

(a) Potential energy

- (b) Pressure energy
- (c) Kinetic energy per unit weight
- (d) None of the above

| 4. | The term $\frac{p}{w}$ is known as | | |
|-----|---|---|--|
| | (a) Kinetic energy per unit we | ight (b) Pressur | e energy |
| | (c) Pressure energy per unit w | eight (d) Potenti | al energy |
| 5. | The term Z is known as | | |
| | (a) Potential energy (b) | Potential en | ergy per unit weight |
| | (c) Pressure energy | (d) Pressur | e energy per unit weight |
| 6. | The difference of pressure manometer is given by | head (h) measured | by mercury-oil differential |
| | | , | |
| | (a) $h = \left(\frac{S_1}{S_2} - 1\right)x$ (b) $h = \left(\frac{S_1}{S_2} - 1\right)x$ | $\frac{S_2}{S_1} - 1 x$ (c) $h = (S_1)$ | $-S_2$) x (d) h = $(S_2 - S_1)$ x |
| | where, | | |
| | $S_1 = Sp.$ | gravity of oil | |
| | $S_2 = Sp. g$ | gravity of mercury | |
| | x = differential formula x = x = x = x = x | ence of mercury level | |
| 7. | Pitot-tube is used to measure | | |
| | (a) Discharge (b) Avera | ge Velocity (c) Velocit | y at a point (d) Pressure at a point |
| 8. | Venturimeter is used to measu | | |
| | (a) Discharge (b) Avera | ge velocity (c) Velocit | y at a point (d) Pressure at a point |
| 9. | Orifice meter is used to measu | | |
| | (a) Discharge (b) Avera | ge velocity (c) Veloci | ty at a point (d) Pressure at a point |
| 10. | The rate of flow through vent | | ion Innessitie |
| | | (c) $H^{3/2}$ | |
| 11. | The kinetic energy correction | factor for laminar f | low through a circular pipe is |
| | approximately equal to | | (1) 0.05 |
| | (a) 1.0 (b) 1.5 | | (d) 2.25 |
| 12. | . The Bernoulli's equation deals | | the second secon |
| | (a) mass (b) work | (c) length | (d) energy |
| 13. | Which of the following is not t | | |
| | (a) The fluid is ideal | • , | uid is compressible |
| | (c) The flow is irrotational | ` ' | ow is steady |
| 14. | · · · · · · · · · · · · · · · · · | | |
| | (a) Bernoulli's equation | | ntum equation |
| | . (c) Continuity equation | (d) None | of the above |

392

ANSWERS

| 1. (a) | 2. (b) | 8. (a) | 4. (c) | 5. (b) |
|---------|-----------|---------|---------|---------|
| 6. (b) | 7. (c) | | 9. (a) | 10. (b) |
| 11. (c) | _ 12. (d) | 13. (b) | 14. (a) | |

SET-7: Flow through Pipes

The loss of head due to friction according to Darcy's formula is

(a)
$$\frac{f l V^2}{2g d}$$

(b)
$$\frac{4 f l V^2}{2g d}$$

(b)
$$\frac{4 \text{ f } l \text{ V}^2}{2 \text{ g d}}$$
 (c) $\frac{4 \text{ f } l \text{ V}^2}{\text{g d}}$

(d)
$$\frac{4 f l V}{2g d}$$

The T.E.L. lies over the H.G.L. by an amount equal to 2.

(a)
$$\frac{V^2}{2g}$$

(c)
$$\frac{P}{\rho g}$$

(d) $\frac{V}{2g}$

The hydraulic mean depth for a circular pipe is 3.

(a)
$$\frac{d}{2}$$

(d) $\frac{d}{6}$

The loss of head due to sudden enlargement is given by

(a)
$$\frac{0.5 \text{ V}_2^2}{2\text{g}}$$

(b) $\frac{(V_1 - V_2)^2}{2g}$ (c) $\frac{(V_1 + V_2)^2}{2g}$

The loss of head at the exit is 5.

(a)
$$\frac{0.5 \,\mathrm{V}^2}{2 \,\mathrm{g}}$$

(b) $\frac{V^2}{2g}$

(c) $\frac{\left(V_1 - V_2\right)^2}{2g}$

The relation between friction factor and coefficient of friction (f) is given by

(a)
$$C_f = f$$

(b) $C_f = 2f$

(c) $C_f = 4f$

(d) $C_f = 3f$

Loss of head due to friction in pipe is given by

(a) Bernoulli's equation

(b) Continuity equation

Manning's equation

(d) Darcy-Weisbach equation

A compound pipe is required to be replaced by a new pipe. Both the pipes are said to be equivalent if both of them have same

length and diameter (b) loss of head (c) discharge

(d) both 'b' and 'c'

In case of parallel pipes

loss of head for all pipes is same

total discharge is equal to sum of discharge in all pipes

Total loss of head is equal to the sum of loss of heads in all the pipes. (c)

Both 'a' and 'b' (d)

| 10. | For | pipes | arranged | in | series |
|-----|-----|-------|----------|----|--------|
|-----|-----|-------|----------|----|--------|

- The head loss must be same in all the pipes (a)
- The velocity must be same in all pipes (b)
- The flow may be different in different pipes
- The total flow is same flowing through each pipe

11. Pipes are arranged in parallel to

increase discharge

- decrease head loss (b)
- decrease discharge (c)

(d) reduce length of pipe

12. The power transmitted through a pipe is maximum when the head loss due to friction is equal to

(b) $\frac{H}{3}$

(c) H

(d) $\frac{H}{4}$

ANSWERS

| 1. (b) 2. (a) 3 6. (c) 7. (d) 8 11. (a) 12. (b) | (c) 4. (b) (d) 9. (d) | 5. (b) 10. (d) |
|---|-----------------------|-------------------|
|---|-----------------------|-------------------|

SET-8: Orifice and Mouthpieces

The theoretical velocity of the jet at vena-contracta is

- (a) $\sqrt{2gH}$
- (b) $2\sqrt{gH}$
- (c) $2g\sqrt{H}$
- (d) 2gH

The relation between C_v, C_c and C_d is 2.

- (a) $C_d = C_v + C_c$ (b) $C_d = C_v C_c$ (c) $C_d = C_v \times C_c$
- (d) $C_d = C_v/C_c$

The coefficient of velocity is determined experimentally by using the relation 3.

(a)
$$C_v = \sqrt{\frac{y^2}{4xH}}$$
 (b) $C_v = \sqrt{\frac{y}{4xH}}$ (c) $C_v = \sqrt{\frac{x^2}{4yH}}$

(d) $C_v = \sqrt{\frac{x}{\Delta v H}}$

Coefficient of discharge for external cylindrical mouthpiece is

- (a) 0.98
- (b) 0.90
- (c) 0.82
- (d) 0.855

An orifice is said to be large, if 5.

- (a) Size of orifice is large
- (b) Velocity of flow is large
- (c) Available head of water is more than 5 times its height
- (d) Available head of water is less than 5 times its height

An orifice fitted with some kind of pipe extension is known as

- Notch
- (b) Weir
- (c) mouthpiece
- (d) Nozzle

ANSWERS

| | | | | | | Γ. | 4.15 | 5 (1) | 6 (0) |
|---|----|-----|------|-----|--------|----------|------|----------|--------|
| ļ | 1 | (0) | 1 2 | (c) | 3. (c) | i 4. | (d) | 5. (a) | 0. (C) |
| 1 | 1. | (a) | ۲۰ ا | (0) | J. (4) | <u> </u> | | <u> </u> | 6. (c) |

Objective Civil Eng. \ 2016 \ 50

SET-9: Notches and Weirs

| 1. | The discharge over rectangular notes | |
|----|--|---|
| | (a) directly proportional to H ^{3/2} | (b) Directly proportional to H ^{5/2} |
| | (c) Inversely proportional to H ^{3/2} | (d) Inversely proportional to $\mathrm{H}^{5/2}$ |
| 2. | The discharge over a triangular note | h is |
| | (a) directly proportional to H ^{3/2} | (b) Directly proportional to H ^{5/2} |
| | (c) Inversely proportional to H ^{3/2} | (d) Inversely proportional to $\mathrm{H}^{5/2}$ |
| 3. | The discharge over a rectangular not | ch is given by |
| | (a) $\frac{3}{2}C_d.L\sqrt{2gH}$ | (b) $\frac{2}{3}C_d.L\sqrt{2g} \times H$ |
| | (c) $\frac{2}{3}$ C _d . L. $\sqrt{2g} \times H^{3/2}$ | (d) $\frac{2}{3}$ C _d .L. $\sqrt{2g}$.H ^{5/2} |
| 4. | The maximum discharge over a broa | d crested weir is |
| | (a) 1.71 $C_d \cdot L \cdot \sqrt{2gH}$ | (b) 1.71 $C_d \cdot L \cdot \sqrt{2g} H$ |
| | (c) 1.71 $C_d \cdot L \cdot \sqrt{2g} \times H^{3/2}$ | (d) $1.71 \text{ C}_{\text{d}} \cdot \text{L} \cdot \text{H}^{3/2}$ |
| 5. | The upper surface of a weir over wh | nich water flows is known as |
| • | (a) nappe (b) crest | (c) edge (d) weir top |
| 6. | Cipolletti weir is a special type of | . weir. |
| | (a) triangular (b) trapezoidal | (c) stepped (d) all of above |
| | | SWERS |
| · | 1. (a) 2. (b) 3. (c) | 4. (d) 5. (b) 6. (b) |
| Δı | SET-10 : Comp | ressible Fluid Flow |
| 1. | The value of gas constant (R) is | |
| | (a) $0.287 \text{ J/kg}^{\circ}\text{k}$ (b) $2.87 \text{ J/kg}^{\circ}\text{k}$ | (c) 28.7 J/kg°k (d) 287 J/kg°k |
| 2. | When a gas is heated or expanded i | n such a way that the product of its pressure |
| | and volume remains constant, it is | called |
| | (a) isothermal process | (b) adiabatic process |
| | (c) isobaric process | (d) None of these |

3. The velocity of sound wave is given by

(a)
$$C = \sqrt{\frac{k}{RT}}$$
 (b) $C = \sqrt{\frac{1}{kRT}}$ (c) $C = \sqrt{kRT}$ (d) $C = k\sqrt{RT}$

| 1. (d) | 2. (a) | 3. (c) | 4. (b) | 5. (d) |
|---------|---------|--------|--------|---------|
| 6. (a) | 7. (b) | 8. (b) | 9. (b) | 10. (d) |
| 11. (d) | 12. (d) | | | |

SET-11

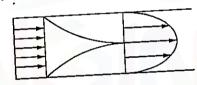
- When shear stress is applied to a substance it is found to resist it by static 1. deformation. The substance is a (d) uncertain
 - (a) liquid
- (b) solid
- (c) gas
- A fluid is said to be Newtonian when the shear stress is 2. directly proportional to the velocity gradient

 - inversely proportional to the velocity gradient (b)
 - independent of the velocity gradient (c)
 - None of these (d)

(GATE)

(GATE)

- Shear stress develops on a fluid element, if
 - The fluid is at rest The fluid container is subject to uniform linear acceleration (b)
 - The fluid is in viscid (c)
 - The fluid is viscous and the flow is nonuniform
- Bodies in flotation to be in stable equilibrium, the necessary and sufficient condition is that the centre of gravity is located below the
 - (a) centre of gravity (b) Centroid
- (c) metacentre
- (d) epicentre
- What can be definitely said about the tube flow in the diagram below? (GATE) 5.



- FIG. Q-5
- (a) Flow is turbulent
- Compressible flow
- Flow is laminar
- (d) Incompressible flow
- In which of the following arrangements would the vertical force on the cylinder due to water be the maximum?

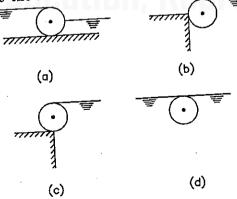


FIG. Q-6

Fluid Mechanics

397

If for a fluid in motion, pressure at a point is same in all directions, then the fluid is (GATE)

(a) a real fluid

(b) a Newtonian fluid

(c) an ideal fluid

(d) a non-Newtonian fluid

A vertical triangular plane area, submerged in water, with one side in the free surface, vertex downward and altitude h, has the pressure centre below the free surface by (GATE)

The unit of dynamic viscosity is

9.

(b) $\frac{}{3}$

(d) $\frac{}{2}$

(GATE)

(a) m²/sec

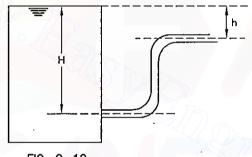
(b) N.sec/m²

(c) Pa.sec/m²

(d) $kg.sec^2/m^2$

10. The discharge velocity at the pipe exit in the given figure is

(GATE)



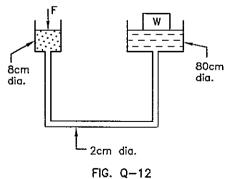
(a) $\sqrt{2}$ gH

(c) $\sqrt{2g(H+h)}$

FIG. Q-10

11. The centre of pressure of a liquid on a plane surface immersed vertically in a static body of liquid, always lies below the centroid of the surface area, because (GATE)

- in liquids the pressure acting is same in all directions (a)
- (b) there is no shear stress in liquids at rest
- the liquid pressure is constant over depth (c)
- the liquid pressure increases linearly with depth
- 12. A force F of 800 N is applied as shown, what maximum weight W can be supported ? Neglect the weight of the pistons. (NTPC)



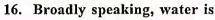
(a) 800 N

(b) 8000 N

(c) 40000 N

(d) 80,000 N

| 398 | Object | tive Civil Engineering |
|-----|---|--------------------------------|
| 13. | Water is flowing with a flow rate of 0.002 m ³ /s. What is its outlet where the area is 4 cm ² ? | average velocity at an (NTPC) |
| | (a) 50 m/s (b) 20 m/s (c) 10 m/s | (d) 5 m/s |
| 14. | For a fluid, the shear stress was found to be directly prangular deformation. The fluid is classified as (a) Newtonian (b) Non-Newtonian (c) Dilantant fluid | (NTPC) |
| 15. | A vertical square gate holds back water as shown in fig moment about the bottom edge is | ure given below. The (NTPC) |
| | $ \begin{array}{c c} \hline \hline \hline \hline \hline \hline \hline \hline \hline \hline \hline \hline \hline $ | (b) $\frac{\gamma L^3}{3}$ |



(GATE)

(d) $\frac{\gamma L^4}{6}$

(a) 10 times more compressible than steel

FIG. Q-15

- (b) 80 times more compressible than steel
- (c) 80 times less compressible than steel
- (d) 800 times less compressible than steel

17. A pathline is the

(GATE)

- (a) Mean direction of a number of particles at the same instant of time
- (b) Instantaneous picture of positions of all particles in the flow which passed a given point

(c) $\frac{\gamma L^4}{3}$

- (c) Trace made by a single particle over a period of time
- (d) Path traced by continuously injected tracer at a point

18. A floating body is in stable equilibrium

(Civil Services)

- (a) When its metacentric height is zero
- (b) When the centre of gravity of the body is below the centre of buoyancy
- (c) When its metacentre is above the centre of gravity of body
- (d) None of these

19. F μ = 0.06 poise, ρ = 0.9 gm/cm³, kinematic viscosity ν in stokes is

(Civil Services)

- (a) 0.04
- (b) 0.054
- (c) 0.067
- (d) 0.4
- 20. A worden plank (sp. gravity 0.5) 1 m × 1 m × 0.5 m floats in water with 1.5 kN load on it with 1 m × 1 m surface horizontal. The depth of plank lying below water surface shall be (Civil Services)
 - (a) 0.178 m
- (b) 0.250 m
- (c) 0.403 m
- (d) 0.50 m

| | (IES) |
|-----|--|
| 27. | The dimensions of kinematic viscosity are (a) $ML^{-1}T^{-2}$ (b) $ML^{-1}T^{-1}$ (c) LT^{-1} (d) LT^{-2} |
| | (8) MIC 1 - (0) 147D 7 (-) - |
| 28. | The dimensions of a pressure gradient in a fluid flow are (GATE) (a) $ML^{-1}T^2$ (b) $ML^{-3}T^{-2}$ (c) $ML^{-2}T^{-2}$ (d) $M^{-1}L^{-3}T^{-2}$ |
| | (a) $ML^{-1}T^2$ (b) $ML^{-3}T^{-2}$ (c) $ML^{-2}T^{-2}$ (d) $M^{-1}L^{-3}T^{-2}$ The point in the immersed body through which the resultant pressure of the liquid |
| 29. | The point in the immersed body through which the resultant probabilities (IES) |
| | may be taken to act is known as (a) Centre of gravity (b) Centre of buoyancy |
| | (a) Contro of gravity |
| | (c) Centre of pressure (d) metacentre If the weight of a body immersed in a fluid exceeds the buoyant force, then the |
| 30. | 11207 |
| | body will (a) rise until its weight equals the buoyant force |
| | t and it may finally sink |
| | |
| | (c) float |
| ~ 4 | (d) None of these Which one of the following pressure units represents the least pressure? |
| 31. | (b) mm of mercury (c) N/mm ² (d) kgi/cm |
| -S | the last 6 m × 6 m holds water on one side with the free surface at its top. |
| F | The moment about the hottom edge of the gate of the water love will be () |
| | enacific weight of water) |
| | (a) 18 y (b) 36 y (c) 72 y (d) 210 y |
| 33 | In a fluid flow, the line of constant piezometric head passes through two points |
| | which have the same |
| | (a) Elevation (b) pressure (c) Velocity (d) Velocity potential |
| 34 | (a) Elevation (b) pressure (All other conditions and parameters remaining the same, water hammer pressure (IES) |
| | can be reduced by |
| | (a) using pipe of greater diameter (b) using pipe of greater wall thickness |
| | (c) using a more elastic pipe (d) increasing the velocity of pressure wave |
| 35 | is. The flow of water in wash hand basin when it is being emptied through a central (IES) |
| | opening is an example of |
| | (a) free vortex (b) forced vortex (c) rotational vortex (d) Rankine vortex |
| 30 | (a) Hee voices (b) forest (c) forest (d) file (d |
| | loss in the jump is nearly (d) 0.45 m |
| | (a) 1.0 m (b) 0.9 m (c) 0.7 m |
| 3 | 7. There are four variables, namely |
| | E (A Quanto megarine a) |
| | g (acceleration due to gravity) μ (viscosity of water) |

| | They are associated with Mach, Euler, Froude and Reynolds numbers respectively, in |
|-----|---|
| | the order |
| | (a) E, p, μ , g (b) p, E, μ , g (c) p, E, g, μ (d) E, p, g, μ |
| 38. | The coefficient of velocity for an orifice is given by (using usual notations) (IES) |
| | (a) $\frac{x}{2\sqrt{yH}}$ (b) $\frac{2x}{\sqrt{yH}}$ (c) $\frac{x}{\sqrt{yH}}$ (d) $\sqrt{\frac{x^2}{2yH}}$ |
| | (a) $2\sqrt{yH}$ (b) \sqrt{yH} \sqrt{yH} \sqrt{yH} \sqrt{yH} |
| 39. | An object floats in water such that nine tenth of the object is submerged. What is |
| | its density? (d) 900 kg/m^3 |
| | (a) 600 kg/m^3 (b) 700 kg/m^3 (c) 800 kg/m^3 (d) 900 kg/m^3 |
| 40. | A real fluid is any fluid which (a) has surface tension and is incompressible (b) has zero shear stress |
| | (a) has surface tension and it |
| | (C) has constant viscosis, with |
| 41. | The moody diagram is used in fluid mechanics to obtain the (a) drag coefficient (b) strouhal number |
| | (d) Manning's constant n |
| | (c) friction factor (d) Manning's constant, if A model study is to be performed to investigate the problem of the pollution of |
| 42. | smoke emanating from the stacks on the deck of a passenger ship. A parameter |
| | smoke emanating from the stacks on the deek of a passenger of |
| | of primary concern in this study is the (a) Reynolds number (b) Mach number (c) Froude number (d) Weber number |
| 42 | ~//// |
| 43. | a click Click and unit of time |
| | (a) a volume of fluid flowing per unit of time (b) a volume fixed in space |
| | (c) The volume in which a control device is situated |
| | (d) the volume of the fluid controlling device |
| 44 | A model of reservoir is emptied in 10 minutes. If the model scale is 1:25, the |
| 7.5 | time taken by the prototype to empty itself, would be (1ES) |
| | (a) 250 minutes (b) 50 minutes (c) 6250 minutes (d) 2 minutes |
| 45 | . The ratio of inertia force to pressure force is called |
| | (a) Weber number (b) Froude number (c) Euler number (d) Prandtl number |
| 46 | An equipotential line |
| | (a) has no velocity component normal to it (b) has constant dynamic pressure |
| | (c) has no velocity component tangent to it (d) is the same as stream line |
| 47 | Existence of velocity potential implies that (GATE) |
| | (a) Fluid is in continuum (b) fluid is irrotational |
| | (c) Fluid is ideal (d) fluid is compressible |
| | |
| | |

| 48. | In a | flow field, the | stream line and | equipot | tential lines | | (| GATE) |
|------------|--|---|----------------------|---------|--------------------|-----------|-----------|-------------------|
| | (a) are parallel (b) are orthogonal anywhere in the flow field | | | | | | | |
| | (c) | (c) cut at any angle (d) cut orthogonally except at the stagnation point | | | | | | |
| 49. | For | For fully developed flow through a pipe, the ratio of the maximum velocity to the | | | | | | |
| | ave | rage velocity is | | | | | (| GATE) |
| | (a) | 1 | (b) 2 | (c) | $\sqrt{2}$ | (d) | 4 | • |
| 50. | The | path traced by | a single particle | of smo | ke issing from a | a cigare | tte is a | |
| | (a) | Stream line | (b) Streak line | (c) | Pathline | (d) | Flowli | ne |
| 51. | If the | he Froude numb | er of flow in an | open cl | hannel is more t | han 1, t | he flow | is said |
| | to b | e | • | | | (| Civil S | ervices) |
| | (a) | critical | (b) Shooting | (c) | Streaming | (d) | transiti | onal |
| 52. | Osc | illating hydrauli | ic jump is formed | l when | the Froude's nu | ımber i | s | (IES) |
| | (a) | 1.0 | (b) 1.5 | (c) | 2.0 | (d) | 5.0 | |
| 53. | Agi | ng of pipes impl | lies | | | | | (IES) |
| | (a) | relative roughne | ss decrease with tir | me | (b) pipe becomin | ig smoot | her with | n time |
| | (c) | increase in abso | olute roughness | | (d) decrease in | absolute | roughr | iess |
| 54. | A c | entrifugal pump | takes too much | power | due to | (| Civil S | ervices) |
| | (a) | low speed | (b) air in water | (c) | air leakage | (d) | heavy | liquid |
| 55. | If s | pecific speed of | a turbine is 6, th | en tur | bine should be | | | (IES) |
| | (a) | Francis | (b) Kaplan | (c) | Pelton wheel | (d) | Thoms | on |
| 56. | The | specific speed o | of a pump is defin | ned as | the speed of a p | ump of | such s | ize that (IES) |
| | (a) | With unit disch | arge at unit head | | (b) it requires u | nit powe | er for u | nit head |
| | (c) | it delivers unit o | lischarge at unit he | ad | (d) it delivers un | it discha | arge at a | ny head |
| 57. | A fo | oot value is prov | ided on | | | | | (IES) |
| | (a) | Centrifugal pun | np (b) Kaplan turb | ines (c |) Pelton wheels | (d) | all of t | these |
| 58. | If sp | pecific speed of | a turbine is 800, | then to | erbine should be | (Civil | Service | s) |
| | (a) | Francis | (b) Kaplan | (c) | Girard | (d) | Pelton | wheel |
| 59. | The | discharge throu | igh rectangular w | veir va | ries as | | | (IES) |
| | (a) | H | (b) $H^{1/2}$ | (c) | H^2 | (d) | $H^{3/2}$ | |
| 60. | The | hydraulic gradi | ient line is always | 8 | | | | (IES) |
| | (a) | below the total | energy line | (b) | parallel to the b | ottom | | |
| | (c) | above the total | energy line | (d) | none of these | | | |
| 61. | If a | centrifugal pun | ap is noisy in ope | ration | it may be due t | 0, | | (IES) |
| | (a) | Faulty priming | • | (b) | Suction head too | high | | |
| | (c) | air in water | | (d) | Mechanical defe | cf | | |

Fluid Mechanics

16.

(b)

17.

(c)

| 87. | . The locus of elevations that water will rise in a series of pitot tubes is called | | | | | | | |
|--|---|----------------|-----------|--------------------------|----------|---|------------|-------------|
| | | rgy grade line | | | | hydraulic grade li | | |
| | (c) pres | sure head | | | | velocity head | | |
| 88. The continuity equation is based on the principle of conservation of | | | | | | | | |
| | (a) mas | | (b) ener | | | momentum | | ne of these |
| 89. | In steady | flow of a f | luid, tot | al accele | eration | of any fluid part | | |
| | | be zero | | | | is never zero | *, | |
| | (d) is a | lways zero | | | (d) | is independent of | co-ordina | ates |
| 90. | The non- | -dimensinal | paramet | ter is | | <u>-</u> | | |
| | (a) From | ıde number | | | (b) | Darcy - Weisbach | n friction | factor |
| | (c) Che | zy's coefficie | nt | | (d) | Mach number | | |
| 91. | The non- | -dimensional | parame | eter is | | | | |
| | (a) Spec | cific weight | | | (b) | Manning's coeffic | ient n | |
| | (c) angu | ılar velocity | | | (d) | Specific gravity | | |
| 92. | In the Be | ernoulli's equ | uation, u | used in p | oipe flo | w each term rep | resents | |
| | | gy per unit v | | | | energy per unit m | | |
| | (c) ener | gy per unit v | olume | | (d) | energy per unit flo | ow length | |
| 93. | The relat | ion between | hydrau | lic coeff | icients | C _v , C _c and C _d is | given by | |
| | (a) $C_v =$ | | | | | $C_d = C_v \times C_c$ | | |
| | (c) $C_v =$ | $C_d + C_c$ | | | (d) | $C_d = C_v + C_c$ | | |
| 94. | For the m | ost economic | rectangi | ular chan | nel sec | tion having width = | b, water | depth = d, |
| | (a) b = | | | | | b = 1.5 d | (d) b = | |
| 95. | For the n | 10st economi | ic trapez | zo <mark>id</mark> al ch | annel | the relation betwe | en hydra | ulic radius |
| | | lepth of flow | | | | | | |
| | (a) R = | d (| (b) R = | 2d | (c) | $R = \frac{d}{2}$ | (d) R = | = 3d |
| 96. | At room | | | | | ematic viscosity of | f water | |
| | | oth greater t | | | | | | |
| | | oth less than | | | | | | |
| | (c) are r | espectively g | reater th | an and I | ess tha | n that of air | | |
| | | espectively le | | | | | | |
| | , , | | | <i>O</i> | | · · · · · · · · · · · · · · · · · · · | | |
| | | | | : ANS | WERS | : | | |
| | 1. (b) | 2. | (a) | 3. | (d) | 4. (c) | 5. | (c) |
| | 6. (b) · | 7. | (c) | 8. | (c) | 9. (b) | 10. | (b) |
| | 11. (d) | 12. | (d) | 13. | (d) | 14. (a) | 15. | · · |
| | 11. (u) | 12. | (a) | 15. | (a) | 14. (a) | 15. | (d) |

18. (c)

19. (c)

20. (c)

Objective Civil Engineering

| 21. (d) | 22. (b) | 23. (d) | 24. (a) | 25. (b) |
|---------|---------|---------|---------|---------|
| 26. (d) | 27. (c) | 28. (c) | 29. (c) | 30. (b) |
| 31. (c) | 32. (b) | 33. (a) | 34. (c) | 35. (a) |
| 36. (d) | 37. (d) | 38. (a) | 39. (d) | 40. (d) |
| 41. (c) | 42. (a) | 43. (b) | 44. (b) | 45. (c) |
| 46. (c) | 47. (b) | 48. (b) | 49. (b) | 50. (c) |
| 51. (b) | 52. (c) | 53. (a) | 54. (b) | 55. (c) |
| 56. (c) | 57. (a) | 58. (b) | 59. (d) | 60. (a) |
| 61. (d) | 62. (c) | 63. (a) | 64. (d) | 65. (a) |
| 66. (d) | 67. (d) | 68. (c) | 69. (c) | 70. (c) |
| 71. (a) | 72. (c) | 73. (b) | 74. (b) | 75. (a) |
| 76. (d) | 77. (d) | 78. (c) | 79. (d) | 80. (c) |
| 81. (a) | 82. (b) | 83. (a) | 84. (a) | 85. (c) |
| 86. (d) | 87. (a) | 88. (a) | 89. (a) | 90. (c) |
| 91. (d) | 92. (a) | 93. (b) | 94. (a) | 95. (c) |
| 96. (b) | | <u></u> | | |

EXPLANATIONS

12. (d) The pressure at a given height in a static fluid is constant. Hence, the pressure under both pistons is the same.

$$\therefore \frac{\frac{F}{A_1} = \frac{W}{A_2}}{\frac{\pi}{4} \times 8^2} = \frac{W}{\frac{\pi}{4} \times 80^2}$$

$$W = 80,000 \text{ N}$$

13. (d)
$$Q = AV$$

406

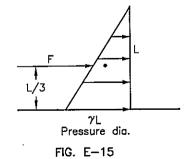
$$\therefore V = \frac{Q}{A} = \frac{0.002}{4 \times 10^{-4}} = 5 \text{ m/s}$$

15. (d) Gate is square

$$\therefore$$
 area of gate = L × L

Total force, $F = (\frac{1}{2} \cdot \gamma L \cdot L) \times length of gate$

$$F = \frac{1}{2} \cdot \gamma \cdot L^3$$



: Moment at base

$$M = \frac{1}{2} \gamma \cdot L^3 \times \frac{L}{3} = \frac{1}{6} \gamma L^4$$

19. (c)
$$v = \frac{\mu}{\rho} = \frac{0.06 \text{ Poise}}{0.9 \text{ gm/cm}^3} = 0.067 \text{ stokes}$$

OR

 $\mu = 0.06 \text{ Poise} = 0.006 \text{ N.s/m}^2$ $\rho = 0.9 \text{ gm/cm}^3 = 900 \text{ kg/m}^3$

$$v = \frac{\mu}{\rho} = \frac{0.006}{900} = 6.66 \times 10^{-6} \text{ m}^2/\text{s}$$

$$= 6.66 \times 10^{-6} \times 10^4 \text{ stokes}$$

$$= 0.067 \text{ stokes}$$

$$1 \text{ stoke} = 1 \times 10^{-4} \text{ m}^2/\text{s}$$

20. (c) Volume of plank = $1 \times 1 \times 0.5 = 0.5 \text{ m}^3$ density of plank = $0.5 \times 1000 = 500 \text{ kg/m}^3$

$$G = \frac{\gamma_s}{\gamma_w}$$

:. wt.of plank = $0.5 \times 500 = 250 \text{ kg}$

Additional wt = 1.5 kN =
$$\frac{1.5 \times 1000}{9.81}$$
 kg = 153 kg

- \therefore Total wt = 250 + 153 = 403 kg
- .. Total wt. of water displaced = 403 kg

$$\therefore \text{ vol. of plank immersed} = \frac{403}{1000} = 0.403 \text{ m}^3$$

$$\therefore$$
 depth = $\frac{\text{vol.}}{\text{Area}} = \frac{0.403}{1 \times 1} = 0.403 \text{ m}$

28. (c) Pressure gradient = pressure intensity per unit length $= \frac{\text{dimension of pressure intensity}}{\text{dimension of length}}$ $= \frac{\text{ML}^{-1} \text{ T}^{-2}}{\text{I}} = \text{ML}^{-2}\text{T}^{-2}$

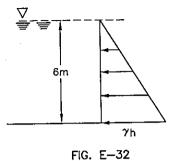
32. (d) Total force on gate

$$F = \frac{1}{2} \cdot (\gamma h) \cdot h \times length \text{ of gate}$$

$$= \frac{1}{2} \times \gamma \times 6 \times 6 \times 6$$

$$= 108 \gamma$$

$$\therefore \text{ Moment at base} = 108 \ \gamma \times \frac{1}{3} \times 6 = 216 \ \gamma$$



36. (d)
$$h_L = \frac{(y_2 - y_1)^3}{4y_1y_2} = \frac{(1.4 - 0.4)^3}{4 \times 0.4 \times 1.4} = 0.446 \text{ m} \approx 0.45 \text{ m}$$

38. (a)
$$C_v = \sqrt{\frac{x^2}{4yH}} = \frac{x}{2\sqrt{yH}}$$

39. (d) density of object = density of water
$$\times \frac{9}{10}$$

= $1000 \times \frac{9}{10}$
= 900 kg/m^3

44. (b)
$$T_r = \frac{T_p}{T_m}$$
 but $T_r = \sqrt{L_r} = \sqrt{25}$

$$=\frac{25}{1}$$

$$=25$$

$$\therefore 5 = \frac{T_p}{10} \qquad \therefore T_p = 50 \text{ minutes}$$

51. (b)
$$F_r = 1$$
 critical flow $F_r < 1$ Subcritical flow (Streaming) $F_r > 1$ Super critical flow (Shooting)

85. (c)
$$h^* = \frac{l_g}{A\overline{h}} + \overline{h}$$

= $\frac{bh^3/36}{\frac{1}{2}bh \times \frac{h}{3}} + \frac{h}{3} = \frac{h}{6} + \frac{h}{3} = \frac{h}{2}$

18.

Building Materials

MCQ'S

SET-1

| 1. | Gypsum is a | |
|-----------|---|-------------------------------|
| | (a) mechanically formed sedimentary rock | |
| | (b) metamorphic rock | |
| | (c) chemically precipitated sedimentary rock | |
| | (d) igneous rock | |
| 2. | Granite is not suitable for ordinary building | g purpose because |
| | (a) it can not be polished (b) | it is not fire proof material |
| | (c) it is costly (d) | it has less crushing strength |
| 3. | Which of the following trees yields hard we | ood ? |
| • | (a) deodar (b) chir (c) | shishum (d) pine |
| ١. | In which of the following pairs both trees | yield soft wood ? |
| r. | (a) deodar and shishum (b) | chir and sal |
| | (a) deodar and since | chir and deodar |
| | Which of the following timbers is suitable | |
| 5. | | mahogany (d) deodar |
| | (a) mulberry (b) sal (c) In which of the following directions the tensile st | _ |
| 5. | In which of the following directions the tensite se | tong or transfer or the |
| | (a) Parallel to grain (b) 45° to grains | Same in all direction |
| | (c) 1 ciponaloular to gramm | |
| 7. | The moisture content in a well seasoned tin | 15% to 20% (d) 100% |
| | (4) 4/0 (0 0/0 | |
| 3. | The trunk of tree left after cutting all the | branches is known as |
| | (a) log (b) sattern | plank (d) baulk |
| €. | The age of a tree can be known by examin | ning |
| | (a) cambium layer (b) annular rings (c) med | lullary rays (d) heart wood |
| 10. | 1.C. C | |
| | (a) less than one year (b) | 1 to 5 years |
| | | more than 10 years |
| | | |

Objective Civil Eng. \ 2016 \ 52

| 11. | A first class brick when water more then | immersed in cold | water for 24 hrs s | should not absorb | | |
|-----------|--|---------------------|----------------------------------|--------------------------|--|--|
| - | (a) 15% (b) 2 | 0% (c) | 22% | (d) 25% | | |
| 12. | Crushing strength of the | ` ′ | | then | | |
| - | (a) 3.5 N/mm^2 (b) 7 | | | (d) 14 N/mm ² | | |
| 13. | The percentage of alumi | | k earth lies betwe | en | | |
| | | 0 to 30% (c) | | (d) 70 to 80% | | |
| 14. | The nominal size of the modular brick is | | | | | |
| | (a) 190 mm × 90 mm × | 80 mm (b) | 190 mm × 190 г | nm × 90 mm | | |
| | (c) 200 mm × 100 mm > | (d) x 100 mm | 200 mm × 200 r | nm × 100 mm | | |
| 15. | The process of mixing cla | | | | | |
| | (a) kneading (b) n | | pugging | (d) drying | | |
| 16. | The frog of the brick in | a brick masonry | is generally kept | on | | |
| | (a) bottom face (b) to | | | (d) longer side | | |
| 17. | No. of bricks required f | for our cubic metro | e of b <mark>ric</mark> k majori | ty is | | |
| | (a) 400 (b) 4 | | 500 | (d) 550 | | |
| 18. | Quick lime is | | | | | |
| | (a) calcium carbonate | (b) |) calcium oxide | | | |
| | (c) calcium hydroxide | (d) | none of the above | /e | | |
| 19. | The main ingredients of | port land cement | are | | | |
| | (a) lime & silica | (b) |) lime & alumina | | | |
| | (c) silica & alumina | (d) |) lime & iron | | | |
| 20. | For testing compressive | strength of cemen | t the size of cube | used is | | |
| | (a) 90 mm (b) 7 | 0.6 mm (c) |) 100 mm | (d) 150 mm | | |
| 21. | The slump recommende | d for mass concret | e is about | | | |
| | (a) 25 mm to 50 mm | (b) |) 50 mm to 100 n | nm | | |
| | (c) 100 mm to 125 mm | |) 125 mm to 150 | | | |
| 22. | For testing compressive | | | ent morter is made | | |
| | by mixing cement & sta | | | | | |
| | | ` ' |) 1 : 4 | (d) 1 : 6 | | |
| 23. | Which of the following co | | | | | |
| | (a) ordinary portland cen | • | (b) low heat cement | | | |
| | (c) rapid hardening ceme | • | (d) sulphate resisting cement | | | |
| 24. | Which of the following | | | | | |
| | | | , | high carbon steel | | |
| 25. | The amount of water u | | | | | |
| | (a) 0.2 litre (b) (| 0.4 litre (c |) 0.6 litre | (d) 0.8 litre | | |
| | | | | | | |

| 26. | The Distemper is used to coat | · | | |
|-------------|---|-----------------------|-------------------|--|
| | (a) external concrete surface (b) interi- | or surface not expose | ed to weather | |
| | (c) wood work (d) comp | (d) compound walls | | |
| 2 7. | Cement used in underwater construction | work is | | |
| | | o) Quick Setting Cer | ment | |
| ٠ | | d) Rapid hardening | cement | |
| 28. | Calcination is the process of heating the | limestone with | | |
| | | c) Air | (d) Cement | |
| 29. | The lime specifically used for white wash | ing is | | |
| | (a) Fat lime (b) Poor lime (c) | c) Hydraulic lime | (d) Colour lime | |
| 30. | Commonly used raw material in the man | ufacture of cement | is | |
| | | c) lime stone | (d) Rock | |
| 31. | Bulking of sand is caused because of | | | |
| | (a) Voids (b) Surface moisture (c |) Clay (d) | None of above | |
| 32. | Which lime contains high percentage of | calcium oxide? | | |
| | | c) Fat lime | (d) All of above | |
| 33. | Lime morter is generally made with | | | |
| | (a) Fat lime (b) hydraulic lime (| c) Quick lime | (d) None of above | |
| 34. | Which lime Slack with water and mainly | contains calcium | oxide? | |
| | (a) Poor lime (b) hydraulik lime (| c) Quick lime | (d) Fat lime | |
| 35. | Strength to cement is provided by | | | |
| | | b) Tri - Calcium S | ilicate | |
| | (c) Tri - Calcium aluminate | (d) Tri - Calcium a | lumino femite | |
| 36. | PCC means | | | |
| | (a) Plain Cement Concrete | (b) Part of Cement | | |
| | (c) Tapor Coment Control | (d) None of above | | |
| 37. | To reduce the weight of concrete followi | ng which material | is used? | |
| | | (c) Rock | (d) All the above | |
| 38. | Cement concrete is a mixture of | | | |
| | (a) cement, sand and gravel | (b) water, cement as | nd gravel | |
| | (c) sand, water and cement | (d) None of above | | |
| 39. | In concrete proportion 1:3:6, where | 3 indicating | | |
| | | (c) Water | (d) Aggregate | |
| 40. | Full Form of R. C. C. is | | | |
| | | (b) Road Cement C | oncrete | |
| | | (d) Rapid Cement C | Concrete | |

Minimum of 40% of iron is available

(a) Obtained by purifying pig iron.

(b) Black bard

(c) Sidenite

54.

55.

(a) Magnetite

Cast iron is

(d) Limonte

| | (b) May contain 2 to 5 percent of carbon | n with other impurities. |
|------------|--|---|
| | (c) Manufactured in required shape. | |
| | (d) All the above. | |
| 56. | Shale is which type of rock? | |
| | (a) Igneous (b) Sedimentary | (c) Plutonic (d) Meta morphic |
| 57. | Marble is which type of rock? | |
| | (a) Plutonic (b) Sedimentary | (c) Igneous (d) Metamorphic |
| 58. | Frog is which portion of brick? | |
| | (a) Front (b) Back | (c) Bottom (d) Top |
| 59. | Basalt can be classified as | |
| | (a) Metamorphic rock | (b) Sedimentary rock |
| | (c) Extrusive igneous | (d) Intrusive igneous |
| 60. | Slate belong to | |
| | | (c) Foliated rock (d) Metamorphic rock |
| 61. | The indentation provided in a face of | |
| | (a) Strike (b) Frog | (c) Pallet (d) None of above |
| 62. | The standard size of brick as per Indi | |
| | (a) $20 \times 10 \times 10$ cm | (b) $19 \times 9 \times 9$ cm |
| | (c) $24 \times 12 \times 6$ cm | (d) 18 × 9 × 9 cm |
| 63. | The bricks manufactured to withstand | |
| | (a) First class bricks | (b) Second class bricks |
| | (c) Refractory bricks | (d) None of the above |
| 64. | The red colour obtained by the bricks | |
| | (a) Lime (b) Iron Oxide | |
| 65. | When a brick is cut into two haves lo | |
| | (a) King closer (b) Voussoir | (c) Cornice brick (d) Queen closer |
| 66. | Asphalt is obtained from | (I) Block Distillation |
| | (a) Bitumen Distillation | (b) Plastic Distillation |
| | (c) Petroleum Distillation | (d) None of above |
| 67. | Bullet Proof glass is made with | (IN XII'sh test pleatic |
| | (a) Steel | (b) High test plastic |
| | (c) Stainless Steel | (d) All the above |
| 68. | Bitumen may be dissolved in | () Q 12 Q 11 11 () Q d l an diminida |
| | (a) Water (b) Carbon dioxide | (c) Sodium Chloride (d) Carbon disulphide |
| 69. | Plastic asphalt is | () The December of the Alberta Street |
| | (a) Elastic (b) Water Proof | (c) Fire Proof (d) all the above |
| | | · |

Building Materials

| | | 20 () | | 00 () |
|---------|---------|---------|---------|---------|
| 26. (b) | 27. (b) | 28. (c) | 29. (a) | 30. (c) |
| 31. (b) | 32. (d) | 33. (b) | 34. (c) | 35. (a) |
| 36. (a) | 37. (b) | 38. (a) | 39. (b) | 40. (c) |
| 41. (d) | 42. (c) | 43. (d) | 44. (c) | 45. (a) |
| 46. (b) | 47. (b) | 48. (d) | 49. (c) | 50. (d) |
| 51. (a) | 52. (a) | 53. (d) | 54. (b) | 55. (d) |
| 56. (b) | 57. (d) | 58. (d) | 59. (c) | 60. (d) |
| 61. (b) | 62. (b) | 63. (c) | 64. (b) | 65. (d) |
| 66. (c) | 67. (b) | 68. (d) | 69. (d) | 70. (c) |
| 71. (d) | 72. (c) | 73. (a) | 74. (c) | 75. (d) |
| 76. (a) | 77. (a) | 78. (a) | 79. (b) | 80. (d) |
| 81. (a) | 82. (b) | | | |

SET-2

| 1. | Granite is an exam | nple of | | | |
|-----|--------------------|------------------------|------------|--------------------|----------------------|
| | (a) Igneous rocks | (b) Sedimentary ro | cks (c) | Metamorphic rocl | ks (d) aqueous rocks |
| 2. | Marble is an exar | nple of | | | |
| | (a) Igneous rocks | (b) Sedimentary ro | cks (c) | Metamorphic rock | ks (d) aqueous rocks |
| 3. | The colour of gra | nite is | | | |
| | (a) grey | (b) green | (c) | brown | (d) all of these |
| 4. | Bituminous paint | consists of bitumen d | lissolved | in | |
| | (a) Spirit | (b) naptha | (c) | linseed oil (d) | either (a) or (b) |
| 5. | The base in a pai | nt is added to | | | 7200 |
| | (a) Improve the o | uality of paint | (b) | Make smooth sur | face |
| | (c) hide the surfa | ce to be painted | (d) | all of these | |
| 6. | A good quality st | one must absorb wate | er less th | ıan | |
| | (a) 2.5 % | (b) 5 % | (c) | 10 % | (d) 20 % |
| 7. | In stone masonry | the direction of pres | sure line | is | , |
| | (a) Parallel to na | tural bed | (b) | Perpendicular to | natural bed |
| | (c) inclined to na | tural bed at 30° | (d) | inclined to natura | ıl bed at 45° |
| 8. | Good quality buil | ding stones should no | ot contai | n soluble salts me | ore than |
| | (a) 0 % | (b) 1 % | | 2 % | (d) 3 % |
| 9. | For stones Mhos | scale is used to deter | mi 🌼 | | |
| | (a) toughness | (b) hardness | (c) | flakiness index | (d) durability |
| 10. | | sed for railway ballas | | | |
| | • | (b) dolomite | | marble (d) bas | alt or trap basalt |

415

Building Materials

| 27. | Purest form of iron | is | • | |
|-----|----------------------|------------------------|-------------------------|----------------------|
| | (a) mild steel | (b) cast iron | (c) high carbon steel | (d) wrought iron |
| 28. | In railway tracks m | etal used is | | |
| | (a) cast iron | (b) Wrought iron | (c) mild steel | (d) Stainless steel |
| 29. | Basalt and trap are | used in | | |
| | (a) roads (b) | dams (c) orname | ntal works (d) monun | nental works |
| 30. | Fire bricks are used | l | | |
| | (a) to reflect heat | | (b) to increase heat | flow |
| | (c) to decrease heat | flow | (d) to protect building | ng against lighting |
| 31. | Annual rings of the | tree represent its | | |
| | (a) life | (b) strength | (c) density | (d) type |
| 32. | The paints used on | aircraft structure are | | |
| | (a) Oil paints | (b) dry paints | (c) emulsion paints | (d) cellulose paint |
| 33. | Asphalt is good inst | ulator of | | |
| | (a) heat | (b) sound | (c) electricity | (d) all of the above |
| 34. | Acoustical material | | | |
| | (a) absorb sound | (b) reflect sound | (c) create sound | (d) increase sound |
| 35. | Asphalt is a | | | |
| | (a) Pure bitumen | | (b) impure bitu | |
| | (c) Mixture of bitu | men and inert material | (d) by-product | of bitumen |
| | | | | |

: ANSWERS :

| 1. (a) | 2. (c) | 3. (d) | 4. (d) | 5. (c) |
|---------|---------|---------|---------|---------|
| 6. (b) | 7. (b) | 8. (a) | 9. (b) | 10. (b) |
| 11. (a) | 12. (a) | 13. (a) | 14. (b) | 15. (d) |
| 16. (a) | 17. (c) | 18. (c) | 19. (d) | 20. (c) |
| 21. (d) | 22. (d) | 23. (d) | 24. (b) | 25. (c) |
| 26. (a) | 27. (d) | 28. (c) | 29. (a) | 30. (c) |
| 31. (a) | 32. (d) | 33. (d) | 34. (a) | 35. (c) |
| | | | | |



Building Construction

MCQ'S

| | <u>SET-1:</u> | <u>Shallo</u> | w Foun | dations | • | |
|----|--|---------------|--------------------------------|-------------|-------------|---------------|
| 1. | Which type of foundation is mos (a) Raft foundation (b) Strip | - | ed for founce) Under re | | | |
| 2. | Which of the following is not | a shallo | w foundati | on? | | |
| | (a) Grillage foundation | | (b) Well f | foundation | | |
| | (c) Combined footing | | (d) Mat fo | oundation | | |
| 3. | Raft foundation is not suitable | e | | | | |
| | (a) when the structure loads are | heavy. | | | | |
| | (b) when columns and walls are | close to | each othe | r. | | |
| | (c) when there is large variation | n in the | loads on in | dividual co | lumns. | |
| | (d) none of these | | | | | |
| 4. | When there are chances of diff | erential : | settlements, | , the found | ation prefe | erred is |
| | (a) spread footing (b) wall foot | ing | (c) raft fo | oundation (| d) grillage | foundation |
| 5. | For lowering of water table b | y about | 10 m the f | following n | nethod is | generally the |
| | most suitable. | | | | | |
| | (a) Well Point method | | (b) Shallo | w Well me | ethod | |
| | (c) Deep Well method | | (d) Electr | o-osmosis | method | |
| 6. | Vacuum method of well points | s is gene | rally used | for draini | ng / | |
| | (a) coarse sands | | (b) fine s | ands and s | ilty sands | |
| | (c) silts | | (d) clays | | | |
| 7. | Electro-osmosis for a clayey s | oils gene | erally leads | s to | | |
| | (a) decrease in shear strength | | (b) increase in shear strength | | | |
| | (c) increase in water content | | (d) increase in plasticity | | | |
| 8. | In a Shallow Well System, the | suction | lift is usu | ally limite | d to | |
| | (a) 5 m (b) 10 m | - | (c) 15 m | | (d) 20 |) m |
| | | : ANS | WERS: | | | |
| Г | 1 (a) 2 (b) 2 (d) | 4 (a) | 5 (0) | 6 (b) | 7 (b) | 8 (b) |

| 1. (c) | 2. (b) | 3. (d) | ` ` ` | 5. (a) | 6. (b) | 7. (b) | 8. (b) |
|--------|--------|--------|-------|--------|--------|--------|--------|
|--------|--------|--------|-------|--------|--------|--------|--------|

SET-2: Masonry Construction

| | The brick laid with its length par | rallel to the face of the v | vall, is known as |
|----|--|---|-----------------------------|
| 1. | /1 \ _44_66.0° | TELEIUSEL | • • |
| | (a) header (b) stretcher The brick laid with its breadth p | orollal to the face of the | wall, is known as |
| 2. | The brick laid with its breadth p | (c) closer | (d) none of these |
| | (a) header (b) stretcher | (c) closer | generally known as |
| 3. | (a) header (b) stretcher The 9 cm × 9 cm side of a brick a | as seen in the wan ruce, | (d) header |
| | /1 \ C-+n | 164 110111 | ` ' |
| 4. | (a) stretcher (b) face The 19 cm × 9 cm side of a bric | k as seen in wan face, is | (d) header |
| | | | |
| 5. | (a) stretcher (b) face The position of a brick when la | id on its side 19 cm × 9 | Citi With its 110g |
| | vertical plane, is called | | d) brick held vertically |
| | (1) brick on edge (b) brick on | end (c) brick on bed (c | d) Brick held versions |
| 6. | The portion of a brick cut acros | ss the width is called | (d) bat |
| | (b) half brid | ck (c) bed | (4) 541 |
| 7. | The portion of a brick cut acros | ss the length is called | (d) quoin |
| ′• | (a) king closer (b) queen c | loser (c) bat | (a) quom |
| 8. | To obtain good bonding in brief | k masonry | |
| 0. | () first along bricks are used. | | |
| | (b) vertical joints in alternate con | urses are kept in plumb lir | ie. |
| | (c) bats are used where necessary | y | |
| | (d) all the above. | | |
| 0 | Il to senerally provide | ed for | |
| 9. | (a) heat insulation | (b) sound msurat | |
| | ()ontion of dampness | (d) all the above | |
| | To construct a 10 cm thick par | rtition wall, you will pre | fer |
| 1 | | | |
| | - c hand in which eve | ery course contains both | headers and stretchers is |
| 1 | 1. The type of bond in which ever called | | a t ta.d |
| | to Classic | sh bond (c) Header bond | (d) Stretcher bond |
| | a my time of hand in which alt | ernate courses are stretc | her and header is |
| | | | |
| | (a) English bond (b) Flemis 3. Exposed portions of a vertical | surface at right angles to | the door or window frame, |
| | 3. Exposed portions of a verticus | | |
| | are known as: (a) Jambs (b) lintels | (c) Reveals | (d) Soffits |
| | (a) Jambs (b) linters 14. The vertical sides of door and | window openings provid | led in a wall, are known as |
| | | s (c) Reveals | (d) Soffits |
| | (a) Jambs (b) linter | • | |

| 15. | The header bond is | generally used for | . 1 | |
|-------------|------------------------------|---------------------|--|----------------------|
| | (a) half brick wall | (b) single brick-wa | all (c) $1\frac{1}{2}$ brick wal | (d) arches |
| 16. | The stretchar bond | is generally used f | for | |
| | (a) half brick wall | (b) simple brick w | vall (c) $1\frac{1}{2}$ brick wal | (d) arches |
| 17. | Herringbone bond i | | | |
| | (a) walls having thic | | bricks | |
| | (b) Architectural fini | sh to the face work | 3 | |
| * | (c) ornamental panel | s in brick flooring | | |
| , | (d) all the above | | | |
| 18. | The stone masonry | in which stones of | f same height are laid | in layers, is called |
| | (a) random rubble m | asonry | (b) course rubble mas | onry |
| | (c) uncoursed rubble | masonry | (d) ashlar masonry | • |
| 19. | The stone masonry | of finely dressed | stones laid in cement o | or lime is |
| | (a) random rubble m | asonry | (b) course rubble mas | sonry |
| | (c) uncoursed rubble | masonry | (d) ashlar masonry | 11 14 |
| 20. | The thickness of a | | artition wall, is genera | (d) 20 cm |
| | (a) 5 cm | (b) 10 cm | ` ' | (d) 20 cm |
| 21. | The triangular porti | | jacent arches and the ta | (d) rise |
| | (a) haunch | (-) -I | (c) soffit | (d) Tise |
| 22. | An arch may fail d | | (b) sliding of vousso | ire |
| | (a) crushing of mate | | // Y/ 1\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ | 113 |
| | (c) differential settle | ement of abutments | to receive the arch is | known as : |
| 23. | | | to receive the arch is (c) spandrill | (d) haunch |
| | (a) skewback | (b) soffit | - | 3 |
| 24 | The voussoir place | | (c) springer | (d) haunch |
| | (a) key | (b) soffit | | |
| 25 | The under surface (a) soffit | (b) intrados | (c) extrados | (d) back |
| 26 | The engle between | en skewhack of | a flat arch and the | horizontal, is kept |
| 20 | approximately equ | | | |
| | (a) 0° | (b) 30° | (c) 60° | (d) 90° |
| 27 | . The depth of an a | , , | between | |
| 24 / | (a) ground level an | | (b) crown and spring | ging line |
| | (c) crown and grou | | (d) intrados and extr | rados |
| 28 | | | wall is normally kept | |
| _~ | (a) 5 cm | (b) 10 cm | (c) 15 cm | (d) 20 cm |
| | | | | |

: ANSWERS:

| 1. (b), | 2. (a), | 3. (d), | 4. (a), | 5. (a), |
|----------|----------|----------|----------|----------|
| 6. (d), | 7. (b), | 8. (d), | 9. (d), | 10. (d), |
| 11. (b), | 12. (a), | 13. (c), | 14. (a), | 15. (d), |
| 16. (a), | 17. (d), | 18. (b), | 19. (d), | 20. (b), |
| 21. (b), | 22. (d), | 23. (a), | 24. (a), | 25. (a), |
| 26. (c), | 27. (d), | 28. (b). | | , |

<u>SET-3</u>

| | • | | | | |
|----|--------------------------------------|-----------|-------------------|---------------------|-------|
| 1. | The moisture content in a well seas | soned ti | mber is | | (IES) |
| | (a) 4 to 6 % (b) 10 to 12 % | (c) | 15 to 20 % | (d) 100 % | ` , |
| 2. | The slenderness ratio for masonry | walls sh | ould not be mo | re than | |
| | (a) 10 (b) 20 | (c) | | (d) 40 | |
| 3. | Neoprine is suitable for use in | | | | (IES) |
| | (a) bearing of bridges | (b) | hard duty rubbe | er coatings of floo | ors |
| | (c) joinery work | (d) | Floors for dance | _ | |
| 4. | Timber can be made more fire resi | istant by | y | | (IES) |
| | (a) Dipping and steeping process | (b) | Sir Abel's proc | ess | |
| | (c) Charring | (d) | hot and cold op | en tank treatmen | t |
| 5. | Distemper is used to coat | | | | (IES) |
| | (a) external concrete surfaces (| b) inte | rior surfaces not | exposed to weath | ier |
| | (c) Wood work (| | npound walls | | |
| 6. | Putty is made up of | - | | | (IES) |
| | (a) White lead and turpentine | (b) | Powdered chalk | and raw linseed | oil |
| | (c) red lead and linceed oil | (d) | zinc oxide and | boiled linceed oil | |
| 7. | Limit of proportionality is applicab | le more | in the case of | | (IES) |
| | (a) concrete (b) wood | (c) | cast iron | (d) mild stee | el |
| 8. | The expansion and shrinkage of ply | wood a | re comparatively | very low as (II | ES) |
| | (a) They are held in position by add | hesives | | | |
| | (b) They are glued under pressure | | | | |
| • | (c) Piles are placed at right angles | to each | other | | |
| | (d) they are prepared from veneers | | | • | • |
| 9. | Seasoning of timber is required to | | | | (IES) |
| | (a) Soften the timber | (b) | harden the timb | er | |
| | (c) Straighten the timber | (d) | Remove san from | m the timber | |

| 2 | Objective Civil E | (IES) |
|---|---|-----------------|
| . The important purpose of frog in a | brick is to | (1220) |
| (a) emboss manufacturer's name | | |
| (b) reduce weight of brick | | |
| (c) form keyed joint between brick a | and mortar | |
| (d) Improve insulation by providing | nonows . | (IES) |
| . Surkhi is added to lime mortar to | (b) decrease setting time | |
| (a) Prevent shrinkage | (d) impart hydraulicity | • |
| (c) increase bulk | • • | |
| 2. Before testing setting time of cemer | | onsistency |
| (a) soundness (b) strength 3. Match list I and List II and select co | arrect answer from the codes given | below lists: |
| 3. Match list I and List II and select & List I | List II | |
| (Component of scaffoldin | g) (Function) | |
| | I. Diagonal momen | |
| | 2. Vertical member | |
| B. Ledger C. Brace | 3. Horizontal member | |
| D. Standard | 4. Transverse member | (TIES) |
| Codes: | | (IES) |
| A B C D | | |
| (a) 4 3 1 2 | | |
| (b) 4 3 2 1 | | |
| (c) 3 4 1 2 | | |
| (d) 3 4 2 1 | | (IES) |
| 14. Polyvinyl chloride (PVC) is a | a lastic motorial | (1-1) |
| (a) thermosetting material | (b) thermoplastic material (d) rigid - plastic material | |
| | (d) rigid - plastic material | days and 28 |
| (c) elastoplastic material 15. The approximate ratio between s | strengths of cement concrete us | (IES) |
| days is | | 72(11/2 |
| (a) $\frac{3}{4}$ (b) $\frac{2}{3}$ | (c) $\frac{1}{2}$ (d) | <i>)</i> |
| (a) $\frac{1}{4}$ (b) 3 16. The approximate ratio of strengt | h of 15 cm × 30 cm concrete cyli | nder to that o |
| 16. The approximate ratio of strengt | 18 | |
| 15 cm cube of the same concrete (a) 1.25 (b) 1.0 | (c) 0.85 (d) | 0.50 · |
| (4) | | (IES) |
| 17. What is efflorescence? | on the brick surface due to insolu | ble salts in th |
| | | |
| brick clay | | |

| 27. | Maximum differential settlement, in case of foundations on clayey soils is limited to |
|---|--|
| | (a) 40 mm (b) 60 mm (c) 100 mm (d) 400 mm |
| 28. | Piles are usually not made of |
| | (a) steel - (b) Stainless steel (c) timber (d) RCC |
| 29. | In machine foundations the material used for preventing transmission of vibration is |
| | (a) Cork or rubber (b) lead sheets (c) felt (d) all of these |
| 30. | Black cotton soil is unsuitable for foundations because of its |
| | (a) low bearing capacity |
| | (b) uncertain permeability |
| | (c) Cohesive property |
| | (d) Property to undergo a volumetric change due to variation of moisture content |
| 31. | Raft foundation is generally used, when the area required for individual footing |
| | is more than |
| | (a) 25 % of total area (b) 30 % of total area |
| | (d) 50 % of total area (d) 50 % of total area |
| 32. | Maximum total settlement of raft on sand is usually limited to (a) 65 mm (b) 100 mm (c) 150 mm (d) 200 mm |
| | (a) 05 mm |
| 33. | Maximum total settlement of raft on clayey soils is usually limited to (a) 65 mm (b) 100 mm (c) 150 mm (d) 200 mm |
| 2.4 | (a) 05 mm |
| 34. | The depth of excavation for foundation is usually checked with a (a) tape (b) ranging rod (c) levelling staff (d) boning rod |
| 25 | (a) tape (b) ranging rod (c) levelling starr (d) boning rod Minimum thickness of a wall in stone masonry can not be less than |
| 33. | (d) 50 am |
| 26 | (a) 10 cm (b) 20 cm (c) 35 cm (d) 50 cm In a single day, maximum height of brick masonry to be raised should not b |
| 30. | more than |
| | (a) 0.5 m (b) 1.0 m (c) 1.5 m (d) 2 m |
| 37 | While laying bricks in a wall, frog of the brick must be kept |
| <i>-</i> | (a) upward (b) downward (c) sideward (d) in any position |
| 38. | The external corner in brick masonry is called |
| | (a) Jamb (b) quoin (c) sleeper wall (d) parapet |
| 39. | The part of a wall at the side of an opening in the masonry is called |
| • | (a) quoin (b) Jamb (c) sleeper wall (d) parapet |
| 40. | Expansion joints in masonry walls are provided in wall lengths more than |
| | (a) 10 m (b) 20 m (c) 30 m (d) 40 m |
| 41. | and the second s |
| | (a) 10 mm (b) 20 mm (c) 30 mm (d) 40 mm |
| | |

| Building Construction | water, it is provided |
|---|-------------------------|
| 42. To protect the top of a parapet wall from the action of rain | 1 |
| with a special course called | (d) coping |
| | |
| (a) drip course (b) cornice (c) string course 43. A semitight material which forms an excellent impervious lay | |
| is called (b) bituminous felt (c) mastic asphalt | (d) aluminal |
| 44 In residential buildings, the D.P.C. is provided at | (d) parapet level |
| (a) ground level (b) Plinth level (c) sin 1975 | • • • |
| 45. D.P.C. may be (a) Vertical (b) Horizontal (c) both (a) and (b) | (d) none of these |
| The maximum size of the aggregate used in a D.P.C. is and | ut |
| (b) 11 mm | |
| (a) 6 mm (b) 10 mm (c) 47. Portion of the wall left on the face of the door/window is c | alled |
| - (b) reveal (c) 3111 | (d) all of these |
| (a) Jamb (b) reveal 48. Vertical member used in a door frame is called | |
| (b) still (c) lan | (d) style |
| (a) Post (b) Still (c) rail 49. The bottom horizontal member of a window frame is calle | d |
| | |
| (a) transom (b) sill (c) fair 50. The vertical member of a frame which is employed to su | ıb-divide a window or |
| 50. The vertical member of a mainte | |
| door vertically is called (a) style (b) horn (c) mullion | (d) transom |
| (a) style (b) norm (c) interest (c) 1. The horizontal member of a frame, which is employed to | o sub-divide a window |
| 51. The horizontal member of a reason | |
| opening horizontally is called (a) style (b) horn (c) mullion | (d) transom |
| (a) style (b) norm (c) member of door or wind (c) post | low frame is called |
| | |
| (a) Head (b) horn (c) post 53. Timber or glass pieces fixed in inclined position in a frame of v | entilator are called |
| 53. Timber or glass pieces fixed in incincu position in the styles (b) posts (c) louvers | (d) Sash |
| (a) styles (b) posts (c) louvers 54. The window which projects outward from the walls of a ro | om to provide increased |
| 54. The window which projects outward from the management of opening for admitting more light and ventilation | is called |
| area of opening for admitting more light and | ђеаd |
| (a) dormer window | ham |
| (a) dormer window transo | m r——— horn |
| (b) corner window | post |
| mullion | |
| (c) bay window | → |
| (d) clearstorey window window | N sill |
| Objective Civil Eng. \ 2016 \ 54 | • |

| 55. | | | | e mai | n roof of a room a | nd o | pens above the |
|-----|------|-------------------|----------------------|--------|-----------------------|--------|------------------|
| | adjo | ining verandah | is called | | | | |
| | (a) | dormer window | (b) Corner window | w (c) | bay window (d) cle | ar st | orey window |
| 56. | A w | indow provided | in the sloping roo | f is c | alled | | |
| | (a) | dormer window | | (b) | bay window | | |
| | (c) | Corner window | | (d) | clear storey window | N | |
| 57. | A w | indow ,provided | in the sloping pite | ched | roof is called | | |
| | (a) | Casement windo | | (b) | sky light | | |
| | (c) | dormer window | | ` ' | bay window | | • |
| 58. | | | , | | serve both purpo | ses c | of opening and |
| | clos | | uitable type of doo | | | | |
| | (a) | - | | | revolving door (d) | | _ |
| 59. | | • | able for the entranc | | oanks, hotels, hospit | als, t | heatres etc. are |
| | (a) | sliding door | | | revolving door | | |
| | | collapsible stee | | ` ′ | Rolling steel shutte | | |
| 60. | | | y used of entrance | s of | godowns, sheds, sh | iowr | ooms, garages, |
| | _ | os is | 7 | | | | |
| | (a) | sliding door | | | revolving door | | |
| | (c) | Rolling steel sh | | (d) | both (a) and (c) | | |
| 61. | | _ | | 72 | hould not be more | | |
| | (a) | 12 | (b) 10 | (c) | \bigcirc | (d) | 20 |
| 62. | | _ | s in a flight should | | | A | |
| | (a) | 12 | (b) 3 | (c) | | (d) | 5 |
| 63. | | | g should not be les | | | | |
| | (a) | 1.0 m | (b) 1.2 m | ` ' | 0.9 m | (d) | Stair width |
| 64. | | | nd rise should lie b | | | 44 | 220. |
| | ` | | (b) 40 - 45 cm | | | (D) | 55 - 60 cm |
| 65. | | | nge their direction | | _ | | |
| | ` ' | | (b) 135° | (c) | 180° | (d) | 270° |
| 66. | | -legged stairs ar | ·e | | | | |
| | (a) | Straight stairs | | (b) | Quarter turn stairs | | |
| | (c) | half turn stairs | | (d) | three fourth stairs | | |
| 67. | | | between two conse | | <u>.</u> | | |
| | (a) | | (b) going | (c) | stringer | (d) | none of these |
| 68. | _ | | nd of a series of st | _ | | | |
| | (a) | relief | (b) landing | (c) | platform | (d) | end point |

| 69. | Minimum width of | a stair in a reside | ntial building is | |
|-----|----------------------|---------------------|---------------------------------------|-------------------------|
| | (a) 50 cm | (b) 75 cm | (c) 90 cm | (d) 100 cm |
| 70. | In public buildings, | the minimum trea | ad width should be | |
| | (a) 15 cm | (b) 20 cm | (c) 30 cm | (d) 40 cm |
| 71. | The location of stai | r in a public build | ling should be near | |
| | (a) the entrance | (b) the centre | (c) the lavatory (d) | end of the building |
| 72. | In public buildings, | , maximum riser-is | s limited to | |
| | (a) 10 cm | (b) 12 cm | (c) 15 cm | (d) 20 cm |
| 73. | The angular steps u | used for changing | direction of stairs are | called |
| | (a) angular steps | (b) radial steps | (c) winders | (d) spandril |
| 74. | The height of hand | rail above the tre | ead should be generally | y in between |
| | (a) 40 to 50 cm | (b) 60 to 75 cm | (c) 75 to 80 cm | (d) 100 cm |
| 75. | Minimum headroom | m required in a st | aircase is | _ |
| | (a) · 3.5 m | (-) | (c) 2.1 m | (d) 1.5 m |
| 76. | The first voussoir | at the springing le | vel on either <mark>side</mark> of th | he arch is called |
| | | (b) spandril crov | | (d) rise |
| 77. | The voussoir place | | | |
| | (a) springer | (-) -I | (c) launch | (d) key |
| 78. | The temporary str | ucture to support | an arch during constr | uction is called |
| | (a) Scaffolding | (d) shoring | (c) Jacking | (d) Centering |
| 79. | The bearing of lin | tel should not be l | | |
| - | (a) 10 cm | (b) 15 cm | (c) 20 cm | (d) 30 cm |
| 80. | A rolling steel shu | tter is often used | for | .5 |
| | (a) cinema halls | (b) shops | (c) drawing rooms | (d) beauty parlours |
| 81 | The development | of one or more loc | al swellings on the fin | ished plastered surface |
| | is called | | / N = 131-41 | (d) swelling |
| | (a) Foaming | (b) boiling | (c) blistering | (d) Swelling |
| 82 | . Screws for wood | | | (d) gauge |
| | (a) length | (b) diameter | (c) weight | (d) gauge |
| 83 | . In king post truss | | ertical posts is | (d) none |
| | (a) one | (b) two | (c) three or more | (d) none |
| 84 | | | yond the face of riser | (d) stringer |
| | (a) pitch | (b) nosing | (c) baluster . | (a) stringer |

85. In stairs 'Soffit' is

- (a) A vertical portion of a step providing support to the tread
- (b) A straight step having a parallel width of tread
- (c) The under surface of a stair
- (d) the angle which the line of nosing of the stair makes with the horizontal

: ANSWERS:

| | | 2 () | 4 (b) | 5. (b) |
|---------|---------|---------|---------|---------|
| 1. (b) | 2. (c) | 3. (a) | 4. (b) | • |
| 6. (b) | 7. (d) | 8. (c) | 9. (d) | 10. (c) |
| 11. (d) | 12. (d) | 13. (a) | 14. (b) | 15. (b) |
| 16. (c) | 17. (a) | 18. (d) | 19. (c) | 20. (b) |
| 21. (b) | 22. (c) | 23. (b) | 24. (a) | 25. (c) |
| 26. (b) | 27. (a) | 28. (b) | 29. (d) | 30. (d) |
| 31. (d) | 32. (a) | 33. (b) | 34. (d) | 35. (c) |
| 36. (c) | 37. (a) | 38. (b) | 39. (b) | 40. (d) |
| 41. (b) | 42. (d) | 43. (c) | 44. (b) | 45. (c) |
| 46. (b) | 47. (b) | 48. (a) | 49. (b) | 50. (c) |
| | 52. (b) | 53. (c) | 54. (c) | 55. (d) |
| 51. (d) | 57. (b) | 58. (b) | 59. (b) | 60. (d) |
| 56. (a) | | 63. (d) | 64. (b) | 65. (c) |
| 61. (a) | 62. (b) | | 69. (c) | 70. (c) |
| 66. (c) | 67. (b) | 68. (b) | | 75. (c) |
| 71. (a) | 72. (c) | 73. (c) | 74. (c) | |
| 76. (a) | 77. (d) | 78. (d) | 79. (b) | 80. (b) |
| 81. (c) | 82. (a) | 83. (a) | 84. (b) | 85. (c) |



20.

Construction Management and Equipments

MCQ'S

| POR OF TO SERVICE OF THE SERVICE OF | |
|---|---|
| 1. | On a bar chart the various activities of a project are shown by |
| | (a) Vertical lines (b) horizontal lines (c) shaded area (d) dot marks |
| 2. | Bar charts are consider suitable for |
| | (a) major projects (b) large projects (c) minor projects (d) all of these |
| 3. | CPM network is |
| | (a) activity oriented (b) event oriented (c) slack oriented (d) work oriented |
| 4. | PERT is |
| | (a) activity oriented (b) event oriented (c) slack oriented (d) work oriented |
| 5. | A dummy activity |
| | (a) had no tail event but had only a head event |
| | (b) had only tail event but no head event |
| | (c) had no sequence and can be fitted anywhere |
| | (d) neither require any resource nor any time |
| 6. | A bar chart is drawn for |
| | (a) time versus activity (b) activity versus resources |
| Δŗ | (c) resources versus progress (d) progress versus time |
| 7. | A 'dummy activity' in a network |
| | (a) is represented by a dotted line |
| | (b) is an artificial activity |
| | (c) does not consume any time or resources |
| | (d) all of these |
| 8. | A critical activity has |
| | (a) maximum float (b) minimum float (c) zero float (d) average fl |
| 9. | Which of the following is a dummy activity? |
| | (a) Excavation of foundations (b) Laying the foundation concrete |
| | (c) Awaiting the arrival of concrete (d) Curing the foundation concrete |
| | |

| est path (b) has null path | |
|---|---------------|
| cimum slack (d) has the minimum slack | |
| ich is crashed first has | |
| slope (b) least cost slope | |
| s no relation with cost slope (d) none of these | |
| n project time normally results in | (IES |
| the direct cost and increasing the indirect cost | |
| he direct cost and decreasing the indirect cost | • |
| he direct cost and indirect cost both | |
| the direct cost and indirect cost both | |
| is used for recording | (IES |
| of executive engineer (b) construction measurement | ts |
| of plants and equipment (d) indents for materials to b | e ordered14 |
| ation introduced by F.W. Taylor is called | (IES |
| ganisation (b) functional organisation | |
| ff organisation (d) line organisation | |
| l list II and select the correct answer using the codes | |
| | (IES |
| List II | |
| (Property of activity | ') |
| ivity to be crashed first 1. It has float | |
| ivity 2. It has least cost slope | |
| ivity 3. It maintains logic of | network |
| activity. 4. It has no float | |
| incarinii, vehi ise iiiiin* | |
| C D | |
| 4 3 | |
| 2 4 | |
| 3 1 | |
| 1 2 | |
| vance up to 10 % of the cost of work is given to a co | ontracto (|

Construction Management and Equipments

- (a) on commencement of work at site for payment of loan taken by him
- (b) for the purchase of construction materials
- (c) for the payment of advances to labour and other staff
- (d) For all activities required to start the work at site on finalization of the contract document

Security deposit deducted at 5 % from contractor's bill is

(IES)

- (a) refunded when the contractor has completed the work
- (b) refunded even before the completion of the work provided good progress has been established
- (c) retained till the expected life of the structure of say 100 years and spent for maintenance
- refunded when the defect liability period of six months or one monsoon which ever is later is over

18. In resource levelling

(IES)

- (a) total duration of project is reduced
- (b) total duration of project is increased
- (c) uniform demand of resources is achieved
- (d) cost of project is controlled

19. Consider the following activities in a building construction:

(IES)

- (1) Concreting of roof slabs
- (2) Brick-jelly lime concrete terracing
- (3) erection of frame work for slab (4) Construction of parapet wall in terrace

The correct sequence of these activities is

- (a) 1, 3, 2, 4
- (b) 3, 1, 4, 2
- 3, 1, 2, 4
- (d) 1, 3, 4, 2

20. Consider the following operations:

(IES)

- Drilling 1.
- Blasting
- 3. Mucking
- Placing steel

Placing concrete

The correct sequence of these operations in tunnel construction is

- (a) 1, 2, 4, 3, 5 (b) 1, 3, 2, 4, 5
- (c) 1, 2, 3, 4, 5, (d) 1, 3, 4, 2, 5
- 21. Match list I and list II and select the correct answer using the codes given below (IES) the lists:

| | | List I | | | | List II |
|-----|-----|-----------|----------|---------|--------|---|
| | | (Item) | | | | (characteristic) |
| | A. | Activit | y | - | 1. | Resourceless |
| | B. | Event | | | 2. | Resource consuming element |
| - | C. | Dumm | y | | 3. | Spare time |
| | D. | Float | | | 4. | Instantaneous stage |
| | Cod | les : | , | | | |
| | | Α | В | C | D | |
| | (a) | 1 | 3 | 4 | 2 | |
| | (b) | 2 | 1 | 4 | 3 | |
| | (c) | 2 | 4 | 1 | 3 | |
| | (d) | 3 | 4 | 1 | 2 | |
| 22. | Gra | ider is 1 | used m | ainly | for | (IES) |
| | (a) | trimmi | ng and | finis | hing | |
| | (c) | | ng and | | | (d) finishing, shaping and trimming |
| 23. | Wh | ich one | of the | follo | wing | g is not an excavating and moving type of equipment (IES) |
| | (a) | Buldoz | zer | (b) | clan | m shell . (c) scraper (d) Dump truck |
| 24. | The | e most s | uitable | type | e of e | equipment for compaction of cohesive soils is (IES) |
| | (a) | | ı wheel | | | (b) vibratory rollers |
| | (c) | Sheep | foot ro | llers | | (d) Tampers |
| 25. | The | basic : | action | invol | ved i | in sheep foot rolling is |
| | (a) | kneadi | | | - | essing (c) tamping (d) vibrating |
| 26. | For | excav | ating u | ıtility | tre | enches with precise control of depth, the excavation |
| | | iipment | | | | (IES |
| | (a) | Hoe | | (b) | Sho | ovel (c) Dragline (d) none of these |
| 27. | Co | st of oy | ning a | n equ | ipm | nent would include (IES |
| | (a) | cost o | f depre | ciatio | n, m | naintenance, repair and fuel |
| | (b) | cost o | f invest | tment | , wag | ges of the crew and fuel |
| | (c) | cost o | f fuel, | lubric | ating | g oil, investment and depreciation |
| | (d) | cost o | f invest | tment | , maj | jor repairs and depreciation |
| 28. | Ma | tch list | I (natu | re of | wor | rk) with list II (machine required) and select the correc |
| | ans | swer usi | ing the | code | s giv | ven below the list: (IES) |

Construction Management and Equipments

433

List I

- Excavation and moving A.
- В. Pure excavation
- C. Pure transportation 3.
- Pure hoisting D.

List II

- 1. Derrick
- 2. Dump truck
- Power shovel
- Drag line

Codes:

- 1 2 (a)
- (b) 3 2 1
- 2 1 3 (c)
- 2 (d) 3

29. The difference between the total float and free float is called

- (b) interfering float (c) Independent float
- (d) free float

30. The occurance of the completion of an activity, is called

- (a) head event
- (b) tail event
- (c) dual role event
- (d) none of these

31. In CPM, cost slope is determined by

Crash cost (a) Normal cost

Crash cost - Normal cost (b) Normal time - Crash time

Normal cost (c) Crash cost

Normal time - Crash time (d) Crash cost - Normal cost

32. In PERT, expected time is given by Float = Available time - Required time

(a)
$$\frac{t_0 + t_p + t_m}{2}$$

(b)
$$\frac{t_0 + 2t_p + t_n}{4}$$

(a)
$$\frac{t_o + t_p + t_m}{3}$$
 (b) $\frac{t_o + 2t_p + t_m}{4}$ (c) $\frac{t_o + 4t_m + t_p}{6}$ (d) $\frac{t_o + 4t_p + t_m}{6}$

$$\frac{t_o + 4t_p + t_m}{6}$$

33. Which of the following pairs are correctly matched?

- Sub-critical activity-positive float 1.
- 2. Critical activity - zero float
- Supercritical activity negative float
- (a) 1 and 2
- (b) 2 and 3
- (c) 1 and 3
- 1, 2 and 3

34. CPM requires

- (a) Single time estimate
- (b) double time estimate
- (c) triple time estimate
- none of these (d)

35. PERT stands for

(a) Programme estimation and reporting technique

Objective Civil Eng. \ 2016 \ 55

| | (b) Process estimation and review technique | | | | | |
|-----|---|--------------------------------------|--|--|--|--|
| | (c) Programme evaluation and review technique | | | | | |
| | (d) Planning estimation and resulting technic | lue . | | | | |
| 36. | The start or completion of task is called | | | | | |
| | (a) an activity (b) an event (c) | a duration (d) none of these | | | | |
| 37. | <u> </u> | | | | | |
| | (a) Time (b) resources (c) | time and resources (d) energy | | | | |
| 38. | • | | | | | |
| | (a) concrete cured (b) | walls plastered | | | | |
| | (c) excavation for foundation (d) | all of these | | | | |
| 39. | . Free float is the | | | | | |
| | (a) Portion of independent float (b) | | | | | |
| | (c) portion of interfering float (d) | all of these | | | | |
| 40. | | | | | | |
| | (a) at regular intervals (b) at fixed time | | | | | |
| | (d) whenever there is difference in planned | and actual performance | | | | |
| 41. | . Direct cost of a project is due to | 7 | | | | |
| | (a) delay in project (b) | | | | | |
| | (c) establishment charges (d) | | | | | |
| 42. | _ | | | | | |
| | (a) diaginit | clamshell (d) hoe | | | | |
| 43. | | | | | | |
| | (u) 000m | dipper (d) cab | | | | |
| 44. | - · | 45 0 | | | | |
| | (a) drag line (b) power shovel (c) | | | | | |
| 45. | 5. The effectiveness of explosion during tunn | | | | | |
| | (m) br |) firing (d) squibbing | | | | |
| 46. | • | | | | | |
| | (a) Jackhammer (b) Drifter (c |) Shot drill (d) blast hole drill | | | | |
| 47. | | | | | | |
| | (a) a definite job (b | | | | | |
| | (c) the terminals of an activity (d |) a definite position of an activity | | | | |
| _ | - | | | | | |

436

- (c) its latest start time and earliest finish time
- (d) its earliest finish time and earliest start time for its successor activity
- 60. A canal is trimmed on its sides and bottom by
 - (a) drag line
- (b) trimmer
- (c) trencher
- (d) angle dozer
- 61. Which of the following is not a hauling equipment?
 - (a) tractor
- (b) bulldozer
- (c) dragline
- (d) scraper

- 62. A concrete mixer is specified by
 - (a) The volume of the mixing drum
 - (b) horse power of prime mover
 - (c) volume of mixed concrete discharged after mixing of each batch
 - (d) mixer drum speed

: ANSWERS:

| 1. (b) 2. (c) 3. (a) 4. (b) 5. (d) 6. (a) 7. (d) 8. (c) 9. (c) 10. (d) 11. (b) 12. (b) 13. (a) 14. (b) 15. (c) 16. (c) 17. (d) 18. (c) 19. (c) 20. (c) 21. (c) 22. (d) 23. (d) 24. (c) 25. (a) | | | | | | _ |
|--|---------|---------|---------|----------|---------|---|
| 6. (a) 7. (d) 8. (c) 9. (c) 10. (d) 11. (b) 12. (b) 13. (a) 14. (b) 15. (c) 16. (c) 17. (d) 18. (c) 19. (c) 20. (c) 21. (c) 22. (d) 23. (d) 24. (c) 25. (a) | 1 (b) | 2. (c) | 3. (a) | 4. (b) | 5. (d) | |
| 11. (b) 12. (b) 13. (a) 14. (b) 15. (c) 16. (c) 17. (d) 18. (e) 19. (c) 20. (c) 21. (c) 22. (d) 23. (d) 24. (e) 25. (a) | | | 8. (c) | 9. (c) | 10. (d) | |
| 16. (c) 17. (d) 18. (c) 19. (c) 20. (c) 21. (e) 22. (d) 23. (d) 24. (e) 25. (a) | 1 | | 13. (a) | 14. (b) | 15. (c) | Ì |
| 21. (c) 22. (d) 23. (d) 24. (c) 25. (a) | | (1) | 18. (c) | 19. (c) | 20. (c) | |
| 21. (6) | | | 23. (d) | 24. (c) | 25. (a) | |
| 26. (a) 27. (d) 28. (b) 29. (b) 30. (a) | 1 | | | 29. (b) | 30. (a) | |
| 31. (b) 32. (c) 33. (d) 34. (a) 35. (c) | | | | 34. (a) | 35. (c) | |
| 31. (b) 30 (b) 40 (d) | | | | 39. (b) | 40. (d) | |
| 30. (b) 45. (b) 45. (b) | | | | 44. (a) | 45. (b) | |
| 41. (d) 42. (d) 49. (a) 50. (c) | , | | | | 50. (c) | |
| 40. (b) 57. (a) 55. (a) | | 1 | | | 55. (a) | |
| 51. (a) 52. (a) 50 (b) 60 (b) | | 1 | | nico inc | 60. (b) | |
| 36. (a) 37. (a) 37. (b) | 1 | 1 | 30. (0) | | | |
| 61. (d) 62. (c) | 61. (d) | 62. (c) | | <u> </u> | | 4 |



21.

Earthquake Engineering

| | SHO | RT ANSWER QUESTIONS | |
|-----------|---|---|--------------------|
| 1. Ans | According to IS: 1893 (Par: 4 zones - zone II, III, IV a | rt 1)- 2002, India is divided into and V. | seismic zones. |
| 2. | List important Indian citie | es located in zone V. | |
| Ans | : Bhuj | Tezpur | |
| | Imphal | Mandi | |
| | Guwahati | Srinagar | |
| | Kohima | Darbhanga | |
| 3. | larger than the ground mo | a Richter magnitude 8 earthquake in tion during a Richter magnitude 6 ea 1.0 implies 10 times higher waveform a | rthquake. |
| Alls | : ground motion during 8 earthquake. | earthquake is $10 \times 10 = 100$ times larger | r than 7 magnitude |
| 4. | earthquake? | stations are needed to locate the | epicenter of an |
| 5. | that in the storey above of stiffness of the three storey | ch the lateral stiffness is less than or less than percent of the ys above. | |
| 6. | : 70% and 80% A weak storey is one in w percent of that in the store : 80% | which the storey lateral strength is leave ey above. | ss than |
| 7. | | art-I) - 2002, the ratio (I/R) shall no | t be greater than |
| Ans | : 1.0 | | |
| 8. | After the 2001 Bhuj (Guja was built with base isolation | rat) earthquake, the four-storey Bhuj on using | hospital building |
| Ans | : Lead Rubber Bearings | | |

| 400 | |
|-----|---|
| | ٠ |
| 432 | ٩ |

| 9. | For the earthquake of magnitude 8.0, the maximum intensity of | n MMI scal | e would |
|----|---|------------|---------|
| | be about | • | |

Ans: As per Gutenbery and Richter

$$M_{L} = \left(\frac{2}{3}\right) I_{o} + 1$$

 I_0 = intensity of earthquake

$$8 = \left(\frac{2}{3}\right) I_o + 1$$

 $M_L = magnitude$

$$I_0 = 10.5$$

10. According to MSK (64) scale of intensity the total number of intensity classes (or grades) is _____.

Ans: 12 classes

11. When does the natural building period coincide with the earthquake period?

Ans: When frequency of earthquake force becomes equal to one of the natural frequency of building, i.e. at resonance.

12. According to IS: 13920: 1993, for all buildings which are more than 3 storeys in height, the minimum grade of concrete shall preferably be _____.

Ans. M 20 ($f_{ck} = 20 \text{ MPa}$)

13. As per IS: 13920 - 1993, the thickness of any part of the shear wall shall preferably, not be less than _____.

Ans: 150 mm

14. Which is thickness layer in the cross section of Earth?

Ans: Mantle 2900 km thick.

Crust = 5 to 40 km

Mantle

= 2900 km

outer core

= 2200 km

Inner core

= 1290 km

Total =
$$6400 \text{ km}$$

15. Which are the fastest and slowest seismic waves?

Ans: Fastest - P waves

Slowest - surfaces waves

16. The distance between focus and epicentre is known as _____

Ans: focal depth

17. Which is the most difficult structural element to retrofit?

Ans: Beam

રપ

| 140 | Objective Civil Engineering |
|--------------|---|
| 33. | Earthquake that occur away from the plate boundary are called |
| | : Intraplate earthquake |
| | e.g. 1993 Latur earthquake |
| | 1967 Koyna earthquake |
| 34. | waves can pass through solid, liquid and gas. |
| | : P-waves |
| 35. | waves are most destructive in nature. |
| Anc | · Surface waves |
| 36. | Earthquake size is defined in terms of its and |
| A | . Magnitude Intensity |
| 37. | The magnitude of an earthquake is generally measured on scale. |
| A | . Dighter |
| 38. | The earthquake with magnitude greater than 8.0 are known as |
| | Creat |
| 39. | Hard rock foundations can vibrate more by waves and soft soils can |
| | vibrate more by waves. |
| An | s: (i) P and S |
| | (ii) Surface |
| 40. | What is PGA? |
| An | s: The largest horizontal acceleration of a ground is known as PGA. |
| | PGA value of 0.6 g means, movement of ground can cause a maximum horizontal |
| | force on a rigid structure equal to 60% of its weight. |
| 41. | . Inertia force is equal to |
| An | s: Inertia force = mass × acceleration |
| | . The loads which vary with time are called as |
| Ar | ns: dynamic loads |
| | . A mass in space has number of degree of freedom. |
| | ns: Six |
| 44 | . The effect of damping is to |
| . A i | ns: decrease the amplitude of vibration |
| 45 | The value of damping for buildings may be taken as percent of the |
| | critical for dynamic analysis. |
| | ns: 2 to 5 |
| | 6. Restoring force is equal to |
| A | ns: stiffness × displacement |
| | 7. The equation of dynamic equilibrium is written as |
| A | $m\ddot{x} + c\dot{x} + kx$ |

| 48. | Earthquake load is an example of | | | | |
|-----|---|--|--|--|--|
| Ans | : Non-periodic dynamic load | | | | |
| 49. | If the building is flexible then fundamental natural period of building will be | | | | |
| | | | | | |
| Ans | : longer | | | | |
| 50. | If the building has more mass, the fundamental natural period will be | | | | |
| Ans | : Longer | | | | |
| 51. | The force mx is called as | | | | |
| Ans | : Inertia force | | | | |
| 52. | The force CX is called | | | | |
| Ans | : damping force | | | | |
| 53. | For rigid buildings fundamental period is less than seconds and for | | | | |
| | flexible buildings fundamental period is greater than seconds. | | | | |
| | : (i) 0.3 (ii) 1.0 | | | | |
| 54. | Overhead watertank is the simplest example of | | | | |
| Ans | : Single degree freedom system (SDF) | | | | |
| 55. | In general, taller buildings are flexible and have fundamental | | | | |
| | natural period. | | | | |
| | : (i) more (ii) longer | | | | |
| 56. | In a building 'diaphragm' term is used for | | | | |
| | : Roof | | | | |
| 57. | As per IS: 4326 - 1993 the crushing strength of masonry unit should not be less | | | | |
| | than | | | | |
| Ans | : 3.5 MPa | | | | |
| 58. | Minimum recommended cement: sand mortar for earthquake resistant masonry | | | | |
| | construction is | | | | |
| | : 1:6 | | | | |
| 59. | The thickness of load bearing walls shall not be less than | | | | |
| Ans | : 190 mm | | | | |
| 60. | The masonry load bearing walls can be built up to a maximum of storeys. | | | | |
| | s : 4 | | | | |
| 61. | Open ground storey building is called as | | | | |
| | s: soft storey | | | | |
| 62. | A discontinuous columns are called as | | | | |
| An | s: Floating columns | | | | |
| 63. | A short column attracts inertia force. | | | | |
| An | s: More | | | | |
| Obj | ective Civil Eng. \ 2016 \ 56 | | | | |
| | | | | | |

| Ans | Re-entrant corners of a building are subjected to during earthquake shaking. : Stress concentration |
|-------|--|
| Ans | : Stress concentration |
| | |
| 65. | |
| | Soft storey is an example of irregulatiry. |
| | : vertical |
| 66. | Short columns are also called as columns. |
| Ans | : captive |
| 67. T | The top storeys of a building are subjected to |
| | : Larger forces |
| | When earthquake forces are considered in design, the permissible stresses in material are increased by percent. |
| Ans | : 33% |
| 69. | What is Whythes? |
| | The thick stone walls have two vertical layers of large stones and the space between them is filed with mortar and small stones. The two layers of stones are called as whythes. |
| | TRUE/FALSE |
| • | State whether following statments are true or False and justify your answer in short: |
| 1. | Liquefaction is only possible in cohesive soil. |
| 1. | False: |
| | For liquefaction to occur soil must be loose, granular, saturated, earthquake fore and undrained condition. |
| 2 | As per IS 1893-2002, Gujarat is divided in Zone-III, IV and V only. |
| 2. | |
| • | True Ductile detailing is compulsory for RCC building located in Gujarat. |
| 3. | |
| | False Ductile detailing is necessary for buildings located in zone IV and V. |
| 4. | Design philosophy for gravity loads & design philosophy for lateral loads due to |
| | earthquake are same. |
| | False |
| | Gravity loads are static loads while earthquake load is dynamic load. |
| 5. | Performance of shear walls which are located near geometric centre of building is better than the identical shear wall located on periphery. |
| | False |

Shear walls are more effective when located along the periphery of the building. Such a layout increases resistance of the building to twisting.

6. Non-structural wall will fail before structural wall.

True

7. IS: 13920-1993 has given special detailing for beam-column joint.

True

8. Concrete structures offer less damping as compared to steel structures.

False

Structure damping ratio (ξ)

| Steel frame | 2 to 5% |
|---------------------------|---------|
| RCC frame | 5 to 7% |
| RCC frame with shear wall | 10% |
| Timber wall | 15% |

9. Code specifies higher value of R for building having better performance.

True: Reference IS: 1893-2002, Table-7.

10. Any structure is designed as earthquake proof structure.

False

Practically no building can be made earthquake proof. The engineering intention is to make buildings earthquake resistant.

11. Numbers of intra-plate earthquakes in world are more than number of inter-plate earthquakes.

False

Number of interplate earthquakes are more (about 99%) which occur along the boundaries of the tectonic plates.

12. Kochi is having maximum earthquake risk.

False

Kochi is situated on west coast in Kerala in zone III.

13. Peak Ground Acceleration (PGA) and zero period acceleration (ZPA) are same.

True

Effective PGA shall be taken as ZPA.

(Reference IS: 1893 - 2002, cl.3.11)

14. A building is located on the boundary of zone IV & V. It will be designed as if it is in zone IV.

False

It will be designed for more severe zone.

15. Two identical buildings to be constructed in zone IV & V. Building in zone V should be designed for lower lateral load than building in zone IV.

False

Building in zone V should be designed for higher lateral load.

16. R.C. frame building is more ductile as compared to steel frame building.

Steel frame building is more ductile as compared to R.C. frame building.

17. Taller buildings have smaller fundamental natural period.

False

Taller buildings are more flexible having longer fundamental natural period.

18. Engineers design buildings as earthquake resistant and not earthquake proof.

19. The intensity and magnitude of earthquake decreases as the epicentral distance increases.

Intensity decreases but magnitude remains same with increase in epicentral distance.

20. Surface waves are most destructive in nature.

21. S-Waves are also called shear waves.

True

22. Intensity value of earthquake is based on the experience by the people and performance of structures during earthquake.

True

Energy released in an earthquake of magnitude 6 is double compared to that released in magnitude 3 earthquake.

False

Energy released in EQ of magnitude 6 is (31 × 31 × 31) times more than that released in magnitude 3 EQ.

24. Intensity scale X is the highest intensity scale.

False

Highest intensity in MSK and MMI scale is XII.

25. Generally shallow focus earthquakes are more destructive compared to deep focus earthquakes of same magnitude.

True

26. Natural frequency of vibrating system will remain unchanged if damping level is increased.

False

If damping level is increased, multiplying factor for $\frac{S_a}{g}$ decreases (Table 3 of IS: 1893), i.e., $\frac{s_a}{g}$ decreases.

Earthquake Engineering

445

As $\frac{s_a}{g}$ decreases, period (T) increases (Fig. 2, IS : 1893). $f = \frac{1}{T}$ therefore, as T increases, f will decrease.

27. During liquefaction, underground lighter objects are raising up.

False

Objects will sink into the ground.

28. Over damped system comes to rest, faster than critically damped system.

False

Critically damped system comes to rest, faster than overdamped system.

- 29. Moment resisting capacity of a column should be more than that of beam framing on it.

 True
- 30. Ductility of RCC column increases by providing sufficient confining reinforcement.

 True
- 31. Love waves are most damaging seismic waves.

True

They have only horizontal component of velocity and they create shearing or breaking ruptures.

32. Damping can be neglected in the dynamic analysis of buildings.

False

It should be considered in dynamic analysis.

33. Stiffer columns means greater restoring force.

True

34. Liquefaction of soil generally occur in dry sand.

False

Liquefaction of soil generally occur in loose, saturated sand under dynamic load.

35. Openings in walls should be placed near the corners to prevent failure.

False

Openings in walls should be placed near centre,

36. Roofs made of timber or joists with brick tile coverings are called rigid diaphragms.

False

Flexible diaphragms.

37. It is advisable to have one long room rather than separate small rooms.

False

38. The length of masonry wall should not be greater than 15 m.

True

39. R.C. frame buildings should be designed on the concept of strong column and weak

True

40. Infill walls increases the flexibility of the building.

Infill walls decreases the flexibility of the building.

41. Lower grade steel is more ductile than higher grade steel.

42. As per ductile detailing, the bar in a lateral tie should have 135° hook with extension not less than 75 mm.

True

43. The longitudinal bars in a beam should not be overlapped at the ends. False

Should not be overlapped at the middle of beam.

44. The lateral ties in a column should be provided over the entire lap length at a spacing not more than 150 mm.

True

45. As per IS: 1893 - 2002 earthquake is not likely to occur simultaneously with wind.



Water Supply and Sanitrary Engineering

MCQ'S

(A) WATER SUPPLY ENGINEERING

| 1. The average domestic water consumption per capita per day for an Indian ci | | | | | | |
|---|--|--------------|--------|--------------|-------------|-------------|
| | as per IS: 1172 is | | | | | |
| | (a) 135 lpcd (b) 240 lp | ocd | | 270 lpcd | | 50 lpcd |
| 2. | The multiplying factor, as appli | ied to obtai | n the | e maximum da | ily wat | ter demand, |
| | in relation to the average i.e. per capita daily demand is | | | | | |
| | (a) 15 (b) 1.8 | | (c) | 2.0 | 2.7 | |
| 3. | (a) the neak hourly demand, in relation | | | | | |
| •• | to the maximum daily demand | | | | | |
| | (a) 1.5 (b) 1.8 | | | 2.0 | (d) 2 | |
| 4. | to obtain the neak hourly demand, in relation | | | | | |
| • | to the average daily demand is | | | | | |
| | (a) 1.5 (b) 1.8 | | | 2.0 | (d) 2 | |
| 5. The total water consumption per capita per day, including domestic, comm and industrial demands for an average Indian city, as per IS: 1172 is | | | | | commercial, | |
| | | | | | is | |
| | (a) 135 lpcd (b) 240 lp | | (c) | 270 lpcd | (d) 1 | 50 lpcd |
| 6. | (a) the second second as assumed as | | | | | |
| | (a) 5% (b) 7.5% | | (c) | 15% | (d) 2 | 25% |
| 7. $Q = 3182 \sqrt{P}$ is the formula for fire demand furnished by | | | | | | |
| | (a) Kuichling | | (b) | Freeman | | |
| | (c) National Board of fire prote | ction | | Buston | | |
| 8. The total water requirement of a city is generally as: | | | | | n the b | asis of: |
| 0. | (a) Maximum hourly demand | | | | | |
| | (b) Maximum daily demand + fire demand | | | | | |
| | (c) average daily demand + fire demand | | | | | |
| | (d) greater of (a) and (b) | | | | | |
| ^ | The most satisfactory formula for an estimate of fire demand Q for a city of | | | | | |
| 9. | annulation D in thousands for | Indian cond | lition | s is | | |
| population P in thousands for Indian conditions is | | | | | | |

448

| (a) $1136 \left[\frac{P}{10} + 10 \right]$ | Freeman's formula |
|---|-------------------|
|---|-------------------|

- (b) 3182 √P ... Kuichling's formula
- (c) 4640 \sqrt{P} [1 0.01 \sqrt{P}] ... National board of fire

The average rainfall of a country is a figure which is average over a period of 10.

- (a) 10 years
- (b) 15 years
- (c) 50 years
- (d) 35 years

: ANSWERS :

| 1. (a) 2. (b) 3 | 3. (a) | 4. (d) | 5. (c) |
|-----------------|--------|--------|---------|
| 6. (c) 7. (a) 8 | 3. (d) | 9. (b) | 10. (d) |

The evaporation losses from the surface of a reservoir can be reduced by 11. sprinkling

(a) D.D.T.

- (b) acetyl alcohol
- (c) Potassium permangamate
- (d) none of these

Pipes for branches to bathrooms and lavatories in domestic water supply are usually

- (a) 10 mm
- (b) 12 mm
- (c) 15 mm
- (d) 25 mm

The most widely used type of a tube well in India is 13.

(a) a dug well

(b) a cavity well

(d) a ranney well

(c) a stainer well The suitable method for forecasting population for a young and a rapidly growing 14. city is

- (a) arithmetic mean method
- (b) geometric mean method
- (c) comparative graphical method
- (d) None of these

The maximum quantity of water that can be supplied from a reservoir with full 15. guarantee during critical periods is called

- (a) reservoir yield
- (b) design yield
- (c) secondary yield (d) firm yield

Select the correct relationship between porosity (N), specific yield (y) and specific 16. retention (R). (d) Y = N + R

- (a) R = N + Y
- (b) N = Y + R
- (c) R > N + Y

The presence of algae in water indicates that the water is 17.

- (a) hard
- (b) soft
- (c) acidic
- (d) pure

Turbidity of water is due to 18.

- (a) algae
- (b) fungi
- (c) organic salts
- (d) finely divided particles of clay, silt, etc.

Objective Civil Eng. \ 2016 \ 57

| 31. | Dental carries in c | hildren may be caused | l due to water defic | ient in |
|-----|------------------------|--------------------------|------------------------------------|----------------------|
| | (a) calcium | (b) iron | (c) fluorides (d) | none of these |
| 32. | Higher quantities | of copper, more tha | n 2.5 mg/l or so, | can cause diesease |
| | pertaining to | | • | · |
| | (a) kidney | (b) lungs | (c) lever | (d) teeth |
| 33. | Blue baby disease | may be caused in inf | fants due to drinkin | ig water containing |
| | higher concentration | on of | | • |
| | (a) nitrites | (b) nitrates | (c) lead | (d) arsenic |
| 34. | The metal which is | the most hazardons t | o human beings, am | ong the following is |
| • | (a) Iron | (b) Barium | (c) Silver | (d) Arsenic |
| 35. | The only metal am | ong the following whi | ch is not toxic to hu | ıman beings is |
| | (a) Sodium | (b) Mercury | (c) Lead | (d) cadmium |
| 36. | The maximum allo | wable concentration o | f iron in water is | |
| | (a) 1.0 PPM | (b) 0.05 PPM | • ' | (d) 0.03 PPM |
| 37. | The maximum safe | permissible limit of | chlorid <mark>es in domesti</mark> | c water supply is |
| | (a) $0.5 \text{ mg/}l$ | (b) 2.5 mg/l | (c) 250 mg/l | (d) 100 mg/l |
| 38. | The maximum safe | permissible limit of | sulp <mark>hates in domesti</mark> | c water supplies is |
| | (a) 100 mg/l | (b) 200 mg/l | (c) 500 mg/l | (d) 1000 mg/l |
| 39. | The valve which al | llows the flow only in | one direction is | |
| | (a) reflux valve | (b) Sluice valve | (c) gate valve (d |) air-relief valve |
| 40. | 'Wholesome water | is the one, which doe | sn't contain | |
| | (a) Pathogenic bac | teria | | |
| | (b) Suspended mat | ter quantities harmful t | o man | |
| | (c) dissolved matte | er quantities harmful to | man | |
| | (d) all of the above | e | | |
| | | enstine. D. | annina la | |
| | | : ANSWEI | RS: | |

| 31. (c) | 32. (b) | 33. (c) | 34. (c) | 35. (a) |
|---------|---------|---------|---------|---------|
| 36. (c) | 37. (c) | 38. (b) | 39. (a) | 40. (d) |

- 41. The treatments which are generally given to treat raw water supplies, follow the sequence.
 - (a) Screening, sedimentation, disinfection, filtration
 - (b) Screening, sedimentation filtration, disinfection
 - (c) Screening, sedimentation, disinfection, aeration
 - (d) Screening, sedimentation, coagulation, filtration, disinfection

| 452 | Water Supply and Sanitary Engineering |
|-----|--|
| 54. | A check valve is also known as (a) relief valve (b) reflux valve (c) blow off valve (d) none of these |
| 55. | A sluice valve is also known as (a) air-inlet valve (b) scour valve (c) gate valve (d) none of these |
| 56. | Scour valves are provided (a) at the screet corners to control flow (b) at the dead ends to drain out the waste water (c) at every summit of the rising main (d) at the foot of the rising main to prevent back running of water. |
| 57. | The formula which is most appropriate for the design of pressure pipes is: (a) Darcy-weisbach formula (b) Manning's formula (c) Chezy's formula (d) Dupuit's formula |
| 58. | 'Shrouding' is essentially provided in (a) strainer type tube wells (b) cavity type tube wells |
| 59. | (c) slotted pipe tube wells (d) all of the above The settling velocity of inorganic particles in a sedimentation tank of a water treatment plant is governed by (a) Darcy's law (b) Stoke's law (c) Dupuit's law (d) all of the above |
| 60. | (a) Darcy's law (b) Stoke's law (c) Dupuit's law (d) note of these Sedimentation can remove inorganic particles having specific gravity upto (a) 2.65 (b) 1.65 (c) 1.20 (d) 1.03 : ANSWERS: |
| | (a) (b) (55 (c) |
| | 51. (d) 52. (d) 53. (b) 54. (b) 33. (c) 56. (b) 57. (a) 58. (c) 59. (b) 60. (c) |
| 61. | will |
| 62. | and having length L_1 depth = H, width = B is given by |
| 63 | (a) $\frac{Q}{B.H.}$ (b) $\frac{Q}{B.L.}$ (c) Q.B.H. (d) $\frac{Q}{B.H.L.}$ The horizontal flow velocity in a sedimentation tank passing a discharge = Q, having length = L, width = B and depth = H is given by |
| | (a) $\frac{Q}{B.H.}$ (b) $\frac{Q}{B.L.}$ (c) B.H. (d) none of the above |

| , | , | |
|-----|--|---------------------------------------|
| 42. | A harmful organism, which may be present in | |
| | |) escherichia-coli |
| | | none of these |
| 43. | The bacteria which survive in the presence a | as well as absence of oxygen, are |
| | called | |
| | |) B-coli (d) E-coli |
| 44. | Breweries and distilleries preferably require | |
| | |) potable waters (d) none of these |
| 45. | The waters to be used for boilers should be | |
| | (a) Hard (b) soft (c) |) Potable (d) none of these |
| 46. | The B-coli test is conducted to serve as a | |
| | (a) very good indicator of pathogenic bacteria | • |
| | (b) very good indicator of acidity | |
| | (c) very good indicator of alkalinity | |
| | (d) means of detecting turbidity | |
| 47. | The suspended solids present in water may give | |
| | |) colour (d) none of these |
| 48. | | |
| | (a) apparent colour (b) true colour (c |) colour (d) b and c both |
| 49. | | |
| | (4) |) Silica scale |
| | (b) month some |) all of the above |
| 50. | | |
| | (4) 1413133 31 1111111 | hardness of water |
| | (c) DO in water (d | residual chlorine in water. |
| | : ANSWERS : | |
| | | |
| | 41. (b) 42. (c) 43. (b) | 44. (a) 45. (b) |
| L | 46. (a) 47. (a) 48. (d) | 49. (a) 50. (b) |
| 51. | Based on the cobalt scale, the maximum permis | sible colour for domestic supplies is |
| J1. | | e) 25 (d) 20 |
| 52. | | |
| Ju. | _ | e) 5 (d) 0 |
| 53. | | |
| JJ. | system is | |
| | | c) Scour valve (d) reflux valve |
| | (4) 411 14110 (0) 514100 14110 (0 | (-) |

| 64. | Detention time f discharge = Q, I | or a sedimentation to ength = L, Width = 1 | ank (continuous to B and depth = H | flow type) is given for a is given by |
|-----|--------------------------------------|---|------------------------------------|--|
| | (a) $\frac{LBH}{O}$ | (b) $\frac{Q}{LBH}$ | (c) $\frac{Q}{B.L.}$ | (d) none of the above |

Surface loading or overflow velocity of a plain sedimentation tank may vary in 65. the range

(a) $100-500 l/hr/m^2$

(b) 500-750 l/hr/m²

(c) 1000-1250 l/hr/m²

(d) none of these

The velocity of flow of water in a plain sedimentation tank may normally be 66.

(a) 15-30 cm/sec

(b) 15-30 cm/min

(c) 15-30 cm/hr

(d) none of these

The most widely used coagulant for water treatment is 67.

(a) lime and soda

(b) Chlorinated copperas

(c) ferrous sulphate

(d) alum

A clariflocculator is a 68.

(a) plain sedimentation unit

(b) aeration unit

(c) coagulation-sedimentation unit

(d) none of the above

The detention time for a water sedimentation tank may vary between 69.

(a) 1-2 hr

(b) 2-4 hr

(c) 4-8 hr

(d) 16-24 hr

Particles of around 1 micron (10⁻⁶ m) size are best removed by

(a) filtration

(b) plain sedimentation

(c) Chemical precipitation

(d) chemical coagulation

: ANSWERS :

| 61. (a) | 62. (b) | 63. (a) | 64. (a) | 65. (b) |
|---------|---------|---------|---------|---------|
| 66. (b) | 67. (d) | 68. (c) | 69. (c) | 70. (d) |

The aeration of water is done to remove 71.

(a) colour

(b) Turbidity

(c) odour

(d) bacteria

Disinfection of water is done by 72.

(a) filtration

(b) Passing chlorine (c) Alum

(d) Heating

In water distribution scheme the commonly used pipe is 73.

(a) C.I. Pipes

(b) G.I.Pipes

(c) Hume pipes

(D) PVC Pipes

Rapid gravity filters remove bacteria to as much as 74.

(a) 80-90%

(b) 90-95%

(c) 98-99%

(d) none of these

| 83. | The | efficiency o | f sediment | tation does 1 | not de | pend | upon | • |
|-----|-------|---------------|---------------------------|----------------------------|-------------|---------|------------------------------|---------------------|
| | | detention tir | | | | (b) | depth of tank | |
| | (c) | length of ta | nk | | | | horizontal velo | city of flow |
| 84. | Whi | ch of the fo | llowing tr | eatment red | uce sa | ılinity | y of water. | |
| | 1. | Flocculation | and sedin | nentation | | 2. | Filtration | +t - + |
| | 3. | Reverse osn | nosis | | | | Electrodialysis | W. Carrier |
| | Sele | ct the corre | ct answer | using the co | odes g | iven | below: | |
| | (a) | 1 and 2 | (b) | 3 and 4 | | (c) | 2 and 3 | (d) 1 and 4 |
| 85. | If p | resent in wa | ater, chlor | ination of w | ater (| loes 1 | not reduce the | |
| | | _ | | | | (b) | organic matter | |
| | (c) | B.O.D. | | | | (d) | dissolved oxyg | en content |
| 86. | Mat | ch list I wit | th list II a | nd select the | corre | ect an | iswer using co | ed given below the |
| | lists | | | | | | ** | |
| | | List I | | | | List | | |
| | A. | cast iron pi | | | | 1. | Simplex joint Spigot and soc | sket joint |
| | В. | Asbestos co | | | | 2. | Screwed joint | zket joint |
| | C. | Plain ended | | ject to | | 3. | Screwed Joint | |
| | | frequent vi | brations | | | 4. | Victaulic joint | - |
| | D. | G.I.pipe | | 0 | | 74. | Victaurio joni | • |
| Cod | | A | В | C | D 4 | | | |
| | (a) | 1 | 3 | 2 | 4 | | | |
| | (b) | 2 | | 4 | 1 | | | |
| | (c) | 3 | 4 | 2 | . 3 | | | |
| | (d) | 4 | 2 | 1 | | the co | nnection of th | e service pipe with |
| 87. | The | e water sup | ply to a h | ouse begins | nectio | n cor | nprises. | |
| | | | | ns. The con Goose nec | heetto F | 3 | ferrule | 4. Water meter |
| | 1. | stop cock | | | | | | |
| | | | uence of t | hese connect) 3, 1, 2, 4 | .10113 1 | (c) | 3. 2. 1. 4 | (d) 1, 2, 4, 3 |
| | (a) | 1, 2, 3, 4 | | r treatment | | | -, -, , | |
| 88. | | pe II settiin | ig in water | articles in d | ilute s | uspen | sions | |
| | (a) | Settling of | f flocoulent | t particles in | dilute | susp | ension | |
| | (b) | Settling of | f flocculen | t particles in | conce | entrate | ed suspensions | |
| | (c) | Settling of | f noccuien f narticles | in the form | of a s | ludge | blanket | |
| 00 | (d) | Setting o | damand of | in the form fa water sa | mole | was 1 | found to be 0.2 | 2 mg/l. The amount |
| 89. | 111 | blooching t | nemana oz Sowder coz | taining 30% | 6 avai | ilable | chlorine to be | added to treat one |
| | | re of such a | | | | | | • |
| | |) 0.67 mg | | o) 0.06 mg | | (c) |) 1.33 mg | (d) 0.14 mg |
| | (a, | , 0.07 mg | (| , | | ` ' | - | |

Chlorine demand to treat 1 litre of water = 0.2 mg x mg of bleaching powder containing 30% chlorine, will supply $x \times 0.3$ mg of chlorine, To balance

$$0.2 = 0.3 \times x$$

x = 0.67 mg

- 90. Which of the following pairs are correctly matched?
 - 1. lime soda process softening
 - 2. Nalgonda technique...... fluoride removal
 - 3. Aeration..... coagulation
 - 4. Ozonation..... disinfection

Codes:

- (a) 1, 2 and 3
- (b) 1, 3 and 4
- (c) 1, 2 and 4
- (d) 2, 3 and 4

: ANSWERS :

| 81. (a) 82. (a) 83. (b | 84. (b) 85. (d) |
|-------------------------|-----------------|
| 86. (b) 87. (c) 88. (b) | 89. (a) 90. (c) |

- 91. The ideal residual pressure at the farthest consumer's tap in a properly designed water distribution system is in the range of
 - (a) $0.06 \text{ to } 0.20 \text{ N/mm}^2$

(b) $0.21 \text{ to } 0.25 \text{ N/mm}^2$

(c) 0.26 to 0.30 N/mm²

- (d) 0.31 to 0.35 N/mm²
- 92. Consider the following statements in respects of slow sand filters
 - 1. Qualitywise they are more efficient than rapid sand filters
 - 2. They need periodic back washing
 - 3. Their flow rate is much smaller than that of pressure filters of these statements:
 - (a) 1, 2 and 3 are correct
- (b) 1 and 2 are correct

(c) 1 and 3 are correct

- (d) 2 and 3 are correct
- 93. Match list I and list II and select the correct answer using the codes given below the lists.

List I

Type of water Source

- A. Surface water (river or canal)
- B. Water from infiltration gallery
- C. Lake/pond water
- D. Tube well water

List II

Treatment to be given

- Aeration, coagulation, sedimentation, disinfection
- 2. Disinfection
- 3. C_u SO₄ treatment, coagulation, sedimentation, filtration, disinfection
- 4. Coagulation, flocculation, sedimentation. Filtration, disinfection

(c) 2 1 (d) 3 2

2

(b)

3

3

Objective Civil Eng. \ 2016 \ 58

| 458 | | | | | Water | Sup | ply and Sa | nitary Engineering | | |
|------|---|--------------|--------------------|----------------------------------|----------|--------|--------------------------|--------------------------------------|--|--|
| 100. | Uniformity coefficient of filter media is | | | | | | given by | | | |
| | (a) | D_{50}/D_5 | (b) I | D ₅₀ /D ₁₀ | | (c) | D_{60}/D_5 | (d) D ₆₀ /D ₁₀ | | |
| | | | | : AN | SWER | S : | | | | |
| | 91. | (d) | 92. (c) | 93. | (a) | | 94. (d) | 95. (a) | | |
| | 96. | (d) | 97. (c) | 98. | (a) | | 99. (b) | 100 (d) | | |
| 101. | Aft | er which | of the followin | g water | treater | nent | units, the to | rbidity is maximum ? | | |
| | | chlorina | | 9 | | | primary se | | | |
| | (c) | Floccula | ation basin | | | | - | sedimentation | | |
| 102. | Wh | ich one | of the following | chemic | als is e | | - | alorination of water ? | | |
| | (a) | | sulphite | | | | Sodium bio | | | |
| | (c) | Calcium | carbonate | | | (d) | Hydrogen | peroxide | | |
| 103. | In | which tr | eatment unit is | 'schmu | tz deck | e' for | med ? | | | |
| | (a) | Sedimer | ntation tank | | | (b) | Rapid sand | filter | | |
| | (c) | Coagula | tion tank | | | (d) | Slow sand | filter | | |
| 104. | Chl | lorides fi | om water are r | emoved | by | | | | | |
| | (a) | Lime so | da process | | | (b) | Reverse os | mosis | | |
| | (c) | Cation 6 | exchange process | | | (d) | Chemical c | oagulation | | |
| 105. | Wh | ich one (| of the following | statem | ents is | not co | orrect ? | | | |
| | (a) | Solution | of pipe network | k by Ha | rdy cros | s met | hod is a tria | l and error solution. | | |
| | (b) | At a jur | ection of pipes, t | otal inf | low is e | qual t | to the total o | outflow. | | |
| | (c) | Loss of | head due to flo | w in a | clockwi | se di | rection shou | ld be equal to loss of | | |
| | | | flow in a counte | | | | | | | |
| | | | ross method can | | | | | | | |
| 106. | Mat | tch list I | and list II and | select t | he corr | ect ar | iswer using | the code given below | | |
| | the | lists : | | | | | | | | |
| | | List | | | | | ist II | | | |
| | | | ent process | | • | | oved matte | r , | | |
| | A. | | dimentation | | 1. | | olved gases | | | |
| ٤ | .В. | | l precipitation | | 2. | | olved solids | | | |
| ÷ | C. | Slow sai | nd filter | | 3. | | ended solids than 1.0 | s with specific gravity | | |
| | D. | Aeration | | | 4. | Floar | ting solids | | | |
| | | | | | | | ing somus | | | |

| Code | es: | Α | В | С | D | | | |
|------|-------------|---------------|------------|------------|---------|-------|------|-------------------------------------|
| | (a) | 5 | 1 | 4 | 2 | | | |
| | (b) | 3 | 2 | 5 | 1 | | | |
| | (c) | 5 | 2 | 4 | 1 | | | |
| | (d) | 3 | 1 | 5 | 2 | | | |
| 107. | Ma | tch list I (1 | Equation/L | aw) with | list IJ | (rel | ated | application) and select the correct |
| | ans | wer using | the code | given belo | ow th | e lis | ts: | |
| | | List I | | | | | Lis | et II |
| | A. . | Chick's la | aw | | | 1. | Dis | screte particle settling |
| | B. | Darcy - w | eisbach ec | luation | | 2. | Hea | ad loss in pipe |
| | C. | Stoke's la | ıw | | | 3. | . He | ad loss in filter |
| | D. | Carmen -l | kozeny equ | ation | | 4. | Rat | te of bacterial kill |
| Code | : | Α | В | C | D | | | |
| | (a) | 4 | 2 | 1 | 3 | | | |
| | (b) | 3 | 1 | 2 | 4 | | | |
| | (c) | 4 | 1 | 2 | 3 | | | |
| | (d) | 3 | 2 | 1 | 4 | | | |
| 108. | Ma | tch list I (| type of im | purity) w | ith li | st II | (eff | ect) and select the correct answer |
| | usir | ng the code | e given be | low the li | ists : | | | |
| | | List I | | | | | | List II |
| | A. | Carbonate | s and bica | rbonates o | of | | 1. | Permanent hardness |
| | | Ca and M | g | | | | | |
| | B. | Carbonate | s and bica | rbonates | | | 2. | Temporary hardness |
| | | of sodium | t Eul | | | | | shliza iuun <i>k</i> an |
| | C. | Sulphates | and chlori | des of | | | 3. | Alkalinity and softness |
| | | Ca and M | g | | | | | |
| | D. | Oxides of | iron and i | nanagnese | • | | 4. | Colour and taste |
| Code | s: | Α | В | C | D | | | |
| | (a) | 1 | 3 | 2 | 4 | | | • |
| | (b) | 2 | 4 | 1 | 3 | | | |
| | (c) | 1 | 4 | 2 | 3 | | | |
| | (d) | 2 | 3 | 1 | 4 | | | • |
| 109. | | tch list I (| | | | II (p | urpo | se) and select the correct answer |
| | | 5 | 5 | | | | | |

459

| 160 | | | | | List II | |
|---|--|---|--|---|--|--|
| ž. | List I | • | | | Purpose | |
| | Type of pipe | • | | 1. | House plumbling | |
| Α. | Steel pipe | _ | | 2. | Hot water carrying | <u>r</u> |
| В. | Cast iron pip | | | 3. | Distribution main | |
| C. | G.I. pipe | | | 4. | Pumping main | |
| D. | PVC pipe | D | С | D. | 1 0 | |
| Codes: | A | В | 2 | 3 | | |
| (a) | _ | 1 | 2 | 1 | | |
| (b) | _ | 3 | 4 | 3 | | |
| (c) | | 1 | 4 | 1 | | |
| (d) | | 3 | | | | |
| 110. C | onsider the fol | lowing | statem | atan ninge ar | nd sewer pines is | |
| Th | | | | | nd sewer pipes is | |
| 1. | in the mater | ial used | d for the | nines | | |
| | | | | | | |
| 2. | in the pressi | ure of t | he liqui | d flow | | |
| 2. 3. | in the pressi | ure of t | the liquiolids the | d flow y carry | 4.0 | |
| 2. 3. | in the pressi | ure of to inded so itements | the liquicolids the given | d flow y carry above is/are | correct ? | (d) 1 2 and 3 |
| 2. 3. W | in the pressi | ure of to inded so itements | the liquicalids the sign of the side of th | d flow y carry above is/are aly | (c) 2 and 3 | (d) 1, 2 and 3 |
| 2. . 3. W | in the presso in the suspe which of the sta | ure of to inded so itements | the liquicalids the sign of the side of th | d flow y carry above is/are nly : ANSWER | (c) 2 and 3 S: | |
| 2. 3. W | in the presso in the suspe which of the sta | ure of tended so | the liquicalids the sign of the side of th | d flow y carry above is/are nly : ANSWER 103. (d) | (c) 2 and 3 S: | 105 (d) |
| 2. 3. W | in the pressi in the suspe Thich of the sta a) 1 and 3 | inded so tements 102. | the liqui- olids the s given (b) 1 or (a) | d flow y carry above is/are nly : ANSWER 103. (d) 108. (d) | (c) 2 and 3 S: 104. (b) 109. (b) | 105 (d) |
| 2. 3. W | in the pressi in the suspe Thich of the sta a) 1 and 3 | inded so itements 102. | the liqui- olids the s given (b) 1 or (a) | d flow y carry above is/are nly : ANSWER 103. (d) 108. (d) | (c) 2 and 3 S: 104. (b) 109. (b) responsible for ent | 105 (d) 110. (d) eric fever ? |
| 2. 3. W (2 | in the pression the susper which of the state of the stat | inded so itements 102. | the liqui- olids the s given (b) 1 or (a) | d flow y carry above is/are nly : ANSWER 103. (d) 108. (d) | (c) 2 and 3 S: 104. (b) 109. (b) responsible for ent (b) Salmonella | 105 (d) 110. (d) eric fever ? |
| 2. 3. W (2) 1 111. V (| in the pression the susper in the susper in the susper in the susper in the state of the state of the susper in the suspect in the susper in the suspect in | nded so tements 102. 107. he following this toly | the liqui- olids the s given (b) 1 or (a) (a) owing o | d flow y carry above is/are nly : ANSWER 103. (d) 108. (d) rganism is | (c) 2 and 3 S: 104. (b) 109. (b) responsible for ent (b) Salmonella (d) Echinococc | 105 (d) 110. (d) eric fever? typhi us |
| 2. 3. W (2 | in the pression the susper in the susper in the susper in the susper in the state of the state of the susper in the suspect in the susper in the suspect in | nded so tements 102. 107. he following this toly | the liqui- olids the s given (b) 1 or (a) (a) owing o | d flow y carry above is/are nly : ANSWER 103. (d) 108. (d) rganism is | (c) 2 and 3 S: 104. (b) 109. (b) responsible for ent (b) Salmonella (d) Echinococc | 105 (d) 110. (d) eric fever? typhi us |
| 2. 3. W (8 | in the pression the susper in the susper in the susper in the susper in the state of the state of the susper in the suspect in the susper in the suspect in | nded so tements 102. 107. he following this toly | the liqui- olids the s given (b) 1 or (a) (a) owing o | d flow y carry above is/are nly : ANSWER 103. (d) 108. (d) rganism is | (c) 2 and 3 S: 104. (b) 109. (b) responsible for ent (b) Salmonella (d) Echinococc ne population grow | 105 (d) 110. (d) eric fever? typhi us th in a town norma |
| 2. 3. W (a) 111. V (112. (c) | in the pressor in the susper thich of the stand 3 of the stand 3 of the stand 3 of the standard standa | 102. 107. he follower follow | the liqui- colids the s given (b) 1 or (a) (a) cowing o tica wing fac | d flow y carry above is/are nly : ANSWER 103. (d) 108. (d) rganism is | (c) 2 and 3 S: 104. (b) 109. (b) responsible for ent (b) Salmonella (d) Echinococc ne population grow (2) Migrations | 105 (d) 110. (d) eric fever? typhi us th in a town norma |
| 2. 3. W (a) (a) (111. V (112. (a) (b) (c) (d) (d) (d) | in the pressor in the susper which of the star in the susper which of the star in the star in the star in the susper which of the star in | 102. 107. he followic grow | the liqui- colids the s given (b) 1 or (a) (a) cowing o tica wing fac | d flow y carry above is/are nly : ANSWER 103. (d) 108. (d) rganism is in | (c) 2 and 3 S: 104. (b) 109. (b) responsible for ent (b) Salmonella (d) Echinococc ne population grow (2) Migrations (4) logistic gro | 105 (d) 110. (d) eric fever? typhi us th in a town norma |
| 2. 3. W (a) (a) (111. V () (112. () () () () | in the pressor in the susper which of the star in the susper which of the star in the star in the star in the susper which of the star in | 102. 107. he followic grow | the liqui- colids the s given (b) 1 or (a) (a) cowing o tica wing fac | d flow y carry above is/are nly : ANSWER 103. (d) 108. (d) rganism is in | (c) 2 and 3 S: 104. (b) 109. (b) responsible for ent (b) Salmonella (d) Echinococc ne population grow (2) Migrations (4) logistic grogiven below: | 105 (d) 110. (d) eric fever? typhi us th in a town norma |
| 2. 3. W (a) (a) (111. V () (112. () () () () () () () () () () () () () (| in the pression the susper in the suspect in the susper in the suspect | 102. 107. he followic growest answers | the liqui- colids the s given (b) 1 or (a) (a) cowing o tica wing fac the wer usin | d flow y carry above is/are nly : ANSWER 103. (d) 108. (d) rganism is in tors, does the | (c) 2 and 3 S: 104. (b) 109. (b) responsible for ent (b) Salmonella (d) Echinococc ne population grow (2) Migrations (4) logistic grogiven below: (c) 1, 2 and 3 | 105 (d) 110. (d) eric fever? typhi us th in a town norma |
| 2. 3. W (a 1 111. V (112. (((((((((((((| in the pression the susper in the suspect in the susper in the suspect | 102. 107. he followic growest answers | the liqui- colids the s given (b) 1 or (a) (a) cowing o tica wing fac the wer usin | d flow y carry above is/are nly : ANSWER 103. (d) 108. (d) rganism is in tors, does the | (c) 2 and 3 S: 104. (b) 109. (b) responsible for ent (b) Salmonella (d) Echinococc ne population grow (2) Migrations (4) logistic grogiven below: | 105 (d) 110. (d) eric fever? typhi us th in a town norma |

| | List I | | | List II | | | |
|--------|----------------------|-----|------------|---|--|--|--|
| | (Tests) | | (Features) | | | | |
| A. | n in a toat | | | The gradual rise of water level in well is observed as time progresses. | | | |
| В. | B. Recuperation test | | | Rate of pumping is adjusted to constant level of water in well | | | |
| C. | Pressure te | st | 3. | Vigorous mixing of the chemical followed by slow mixing | | | |
| D. | O. Jar test | | 4. | Pipeline is filled up with water, allowed to stand for sometime and then atleast double the maximum | | | |
| | | | | pressure is applied. | | | |
| Codes: | A | В | C | D | | | |
| (a) | 1 | 2 | 3 | 4 | | | |
| (b) | 2 | | 4 | 3 | | | |
| (c) | 1 | 2 | 4 | 3 | | | |
| (d) | 2 | . 1 | 3 | 4 emoved by rapid sand filter from water? | | | |

114. Which of the following are removed by rapid

Dissolved solids 1.

suspended solids

Bacteria 3.

Hel minths

Select the correct answer using the codes given below

Codes:

- (a) 1 and 2
- (b) 2 and 3
- (c) 1 and 3
- (d) 2, 3 and 4

115. Which of the following treatment processes are necessary for removing suspended solids from water?

1. Coagulation

Flocculation 2.

Sedimentation

Disinfection

Select the correct answer using the codes given below:

- (a) 1 and 2
- (b) 1, 2 and 3
- (c) 2 and 3
- (d) 1 and 4

116. The population figures in a growing town are as follows:

| Year | Population |
|------|------------|
| 1970 | 40,000 |
| 1980 | 46,000 |
| 1990 | 53,000 |
| 2000 | 8,000 |

The predicted population in 2010 by Arithmetic Regression method is

- (a) 62,000
- (b) 63,000
- (c) 64,000
- (d) 65,000

Hint:
$$\bar{x} = \frac{6000 + 7000 + 5000}{2} = 6000$$

$$\therefore P_{2010} = P_o + n.\bar{x}$$

$$= 58,000 + 1 \times 6000 = 64,000$$

- 117. The amount of bleaching powder containing 20% available chlorine needed to chlorinate a rural water supply covering a population of 10,000 at 50 lpcd at the rate of 2 ppm is
 - (a) 1 kg
- (b) 5 kg
- (c) 0.2 kg
- (d) 20 kg

Hint: Refer question no.89

Chlorine needed to treat 1 lit of water = 2 mg (ppm)

x mg of bleaching powder containing 20% chlorine

will supply $x \times 0.2$ mg of chlorine

Total quantity of water = $10,000 \times 50$

$$= 5 \times 10^5$$
 litres

To balance,

$$0.2 \times x = 2$$

x = 10 mg...... for 1 litre of water

 \therefore For 5×10^5 litre of water,

Chlorine required =
$$10 \times 5 \times 10^5$$

= 5×10^6 mg

$$= 5 \text{ kg}$$

118. Consider the following statments:

While deciding to locate an intake structure for a city situated on a river bank, intake for water supply should be located

in deep waters 1.

- Sufficiently away from shore lines 2.
- upstream of the populated city which of these are correct?
- near navigational channel

- (a) 1, 2 and 4
- (b) 1, 2 and 3
- (d) 1, 3 and 4 (c) 2, 3 and 4
- 119. Match list I (water quality) with List II (method of determination) and select the correct answer using the codes given below the lists:

List I

(water quality)

- Hardness
- Chlorine В.
- DO C.
- chloride D.

List II

(Method of determination)

- Winkler method 1.
- EDTA method 2.
- Orthotolidine test 3.
- Mohr method 4.

463

| Codes: | A | В | С | D | | |
|--------|-----|---|---|---|---|--|
| (a) | 2 | 3 | 1 | 4 | | |
| (b) | 2 | 4 | 1 | 3 | | |
| (c) | . 1 | 3 | 2 | 4 | • | |
| (d) | 1 | 4 | 2 | 3 | | |

- 120. The purpose of providing balancing reservoir in a water supply distribution system is to
 - (a) equalize pressure in the distribution system
 - (b) Store adequate quantity of water to meet requirements in case of breakdown of flow
 - (c) Store adequate fire fighting reserve
 - (d) take care of fluctuations in the rate of consumption

: ANSWERS :

| 111. (b) | 112. (b) | 113. (b) | 114. (d) | 115. (d) | |
|----------|----------|----------|----------|----------|--|
| 116. (c) | 117. (b) | 118. (b) | 119. (a) | 120. (d) | |

- 121. Which of the following are the common problems associated with the operation of rapid sand filter?
 - 1. Air binding

2. cracking of sand beds

3. Bumping of filter beds

4. mud balls

Codes:

- (a) 1 and 2
- (b) 2 and 3
- (c) 2, 3 and 4
- (d) 1,2,3 and 4
- 122. Match list I (unit in water treatment plant) with list II (impurities removed) and select the correct answer using the codes given below the lists:

List I

- A. Aerator
- B. Rapid sand filter
- "C. Slow sand filter
- D. Sedimentation tank
 (after coagulation and
 flocculation)

List II

- 1. Excess CO₂ and H₂S
- 2. Settleable and colloidal matter
- 3. Suspended matter
- 4. Suspended colloidal and bacteriological matter

| 464 | | | | | Wat | ter S | up | ply and Sanitary Engineering |
|------|------|-----------------------|--------------|-----------|-----------|--------|------|---------------------------------------|
| Code | es : | A | В | С | D | | | |
| | (a) | 1 | 3 | 2 | 4 | | | |
| | (b) | 3 | 1 | 2 | 4 | | | |
| | (c) | 3 | 1 | 4 | 2 | | | |
| | (d) | 1 | 3 | 4 | 2 | | | |
| 23. | СО | nsider the | following i | mpurit | ies | • | | |
| | 1. | CO ₂ and l | H_2S | | | | | |
| | 2. | Finely div | ided susper | ided ma | tter | | | |
| | 3. | Disease ca | using bacte | eria | | | | |
| | 4. | Excess alk | kalinity | | ÷ | | | |
| | The | correct seq | uence of the | e remov | al of the | se imp | our | rities in a water treatment plant is: |
| | (a) | 1, 2, 3, 4 | (b) | 1, 4, 3 | 3, 2 | (| c) | 1,4,2,3 (d) 4, 1, 3, 2 |
| 24. | The | usual size | of residen | tial fer | rule bo | re vai | ies | s from: |
| | (a) | 1 mm to 5 | 5 mm | | | (| b) | 10 mm to 50 mm |
| | (c) | 100 mm to | 500 mm | | | (| d) | 1000 mm to 5000 mm |
| 25. | Rec | iprocating | pumps are | suitab | le for | | | |
| | (a) | low discha | arge and hig | gh heads | s | (| b) | high discharge and low heads |
| | (c) | low discha | arge and lov | w heads | | 75 (| d) | high discharge and high heads |
| 26. | | ich one of lity ? | the follow | ing filte | ers will | prod | uc | e water of higher bacteriological |
| | (a) | Slow sand | filter | | | (| b) | Rapid sand filter |
| | (c) | Pressure fi | ilter | | | . (| d) | Dual media filter |
| 27. | Cor | nmonly use | ed hand pu | mp is t | he | | | |
| | (a) | Centrifuga | l pump | | | (| b) | reciprocating pump |
| | (c) | rotary pun | тр | | | (| d) | axial flow pump |
| 28. | Ma | tch list I (p: | arameters) | with lis | t П (рег | rmissi | ble | e concentration in drinking water) |
| | and | select the | correct an | swer us | sing the | codes | g | given below the list : |
| | | List I | | | | 1 | .ist | t II |
| | A. | Hardness | | | 1 | . 0 | .1 | mg/l |
| | B. | Nitrate con | ncentration | | 2 | 2. 0 | .5 | mg/l |
| | C. | Iron conce | ntration | | 3 | 3. 2 | 00 |) mg/ <i>l</i> |
| | D. | Fluoride co | oncentration | ı | 4 | 1. 4 | 5 r | mg/l |

465

| Codes: | A | В | C | D | |
|--------|---|---|---|---|--|
| (a) | 3 | 4 | 2 | 1 | |
| (b) | 3 | 4 | 1 | 2 | |
| (c) | 4 | 3 | 2 | 1 | |
| (d) | 4 | 3 | 1 | 2 | |

129. The various treatment processes in a water treatment plant are listed below:

- 1. Filteration
- 2. Chlorination
- 3. Sedimentation

- 4. Coagulation
- 5. Flocculation

The correct sequence of these processes in water treatment is

- (a) 1,2,3,4,5
- (b) 4, 5,3,1,2
- (c) 2,3,1,5,4
- (d) 1,2,5,3,4

130. Which one of the following is the purpose of providing a surge tank in a pipeline carrying water?

- (a) To store water
- (b) to increase the pressure throughout the pipeline
- (c) To stop overflowing water
- (d) To protect the pipe line against water hammer

: ANSWERS :

| 121. (d) | 122. (d) | 123. (c) | 124. (b) | 125. (a) |
|----------|----------|-----------|----------|----------|
| 126. (d) | 127. (b) | .128. (b) | 129. (b) | 130. (d) |

(B) SANITARY ENGINEERING

| 1. | The sewerage system originates from |
|-----|---|
| | (a) outfall sewer (b) main sewer (c) house sewer (d) none of these |
| 2. | The sewer which transports the sewage to the point of treatment, is called |
| | (a) house sewer (b) main sewer (c) outfall sewer (d) none of these |
| 3. | A sewer which receives the discharge from a number of independent houses is called: |
| | (a) house sewer (b) intercepting sewer (c) lateral sewer (d) none of these |
| 4. | The waste water coming from kitchens and bathrooms is popularly know as |
| | (a) domestic sewage discharge (b) sludge discharge |
| | (c) drainage discharge (d) none of these |
| 5. | The type of sewage system which carries storm water and sewage is called a |
| | system. |
| | (a) storm water (b) domestic (c) separate (d) combined |
| 6. | Branch and main sewers are widely made of: |
| | (a) R.C.C. (b) P.C.C. (c) glazed stone wave (d) cast iron |
| 7. | Pick up the correct statement from the following: |
| | with self cleansing velocity in sewers |
| | (a) the silting occurs at the bottom |
| | (b) the scouring occurs at the bottom |
| | (c) the silting and scouring both occurs at the bottom |
| | (d) neither silting nor scouring occur at the bottom |
| 8. | Sewer pipes of diameter less than 0.4 m are designed at maximum flow, to be |
| | running at: |
| | (a) full depth (b) $\frac{1}{2}$ full depth (c) $\frac{2}{3}$ full depth (d) $\frac{3}{4}$ full depth |
| | |
| 9. | The water carriage system of collection of waste products is preferred to dry |
| | conservancy system, because: |
| | (a) it is cheaper in initial cost |
| | (b) it does not require treatment before disposal |
| | (c) it is more hygienic in nature (d) it is easier to maintain |
| 10. | For the design of sewers in India, the percentage of sewage discharge is assumed as |
| | (a) 25-30% of water supplied from water works |
| | (b) 75-80% of water supplied from water works |
| | (c) 100 % of water supplied from water works |
| | (d) none of these |
| | |

| | | | : | ANSW | ERS: | | | |
|------|----------------------------------|----------------|-------------------------|-------------|-------------|---------------|---------|---------------------|
| Γ''' | 1. (c) | 2. (c |) | 3. | (c) | 4. (1 |) | 5. (d) |
| | 6. (a) | 7. (d | · | 8. | (b) | 9. (0 | s). | 10. (b) |
| 11. | The sewer | pipes have t | o be des | signed a | nd checke | ed for : | | |
| | | naximum flow | | | (b) | only minimu | | w |
| | (c) both m | naximum and | minimu | n flow | • ' | none of the | | |
| 12. | Sewage tre | atment work | | | designed | for a period | d of | 40.50 |
| | (a) 5-10 y | | b) 15-26 | | | 30-40 years | (b) | 40-50 years |
| 13. | A.C. pipes | are generall | y joined | l by usi | ng | | | |
| | (a) bell ar | nd spigot join | t | | | simplex joir | | |
| | (c) lock jo | oint | | | (d) | none of the | se | |
| 14. | Testing of | sewer may i | | | | (4) | 611 | these tests |
| | (a) water | | 1/ | or test | (-) | test (d) | an | mese tests |
| 15. | | e generally la | id star | ing from | m (b) | outfall poin | | |
| | (a) all tak | | | | | any point le | | alionment |
| | (c) mid p | oint | | | (d) | | | |
| 16. | | im and maxi | mum al | ameters | of sewers | generally a | uoptea | l in the designs, |
| | may be: | | | | (h) | 15 cm and | 300 cı | n |
| | ` ' | and 100 cm | | | , | 60 cm and | | |
| | ` ' | and 450 cm | | | | | | |
| 17. | $\frac{1}{4}$ to $\frac{1}{3}$ s | pace is left v | a <mark>ca</mark> nt in | the des | sign of sev | vers pipes a | t maxi | mum discharge |
| | due to: | | | | | | | *(()) |
| | (a) possib | ole low estim | ates of r | naximun | n and aver | age now | | |
| | | scale possible | | | storm wate | er | | |
| | • • • | pected increas | | ulation | | | | |
| | (d) all th | e above facto | rs | | | ing half full | the | hydraulic mean |
| 18. | | cular sewer | ot diam | eter D | апа гани. | ing han fun | i, the | hydraulic mean |
| | depth is | | Б | | | D | | D |
| | (a) $\frac{D}{2}$ | | (b) $\frac{D}{3}$ | | (c) | $\frac{D}{4}$ | (0 | $\frac{D}{5}$ |
| 10 | 2 A ning 41 | | | from s | | 7 | s, wate | er closets, etc. is |
| 19. | called | ai ieceives u | inchai S | , 11 (111 6 | | | | |
| | (a) Soil | nino | (b) was | ete nine | (c) | vent pipe | (d) a: | ntisiphonage pipe |

20.

| 20. | (a) baseme | ent | (b) in | vert | | (c) | bedding | | (d) | bed l | evel |
|-----|-----------------------|--------------|-----------|------------------|--------|---------------|--------------|---------|--------|--------|-----------|
| | • | | | : ANSW | ERS | : | | | | | |
| | 11. (c) | 12. | (b) | 13. | (b) | $\overline{}$ | 14. | (d) | | 15. | (b) |
| | 16. (b) | 17. | (d) | 18. | (c) | | 19. | (a) | | 20. | (b) |
| 21. | Decayed fu | uits, grass | , leaves, | vegetable | wast | e et | c. belong | to refy | ise c | alled | |
| | (a) sewage | | (b) ga | _ | | | sullage | | | soil v | |
| 22. | Waste wat known as | ters from | bathro | oms, kitel | iens, | was | hing plac | es, wa | ish l | basin: | s etc. is |
| | (a) Garbaş | ge | (b) R | ubbish | | (c) | sullage | | (d) | sewa | ge |
| 23. | Laying of | sewers is | usually | done with | the l | help | of | | | | |
| | (a) a theo | dolite | | | | (b) | - | | | | |
| | (c) a plan | | | | | | sight rail | | | | |
| 24. | The sewer | pipes of c | | | | | | | | | |
| | (a) full de | | 4()) = | full dept | | | | | | | ll depth |
| 25. | The most | suitable so | ection of | f a sewer | in a s | | | | | | |
| | (a) rectan | _ | (b) ci | | | (c) | new egg | shape | d (d) | para | bolic |
| 26. | The appro | ximate % | of wate | er in sewa | ge is | | 77 | | 1.4 | | |
| | (a) 90 % | | (b) 99 |) <mark>%</mark> | | (c) | 99.9 % | | (d) | 100 | % |
| 27. | Manholes | | | | - (| | | | 0 | | |
| | (a) at all | | | | r | | at all cha | | | dient | of sewer |
| | (c) at all | | | | | ` ' | all of th | | | | - 43 |
| 28. | A manholo | | | | | | | | | | |
| | (a) 0.9 m | | | 2 m | | (c) | 1.5 m | | (a) | 2.0 f | n |
| 29. | Manhole o | | | ircular | | | | | | | |
| | ` ' | chitectural | | • , | 41 | . 1 | 1- | | | | |
| | | vent fallin | | cover into | the i | | | 41 | 4 | | |
| | • • | rengthen th | | | | ` ' | to make | | itry c | onve | nieni |
| 30. | An egg-sh | | er, when | compare | i to a | | | | | | |
| | (a) econo | | | | | ` ' | more sta | | | | |
| | | de better se | | sing veloc | ty at | low | discharge | S | | | _ |
| | (d) easier | to constru | ict | | | | | | | | - |
| | | | | | | | | | | | |

: ANSWERS :

| | | | T T | 0.5 (1.) |
|---------|---------|--------------|---------|----------|
| 21 (b) | 22. (c) | 23. (d) | 24. (c) | 25. (b) |
| 21. (b) | 22. (0) | / | 20 (4) | 30. (c) |
| 26. (c) | 27. (d) | [28. (c) | 29. (d) | 30. (c) |

- The flow velocity in a sewer does not depend on 31.
 - (a) its grade

- (b) its length
- (c) its hydraulic mean depth
- (d) its roughness
- In a circular sewer of dia D, if the depth of flow is $\frac{D}{4}$, the wetted perimeter will be

- (a) $\frac{\pi D}{4}$ (b) $\frac{\pi D}{2}$ (c) $\frac{\pi D}{3}$ (d) none of these

Mint: $\frac{d}{D} = \frac{1}{2} [1 - \cos \frac{\theta}{2}] \frac{d}{D} = \frac{D}{4 \times D} = 0.25$ $0.25 = \frac{1}{2} \left[1 - \cos \frac{\theta}{2} \right]$

$$0.25 = \frac{1}{2} \left[1 - \cos \frac{\pi}{2} \right]$$

$$\theta = 120^{\circ}$$

Now
$$P = \pi D \times \frac{\theta}{360} = \pi D \times \frac{120}{360} = \frac{\pi D}{3}$$

- The most prominent force acting on the underground sewer pipes, would be 33.
 - (a) compressive force (b) tensile force
- (c) bending force (d) all of these
- House connections and lateral sewers are widely made of 34.
 - (a) R.C.C.
- (b) P.C.C.
- (c) glazed stoneware (d) cost iron
- S.W. pipes are generally not used for sewer mains because they are : 35.
 - (a) weak in tension

- (b) weak in compression
- (c) hydraulically less efficient
- (d) less reistant to organic corrosion
- The maximum spacing of manholes on sewers having diameter more than 1.5 m is 36.
 - (a) 75 m
- (b) 150 m
- (c) 200 m
- (d) 300 m
- The gas, which is generally found present in sewers in 37.
- (b) CO₂
- (c) CH₄
- (d) all of these
- A drop manhole may be provided along a sewer line 38.
 - (a) when the sewer drops from a height of more than 0.6 m or so.
 - (b) when a branch sewer outfalls into it from a height of more than 0.6 m or so.
 - (c) to provide inspection chambers in the sewer lines
 - (d) for none of these

₹5

Water Supply and Sanitary Engineering

39. Ventilating columns are placed along a sewer line at intervals of about

- (a) 30-50 m
- (b) 75-100 m
- (c) 150-300 m
- (d) 500-750 m

The specific gravity of sewage is 40.

- (b) equal to 1
- (c) slightly less than 1

(d) slightly greater than 1

: ANSWERS :

| 31. (b) | 32. (c) | 33. (a) | 34. (c) | 35. (a) |
|---------|---------|---------|---------|---------|
| 36. (d) | 37. (d) | 38. (b) | 39. (c) | 40. (d) |

When a sewer line dropped below the hydraulic gradient line to pass it through an obstruction the arrangement is know as

- (a) inverted siphon
- (b) depressed weir
- (c) sag pipe
- (d) all of these

(Ventilation columns in sewers are provided to

- (a) help in escaping of foul gases
- (b) help in preventing spread of foul gases
- (c) to provide support to the sewers
- (d) none of these

Gases which are generally evolved during aerobic decomposition of sewage are: 43.

(a) $CO_2 + NH_3 + H_2S$

(b) $CO_2 + NH_3 + H_2S + CH_4$

(c) $CO_2 + NH_3 + SO_2$

(d) $CO_2 + NH_3 + SO_2 + CH_4$

Gases which are generally evolved during anaerobic decomposition of sewage, are

(a) $CO_2 + NH_3 + H_2S$

(b) $CO_2 + NH_3 + H_2S + CH_4$

(c) $CO_2 + NH_3 + SO_2$

(d) $CO_2 + NH_3 + SO_2 + CH_4$

1000 kg of sewage is estimated to contain approximately, total solids, equal to 45.

- (a) 0.5 1 kg
- (b) 2 5 kg
- (c) 5 10 kg (d) 10- 20 kg

46. Imhoff cone is used to measure, in sewage

(a) total solids

(b) total organic solids

(c) total inorganic solids

(d) settleable solids

Minimum D.O. prescribed for a river/stream to avoid fish kills is 47.

- (b) 4 ppm
- (c) 8 ppm
- (d) 10 ppm

48. BOD₅ represents 5 days biochemical oxygen demand at a temperature of

- (a) 0°C
- (b) 20°C
- (c) 30°C
- (d) none of these

The pH of fresh sewage is usually 49.

- (a) less than 7
- (b) more than 7
- (c) equal to 7 (d) equal to zero

The detention period adopted for sewage sedimentation tanks is of the order of

The detention period adopted for oxidation ponds is of the order of

(c) 2-4 hr

(c) 1-2 weeks

(c) 12-36 hrs

(c) grit and sand are removed by grit chambers

(b) 4-8 hr

(b) 2-4 days

The detention period in a septic tank is of the order of

(b) 4-8 hrs

(d) all of the above are correct

58.

59.

60.

(a) 1-2 hr

(a) 24-36 hrs

(a) 2-6 hrs

(d) 24-36 hrs

(d) 2-6 weeks

(d) 2-4 days

: ANSWERS :

| | | | | 55 (-) |
|---------|---------|-------------|---------|---------|
| 51 (b) | 52. (a) | 53. (d) | 54. (b) | 55. (c) |
| 51. (b) | 32(u) | | 50 (4) | 60. (c) |
| 56. (b) | 57. (d) | 58. (a) | 59. (d) | 00. (0) |

The gas which may cause explosion is sewer is 61.

- (a) methane
- (b) carbon-monoxide (c) carnbon-di-oxide (d) ammonia

In sewers the velocity of flow usually should not be 62.

(a) more than 25 m/sec

- (b) less than 8 m/sec
- (c) less than self cleaning velocity
- (d) more than self cleaning velocity

Hydraulic mean radius is:

- (a) average of radii in a sewer line of varying cross-sections
- (b) heads difference between two points in circular pipe lines
- (c) cross-sectional area of sewer divided by wetted perimeter
- (d) mean radius of sewer line

Self-cleaning velocity of sewage flowing in pipe lines is usually 64.

- (a) 0.25 m/sec
- (b) 0.40 m/sec
- (c) 0.8 m/sec
- (d) 1.0 m/sec

- (e) 1.5 m/sec
- (f) 2.0 m/sec

In case of combined sewer, the dry weather flow is 65.

- (a) storm water flowing in it
- (b) domestic sewage flowing in it
- (c) industrial sewage flowing in it
- (d) both industrial and domestic sewage without storm water

For satisfactory working of a sludge digestion unit, the pH range of digested 66. sludge shluld be maintained in the range of

- (a) 3 to 5
- (b) 6.5 to 8
- (c) 8.5 to 10
- (d) any of these

The gas coming out from a sludge disgestion tank is 67.

(a) methane only

- (b) CO2 only
- (c) 70% CH₄ and 30% CO₂
- (d) 30% CH₄ and 70% CO₄

68. Leachate is a coloured liquid, that comes out of

(a) septic tanks

(b) sanitary land fills

(c) compost plants

(d) aerated lagoons

A nahani trap is provided 69.

- (a) at the head of each house drain
- (b) at the outfall end of each house drain
- (c) at the junction of two house drains
- (d) none of these

| 70. | A pipe which is installed in lealled. | nouse drainage to | preserve | the wate | er seal of traps, is |
|-----|---------------------------------------|-------------------------------|-------------------------|--------------------------|----------------------|
| | (a) vent pipe (b) wa | aste pipe (o |) soli pi | pe (d) | antisiphonage pipe |
| | | : ANSWERS : | | | |
| | 61. (a) 62. (c) | 63. (c) | 64. | (b) | 65. (d) |
| | 66. (b) 67. (c) | 68. (b) | 69. | (a) | 70. (d) |
| 71 | The spacing of bars in coars | se screens is gene | rally moi | re than | |
| 71. | | mm or so (| | | (d) 50 mm or so |
| 72. | Activated sludge treatment | | | | |
| , | (a) towns and smaller cities | | o) mediu | | cities |
| | (c) large sized cities | (0 | d) all of | them | |
| 73. | Trickling filter plants are p | referred for sewa | ge treatn | nent for | |
| | (a) towns and smaller cities | | o) m <mark>edi</mark> u | | cities |
| | (c) large sized cities | (| d) (a) an | d (b) bot | h |
| 74. | The working conditions in i | mchoff tanks are | | | |
| | (a) aerobic only | | o) anerol | - | |
| | (c) aerobic in lower compart | | | | |
| | (d) anaerobic in lower comp | artment and aerob | ic in uppe | er compai | tment |
| 75. | The secondary treatment of | | | | |
| | (a) bacteria (b) al | , | , | | (d) none of these |
| 76. | The gas, which is evolved in | n a sludge digesti | on tank, | i <mark>s mainl</mark> y | composed of |
| | | | c) H ₂ S | | (d) CH ₄ |
| 77. | Dissolved organic solids in | | | | |
| | (a) coagulation · (b) hy | | | | |
| 78. | In a shallow waste stabilisa | | | | 7: |
| | (a) aerobic bacteria only | • | b) algae | | |
| | (c) dual action of aerobic ba | | | | |
| 79. | The max ^m efficiency of BO | D ₅ removal is acl | | | |
| | (a) oxidation ditch | · · | | tion pond | , |
| | (c) aerated lagoon | · | • | ing filter | |
| 80. | Compostion and lagooning | | | | |
| | (a) filtration | | • | entation | |
| | (c) sludge digestion | (| d) sewag | ge disposa | al |

| 4/ | 74 | | Water | Supply and | Sanita | ry Engineering |
|-----|----------------------------|--------------------------------|-----------------|------------------|----------|-------------------|
| _ | | <u>.</u> | : ANSWERS | | | |
| | 71. (d) | 72. (c) | 73. (d) | 74. (| d) | 75. (a) |
| L. | 76. (d) | 77. (c) | 78. (c) | | a) | 80. (d) |
| 81. | 1 | of liquid waste | flowing in se | wer line durin | g the p | eriod of rainfall |
| | is carred | | | | | |
| 01 | (a) industria | ` ' | rm sewage | (c) sanitary s | ewage | (d) all of these |
| 82. | Ine most sui | table cross-secti | on of a sewer | to carry the co | ombine | d flow is |
| | (a) circular (e) horse-sho | | g-shaped | (c) square | (0 | d) rectangular |
| 83. | • • | | | | | |
| 001 | (a) average d | f sewer too D.W | | | | • |
| | = | average demand | (b) | twice the aver | | • |
| 84. | | | | from time the | average | e demand |
| | (a) out all se | e collecting the | sewage from r | | | ers is know as: |
| | (c) an interce | | | (b) main sewe | | |
| 85. | | diameter of ma | nhale apenina | (d) trunk sew | er | |
| | | (b) 40 cm (c) | | d) 60 cm | (- | . 70 |
| 86. | | ds treat the sew | | u) oo em | (e |) 70 cm |
| | (a) sedimenta | | on of algae | (c) aerobio | hacter | ia |
| | (d) Oxidation | | (b) and (c) | (6) 4616816 | Daciei | ıa |
| 87. | The trickling | filter works on | | | | |
| | (a) bilogical a | ction (b) filtra | ation process | (c) both (a) and | (b) (d |) none of these |
| 88. | Traps are pro | vided in the <mark>sa</mark> n | itary system | 3 (2) | (-) (- | y none of those |
| | (a) to trap the | rats, snakes ente | ering the sewer | S | | |
| | (b) preventing | the foul gases fr | om sewers ent | ering the reside | nce/toil | lets |
| | (c) to dissolve | the foul gases in | n sewage | | | |
| 00 | (d) to increase | the quick dispos | sal of sewage | | | |
| 89. | | oints are provid | | | | |
| 10 | (a) G.I. pipes | (b) P.V.0 | | (c) S.W. pipes | (d) | C.I.pipes |
| 90. | Usually minim | um diameter of | | re kept as | | |
| | (a) 200 mm | (b) 150 i | mm (| c) 100 mm | (d) | 75 mm |
| | (e) 50 mm | | | | | 1 |
| | | _ | ANSWERS : | | | |

| 1220771710 | <u>•</u> | |
|---|--------------------|--------------------|
| 81. (b) 82. (b) 83. (a) 86. (e) 87. (c) 88. (b) | 84. (d) 89. (d) | 85. (c) 90. (b) |

93. (c)

(c)

98.

94.

99.

(d)

(a)

92.

97.

(a)

(e)

91. (d)

96. (ç)

Downloaded From :www.EasyEngineering.net

95. (d)

100. (a)

101. Match list I with list II and select the correct answer using the codes given below the lists:

| the mais. | | | | List II |
|------------|--------------|-------|----|------------------|
| Lis | st I | | | |
| (Treat | ment units |) | | (Detention time) |
| A. Grit cl | hamber | | 1. | Six hours |
| | ry sedimenta | ition | 2. | Two minutes |
| | ited sludge | | 3. | Two hours |
| D. Sludge | | | 4. | Twenty days |
| Codes: | Α | В | С | D |
| (a) | 3 | 1 | 4 | 2 |
| (b) | 2 | 3 | 1 | . 4 |
| (c) | 2 | 1 | 3 | 4 |
| | 1 | 2 | 3 | 4 |
| (d) | 1 | | | a 1 |

- 102. Which one of the following statements is true of tricking filter sludge?
 - (a) It has a comparatively low sludge volume index
 - (b) It is more difficlt to dewater than activated sludge
 - (c) It has a comparatively low concentration of sludge solids
 - (d) It is bulky
- 103. Match List I with list II and select the correct answer using the codes given below the lists:

| List I (Pipe material) A. Concrete sewer B. Stone ware sewer | List II (Property of material) 1. Cannot withstand high external load. 2. Corrosion resistance in most natural | | | | | |
|--|--|----------------|--|-----|--|--|
| C. Cost iron sewer D. Steel sewer | Cost iron sewer 3. Steel sewer 4. | | Resitant to corrosion from most acids Unsuitable where soil contains excessive | | | |
| Codes: | A | sulphates B | C | D 4 | | |

| Codes: | | Α | В | С | D |
|---------|-----|-------------------|---|---|---|
| Coucs . | (A) | 1 | 2 | 3 | 4 |
| | (B) | 4 | 3 | 2 | 1 |
| | ` ' | 1 1 | 1 | 2 | 3 |
| | (C) | 4 | 1 | 3 | 4 |
| | (D) | ') | 1 | | |

- 104. BOD is preferred to COD as an index of sewage concentration, because
 - (a) BOD represents both carbonaceous and nitrogenous organic matter, while COD may indicate carbonaceous matter only.
 - (b) BOD test is easier to perform and gives more reliable results

- (c) BOD relates specifically to putrescible organic matter which is the most objectionable sewage constituent.
- (d) COD relates to the impurities which can only be removed by chemical treatment which is expensive.
- 105. Under Indian conditions, the average per capits contribution of BOD is
 - (a) 10 to 20 gm/d

(d)

- (b) 20 to 35 gm/d
- (c) 35 to 50 gm/d (d) 50 to 70 gm/d

106. Match list I with list II and select the correct answer using the codes given below the lists

| | List I | Lis | st II | | | |
|-----|------------------|-------|--|-------------|-------------|--|
| A. | Waste pipe | 1. | Carries | waste wate | r | |
| B. | Soil pipe | 2. | Carries | liquid wast | es that do | not include human excreta |
| C. | Vent pipe | 3. | 3. Preserves the water seal of traps through atmospheric air | | | |
| D. | Antisiphona pipe | ge 4. | Carries I | liquid wast | es includir | ng human excreta |
| | | 5. | to preve | | pressures | om drainage system in order and excessive pressure and es. |
| Coc | les : | A | В | C | D | |
| | (a) | 2 | 4 | 5 | 3 | |
| | (b) | 3 | 5 | | 2 | |
| | (c) | 4 | 5 | 1 | 7/2 | |

107. Match List I with List II and select the correct answer using the codes given below the list:

3

| | Li | st I | | | | List II |
|-----|--------|------------|----|---|----|--------------------|
| | (Ca | iuse) | | | | (Effect) |
| A. | Carbo | n monoxic | le | | 1. | Acid rain |
| B. | Carbo | n dioxide | | | 2. | Explosion |
| C. | Metha | ne | | | 3. | Asphyxiation |
| D. | Sulphi | ur dioxide | | | 4. | Green house effect |
| Cod | les : | Α | В | С | | D |
| | (a) | 2 | 3 | 1 | | 4 |
| | (b) | 3 | 4 | 2 | | 1 |
| | (c) | 1 | 3 | 4 | | 2 |
| | (d) | 4 | 2 | 1 | | 3 |

108. Sewage sickness ralates to:

- (a) tioxicity of sewage interferring with response to treatment.
- (b) destruction of aquatics flora and fauna due to gross pollution of receiving bodies of water by sewage.
- (c) reduction in the waste purifying capacity of the soil.
- (d) clogging of pores in soil due to excessive application of sewage to land, obstructing aeration and leading to spetic conditions.

109. Under natural conditions of flow, an unpolluted river would contain

- (a) more dissolved oxygen in summer than in winter
- (b) less dissolved oxygen in summer than in winter
- (c) more or less the same amount of dissolved oxygen in winter and summer.
- (d) the least amount of dissolved oxygen during the floods

110. Consider the data presented in the following table:

| Temperature in °C | BOD reaction rate constant (K) |
|-------------------|--------------------------------|
| 20 | 0.01 |
| 30 | 0.02 |
| 10 | 0.005 |

In the data presented above, the value of K

- (a) should have remained constant
- (b) should have decreased with increase in temperature
- (c) should have remained the same at 20°C and 30°C
- (d) has followed the correct trend

: ANSWERS

| 101. (b) | 102. (a) | 103. (b) | 104. (c) | 105. (d) |
|----------|----------|----------|----------|----------|
| 106. (a) | 107. (b) | 108. (d) | 109. (b) | 110. (d) |

111. For the combined sewerage system egg shaped sewers are preferred because:

- (a) their construction is economical
- (b) they are structurally more stable
- (c) their maintenance is easier
- (d) they offer good flow velocity during the dry weather flow condition.

112. Match List I with List II and select the correct answer using the codes given below the lists:

List I List II (Treatment operation/process) (Terms/Description) Settling in primary sedimentation tank Sludge volume index 1. A. 2. Settling in secondary sedinmentation tank Β. Thickening of sludge 3. Filtration in trickling filter C. Scum removal 4. Activated sludge process D. Recycling of efflent C D Codes: A 1 2 4 3 (a) 3 (b) 4 2 3 2 (c) 3 2 (d) 4

An aeration basin with a volume of 400 m³ contains mixed liquor with suspended solid concentration of 1000 mg/l. The amount of mixed liquor suspended solids in the tank is

- (a) 500 kg
- (b) 250 kg
- (c) 600 kg
- (d) 400 kg

113. The following reactions take place during anaerobic digestion of organics.

1. Methane production

2. Alkaline fermentation

3. Acid fermentation

4. Acid regression

The correct sequence of these reactions is

- (a) 3, 4, 2, 1
- (b) 4, 3, 2, 1
- (c) 3, 4, 1, 2
- (d) 4, 3, 1, 2

114. Traps are used in household drainage systems to:

- (a) prevent entry of foul gases in the houses
- (b) restrict the flow of water
- (c) provide a partial vacuum
- (d) trap the soild wastes

115. Modern coal-based thermal power stations pollute the atmosphere by adding:

(a) NO_x and SO₂

- (b) NO_x, SO₂ and SPM
- (c) NO_v, SO₂, SPM and CO
- (d) NO_x, SPM and CO.

116. Consider the following statements:

Excessive growth of water weeds in a water body is attributed to the:

- 1. increases in the benthic organisms including bacteria
- 2. inbalance in aquatic ecosystem
- 3. execssive inflow of nutrients

Of these statments:

(a) 1, 2 ande 3 are correct

(b) 1 and 2 are correct

(c) 1 and 3 are correct

(d) 2 and 3 are correct

: ANSWERS :

| 111. (d) | 112. (d) | 113. (a) | 114. (a) | 115. (b) |
|----------|------------------|----------|----------|----------|
| 116. (d) | 11 7. (b) | 118. (c) | 119. (b) | 120. (b) |

121. One litre of sewage, when allowed to settle for 30 minutes gives a sludge of 27 cm₃. If the dry weight of sludge is 3.0 gm, then its sludge volume index will be:

(a) 9

(d)

(b) 24

(c) 30

(d) 81

| | | Lis | st-I | | | | List-II |
|----------------------|------|----------|-----------|--------|------|-----|---|
| (Contaminant) | | | |) | | | (Environmental significance) |
| | A. | Suspen | ided soli | ds | | 1. | May cause entrophication |
| | B. | Nutrie | ıts | | | 2. | Toxic, may interfere with effluent reuse |
| | C. | Heavy | metals | | | 3. | May interfere with effluent reuse |
| | D. | Dissol | ved inor | ganic | | 4. | Cause sludge deposits |
| | | solids | | | | | |
| | Cod | les: | Α | В | C | D |) |
| | | (a) | 4 | 1 | 2 | 3 | |
| | | (b) | 2 | 3 | 4 | 1 | |
| | | (c) | 4 | - 3 | 2 | 1 | |
| | | (d) | 2 | 1 | 4 | 3 | |
| 175. | Mat | tch List | -I with | List-I | Land | sel | lect the correct answer using the codes given |
| | belo | w the l | ists : | | | | |
| | | ·Li | st -I | | | | List-II |
| (Physical properties | | | | erties | | | (Limiting value) |
| | | of filte | ring ma | terial | for | | |
| | | tricklin | ng filter | s) | | | |

1.

2.

3.

4.

12.0

100.0

4.0

2.6

Codes: A B C D
(a) 3 1 2 4

Crushing strength, N/mm²

B.

D.

Hardness

Percent wear

Specific gravity

(a) 3 1 2 4 (b) 2 4 3 1

(c) 3 4 2 (d) 2 1 3

176. What is eutrophication of lakes primarily due to?

(a) Multiplication of bacteria (b) Excessive inflow of nutrients

(c) Increase in benthic organisms (d) Thermal and density currents

177. The daily cover of MSW landfills consists of which one of the following?

(a) Compacted soil (b) Geomembrane (c) Geotextile (d) Geocomposite

178. Match List-I with List-II and select the correct answer using the codes given below the lists:

| | List-I | | | I | ist-II | |
|----|--------------|--------|----|-----------|-----------|--|
| | (Air pollu | tant) | | | | an Health) |
| A. | Particulates | | 1. | Impairs | transpor | rt of O ₂ in bloodstream |
| В. | Carbon mo | noxide | 2. | Trritatio | n of mu | cous membrances of respiratory tract |
| C. | Sulphur ox | ides | 3. | Causes | coughin | g, shortness of breath, headache, etc. |
| D. | Photochem | ical | 4. | Causes | respirate | ory illness |
| | oxidants | | | | | |
| | Code: | Α | В | C | D | |
| | (a) | 2 | 3 | 4 | 1 | |
| | (b) | 4 | 1 | 2 | 3 | |
| | (c) | 2 | 1 | 4 | 3 | |
| | (d) | 4 | 3 | 2 | 1 | |

- 179. A machine in a steel plate fabricating industry is found to be producting a sound level of 50 dB. In the expansion plans, one more such machine needs to be added. What will be the combined noise level?
 - (a) 80-100 dB
- (b) 101-150 dB
- (c) 51-70 dB
- (d) 40-50 dB
- 180. Which one of the following is the correct statement?

A heterotroph is an organism that obtains

- (a) its cell carbon from an inorganic source
- (b) its energy from the oxidation of simple inorganic compounds
- (c) its cell carbon as well as its energy from organic matter
- (d) its energy from a natural ecosystem.

: ANSWERS :

| 171. (b) 172. (c) 173. (a) 174. (a) 175. (d) 176. (b) 177. (b) 178. (c) 179. (c) 180. (c) |
|---|
|---|

- 181. In which type of lakes, does a perfect ecological equilibrium among the producers, decomposers, and consumer groups of organisms exist?
 - (a) Senescent lakes

(b) Mesotrophic lakes

(c) Oligotrophic lakes

- (d) Eutrophic lakes
- 182. Which one of the following types of settling phenomenon can be analysed by the classic sedimentation laws of Newton and Stokes?
 - (a) Discrete setting

(b) Flocculent settling

c) Hindered settling

(d) Compression settling

183. Match List I (Treatment Process) with List II (Related Terms) and select the correct answer using the code given below the lists:

List I List II Attached growth system A. Lagoons 1. Trickling filter 2. Algae-bacteria symbiotic relationship B. Oxidation ponds 3. Extended aeration 4. D. Activated sludge process Low cost treatment method В C D Α 3 2 (a) 4 (b) 2 1 4 3 (c) 4 2 (d) 2 3

184. Which is the major pollutant present in photochemical smog?

- (a) PAN (b) SO_2 (c) HC (d) NO_2
- 185. Consider the following statement?

In solid wast management:

- 1. Density separation of solid wastes can be accoplished by air classifiers.
- 2. Iron recovery from solid wastes can be done by magnetic separators.
- 3. Aluminium separation from solid wastes can be accomplished by eddy current separators.

Which of the statements given above are correct?

- (a) 1 and 2 (b) 2 and 3 (c) 1 and 3 (d) 1, 2 and 3
- 186. Which of the following statement in not correct?
 - (a) Settling and sludge digestion occurs in septic tanks in one compartment
 - (b) Settling and sludge digestion occurs in imhoff tank in different compartments
 - (c) Septic tank is a low-rate anaerobic unit whereas an inhoff tank is a high rate anaerobic unit
 - (d) The rate of sludge accumulation in septic tank is approximately 40-70 litres/capita/year

187. Consider the following statements in regard to aerobic and anaerobic treatment processes:

- 1. Biomass production in the aerobic treatment process is more as compared to the anaerobic treatment process.
- 2. Start-up period is more in the aerol o treatment process as compared to the anaerobic treatment process.
- 3. Energy consumption and production is more in the aerobic treatment process as compared to the anaerobic treatment process.

| | | | | | - of concentration |
|------|---|------------------------|------------|------------------------|-----------------------------|
| 195. | For the design of a | storm sewer in a dra | inage are | ea, if the time | e of concentration |
| | is 20 min, then the | duration of rainfall v | will be ta | ken as : | |
| | (a) 10 min | (b) 20 min. | (c) 3 | 0 min. | (d) 40 min. |
| 106 | From ecological co | nsiderations, the mir | nimum le | vel of dissolv | ved oxygen (D.O.) |
| 170. | managery in the riv | ers and streams is: | | | |
| | | (b) 2 mg/l | (c) 4 | mg/1 | (d) $8 \text{ mg/}l$ |
| | (a) $1 \text{ mg/}l$ | Collowiing pollutants | | | |
| 197. | Which one of the | ollowling ponutants | or pairs | or possession | |
| | photochemical reac | tions ? | Y | NANT J NITT | (d) NH, and CO |
| | (a) CO alone | (b) O_3 and PAN | (c) F | AN and Nn ₃ | (d) NH ₃ and CO. |
| 198. | Organisms that mi | neralise organic matt | ter in an | ecosystem a | re called : |
| | (a) producers | | (c) (| lecomposers | (d) carnivorous |
| 100 | Aerobic method of | composting practised | d in India | is called: | |
| 177. | (a) Bangalore meth | | (b) 1 | Nagpur metho | d |
| | • | | (d) | Indore method | 1 . |
| | (c) Delhi method | rs a flowing river, th | ie ranid (| depletion of | dissolved oxygen is |
| 200. | | rs a mowning men, to | ic inpic. | | |
| | due to: | | | | |
| | (a) change in temp | erature of river water | | | |
| | (b) the suspended | particles in river and | waste | | |
| | (c) respiratory acti | vity of aquatic plants | | | |
| | (d) microbial activ | | | | |
| | (u) Interoplat activ | > | | | |

: ANSWERS

| 191. (a) 192. (b) 196. (c) 197. (b) | 193. (d) | 194. (c) | 195. (b) |
|-------------------------------------|----------|----------|----------|
| | 198. (c) | 199. (d) | 200. (b) |

201. Match List 1 (unit) with List II (purpose) and select the correct answer using the codes given below the lists:

List II

List I (Unit)

(Purpose)

A. Leaping weir

1. To prevent grit, sand, debris etc. from entering the

- storm sewer.

 2. To carry the sewer below a stream or railways line
- B. Gutter inlet
 C. Inverted syphon
 D. To carry the sewer below a stream or ranways line
 To drain rain water from roads to the storm sewer.
- D. Catch basin 4. To separate st rm water and the sanitary sewage.

| 1 | ۵ | G |
|---|-----|---|
| - | - 2 | o |

Water Supply and Sanitary Engineering

| Codes: | A | В | C | . D |
|--------|---|-----|---|-----|
| (a) | 4 | 3 | 1 | 2 |
| (b) | 4 | - 3 | 2 | 1 |
| (c) | 3 | 4 | 2 | 1 |
| (d) | 3 | 4 | 1 | 2 |

202. Match List I (Treatment Units) with List II (Types of processes) and select the correct answer using the codes given below the list:

(Treatment units)

- Trickling filter Activated sludge processes
- Oxidation ditch

List II

(Types of processes)

- Symbiotic 1.
- Extended aeretion 2.
- Suspended growth 3.
 - Attached growth

| D. Oxida | tion pone | d | | 4. |
|----------|-----------|---|-----|-------|
| Codes: | A | В | C | D |
| (a) | 3 | 4 | 2 | 1 |
| (b) | 4 | 3 | 1 | 2 |
| (c) | 3 | 4 | 1 | 2 |
| (d) | 4 | 3 | 2 | 7 - 1 |
| ` ' | _ | | .11 | in wo |

203. If the moisture content of a sludge is reduced from 98% to 96%, the volume of sludge will decrease by

Hint:
$$V_2 = V_1 \frac{(100 - 98)}{(100 - 96)} = \frac{V_1}{2} = 0.5 V_1$$

204. In a high rate Trickling filter, the problem of ponding can be solved by:

(a) flooding and raking

- (b) chlorination and supply of air
- (c) raking and chlorination

(d) flooding and supply of air

205. Consider the following statements:

The process of activated sludge can be explained as:

- a physical action whereby the finer suspended particles of sewage form a sublayer for a bacterial film at the surface.
- a chemical action whereby the finer suspended particles and colloidal solids are combined into masses of large bulk.
- a biochemical action whereby the sludge flocs so formed act as vehicle for acrobic bacteria oxidising the organic matter.

Which of these statements are correct?

- (a) 1, 2 and 3
- (b) 1 and 2
- (c) 2 and 3
- (d) 1 and 3

Objective Civil Engineering

497

| 206. | Sewage may | be | disposed | of | without | treatment | into | a | water | body i | f (| the | availa | able |
|------|-------------|----|----------|----|---------|-----------|------|---|-------|--------|-----|-----|--------|------|
| | dilution is | | | | | | | | | - | | | | |

- (a) less than 150
- (b) more than 150
- (c) more than 300 (d) more than 500

207. In a sanitary plumbing of buildings, a two pipe system signifies :

- (a) separate soil-pipe and waste pipe without yent pipes
- (b) A soil cum waste pipe and a ventillating pipe
- (c) Separate soil and waste pipe and a common ventillating pipe
- (d) Separate soil pipe and waste pipe, each with its own vent pipe.

208. The least expensive and most suitable excreta disposal unit for rural areas would be the:

- (a) soak pit
- (b) pit privy (c)
- leaching casspool (d)

septic tank

209. Which one of the following terms correctly describes 'Biomagnification'?

- (a) Reproduction of micro-organisms
- (b) Observation of micro-organisms under a microscope
- (c) Ability of micro-organisms to form zooleal film
- (d) Concentration of toxic materials in the food chain.

210. Which one of the following comprehensive classifications is used for different types of solid waste?

- (a) Residential, commercial and treatment plant wastes
- (b) Food, demolition and construction wastes
- (c) Municipal, industrial and hazardous wastes
- (d) Rubbish, special wastes and wastes from open areas

: ANSWERS :

| <u> </u> | | 1 | | |
|----------|----------|----------|----------|-------------|
| 201. (b) | 202. (d) | 203. (d) | 204. (c) | 205. (a) |
| 206. (d) | 207. (d) | 208. (b) | 209. (d) | 210. (c) |

211. 'Air binding' may occur in

- (a) Sewers
- (b) Artesian well
- (c) Aerator
- (d) Filter

212. The description of solid waste collected is as follows:

Night soil

35 t

Rubbish

40 t

Debris

25 t

Garbage

40 t

The organic solids in the above composition is

- (a) 35 t
- (b) 60 t
- (c) 100 t
- (d) 75 t

Objective Civil Eng. \ 2016 \ 63

498

| 213. | In | an activa | ated sl | udge | process, | the | sludge | volume | index | can | be | controlled | by |
|------|----|-----------|---------|------|----------|-----|--------|--------|-------|-----|----|------------|----|
|------|----|-----------|---------|------|----------|-----|--------|--------|-------|-----|----|------------|----|

- (a) Aeration
- (b) Adding chlorine
- (c) Reducing recycling
- (d) Increasing the depth of aeration tank

214. A primary sedimentation tank is not required for

- (a) Activated sludge system
- (b) Extended aeration system
- (c) Trickling filtration system
- (d) Tapered activated sludge process using pure oxygen for aeration.

215. The term 'Refuse' generally does not include

(a) Putrescible solid waste

- (b) Excreta
- (c) Non-putrescible solid waste
- (d) Ashes

216. Bangalore method and Indore method of disposing solid wastes are

- (a) Identical
- (b) Different as Bangalore method is an anaerobic method
- (c) Different as Bangalore method does not contain human excreta
- (d) Different as Indore method is an incineration method

217. Which one of the following statements explains the term pyrolysis?

- (1) Solid waste is heated in closed containers in oxygen free atmosphere
- (2) They must be provided at the head of all sewers.
- (3) They must be provided at every junction of two or more sewers.
- (4) They must be provided at every 100 m along straight runs of sewers.

Which of the statements given above are correct?

- (a) 1, 2, 3 and 4
- (b) 1, 3 and 4
- (c) 1, 2 and 3
- (d) 2 and 4

218. Which of the following pairs is correctly matched?

(a)
$$\frac{BOD}{COD} = 0$$
;

Waste-water is toxic

(b)
$$\frac{BOD}{COD} = \le 0.20$$
:

Acclimatization of seed is necessary

(c)
$$\frac{BOD}{COD} = \ge 0.6$$
:

Waste-water is non-biodegradable

(d)
$$BOD = COD = 0$$
:

Waste -water is devoid of organic matter

Objective Civil Engineering

- 219. In aerobic conditions the microbial decomposition of organics results in the formation of which one of the following?
 - (a) Stable and objectionable end products
 - (b) Unstable and objectionable end products
 - (c) Unstable and acceptable end products
 - (d) Stable and unobjectionable end products
- 220. Consider the following statements:

The basic difference between water pipes and sewer pipes is:

- 1. in the material used for pipes
- 2. in the pressure of the liquid flow
- 3. in the suspended solids they carry

Which one of the statements given above is/are correct?

- (a) 1 and 3
- (b) 1 only
- (c) 2 and 3
- (d) 1, 2 and 3

: ANSWERS :

| Г | | | 010 (5) | 214 (6) | 215. (b) |
|---|-------------|----------|----------|----------|----------|
| ١ | 211. (d) | 212. (d) | 213. (c) | 214. (b) | ` ´ |
| ļ | 216. (b) | 217. (a) | 218. (d) | 219. (d) | 220. (d) |



Building Planning

| | | | MCQS | SECTION AND DESCRIPTION OF THE PARTY OF THE | | erropology process for the con- |
|-----|--------------------------|--------|------------------|---|------------------|---------------------------------|
| 1. | Proper placing of buil | ding | and its compon | ent r | oms with res | pect to the natural |
| | weathering elements s | uch a | ıs sun, wind, an | id raii | n and environ | mental factors like |
| | topography is called | | · | | | (1) Consider |
| | (a) Planning | | Design | | Orientation | (d) Grouping |
| 2. | A room which receives | | | | | 18 Known as |
| | (a) aspect | ` ′ | prospect | (c) | elegance | (d) circulation |
| 3. | The aspect preferred | for li | | | | (1) NI, the most |
| | (a) North east | (b) | South east | ٠, | South west | (d) North west |
| 4. | In India, wind direction | | | ionsoc | on is | (1) Month word |
| | (a) North east | | South east | (c) | South west | (d) North west |
| 5. | Better aspect for vera | | 1// / / / / / | | | (4) 07 |
| | (a) west, sw | ` ' | NW | | NE | (d) SE |
| 6. | Which IS code is refe | rred | for building pla | anning | regulation at | na services: |
| | (a) IS: 456–2000 | | | | IS: 1893-20 | |
| | (c) IS: 800–2007 | | | (d) | SP -7 (2005) |) – NBC |
| 7. | As per NBC - 2005, 1 | | itals are | * . | | 1141 |
| | (a) Business buildings | 5 | | • • | Assembly bu | |
| | (c) Institutional build | | tion. R | | Mercantile b | |
| 8. | For good orientation, | | the direction of | longe | er outer wans | should be |
| | (a) East-west direction | n | | , , | North-south | |
| | (c) NE-SW direction | | | (d) | NW-SE dire | etion |
| 9. | For better privacy in | a ro | om | | | la ha myayidad |
| | (a) Door should be pl | aced | near the corner | (b) S | ingle shutter do | oor should be provided |
| | (c) Provision of grou | nd g | lass for windows | s (d) a | Il of the above | • . • |
| 10. | For residential buildi | ngs ' | window opening | | | |
| | (a) 5% of floor area | | | • |) 10% of floo | |
| | (c) 20% of floor area | a | | (d |) 25% of floo | гагеа |

| 11. | For Drawing room and bedroom minimu | ım no. of air change | es per hour required |
|----------|--|------------------------------|-------------------------|
| | for good ventilation are | | |
| | (a) 1 (b) 3 (c) | 6 (d) | 10 |
| 12. | For roads wider than 12 m, setback reco | ommended is | |
| | (a) 1.0 m (b) 1.5 m | (c) 2.0 m | (d) 3.0 m |
| 13. | The angle of light plane commonly adop | oted is | |
| | (a) 30° (b) 45° | (c) 63.50° | (d) 55° |
| 14. | FSI is | | |
| | (a) Plot area/total area of all floors | (b) total area of | all floors/plot area |
| | (c) Plot area/area of one floor | (d) area of one | floor/plot area |
| 15. | In Ahmedabad city on a BRTS road if po | ermissible FSI is 4, to | otal area of all floors |
| | that can be constructed in a plot of 100 | | |
| | (a) 250 m^2 (b) 1000 m^2 | (c) 4000 m^2 | (d) 2000 m ² |
| 16. | The usual percentage of consolidated op | en plot (C.O.P.) area | is |
| | (a) 10% (b) 20% | (c) 30% | (d) 40% |
| 17. | Area of Mezzanine floor is restricted to | | |
| | (a) $\frac{1}{2}$ of the area of that floor | (b) $\frac{1}{3}$ of the are | ea of that floor |
| | (c) $\frac{1}{4}$ of the area of that floor | (d) $\frac{1}{5}$ of the are | |
| 18. | Maximum permissible covered area for | a residential plot of s | |
| | (a) 66.67 % (b) 50 % | (c) 40 % | (d) 33.33 % |
| 19. | | | |
| | (a) 60% (b) 50% | (c) 40% | (d) 25% |
| 20. | Plinth height generally provided for bu | | |
| | (a) 0.30 m (b) 0.45 m | (c) 0.75 m | (d) 1.0 m |
| 21. | Minimum width of habitable room is | | |
| | (a) 1.8 m (b) 2.1 m | (c) 2.5 m | (d) 3.0 m |
| 22. | Minimum width required for bath room | n is | |
| | (a) 1.0 m (b) 1.2 m | (c) 1.5 m | (d) 2.0 m |
| 23. | Minimum floor area required for bath | | |
| , | (a) 1.2 m^2 (b) 1.5 m^2 | (c) 1.8 m^2 | (d) 2.5 m^2 |
| 24. | Minimum headroom required on stairc | ase landing is | |
| | (a) 1.5 m (b) 1.8 m | (c) 2.1 m | (d) 2.5 m |
| | ર ૧ | | • |

38. A drawing sheet designated as Ao has size

(b) $594 \times 841 \text{ mm}$

(a) $841 \times 1189 \text{ mm}$

(c) $420 \times 594 \text{ mm}$ (d) $297 \times 420 \text{ mm}$

39. Who is known as the divine architect who spread the knowledge of shilpa-shastras

- (a) Vishveshvaria
- (b) Kautilya
- (c) Vishva karma (d) Chanakya

40. Slum area are commonly known as

(a) Bustees in kolkata

(b) Jhuggi in new Delhi

(c) Cheries in chennai

(d) all of the above

41. Match correctly

Smart city

Country

- King Abdullah
- A. Korea

2. Lavasa B. Saudi Arabia

3. Songdo C. India

Nepolis

- D. Cyprus
- 1-B, 2-C, 3-D, 4-A (a)
- (b) 1-B, 2-C, 3-A, 4-D
- (c) 1-C, 2-B, 3-D, 4-A
- (d) 1-C, 2-B, 3-A, 4-D

42. The type of planning system of Gandhinagar city is

- (a) Concentric and radial street system
- (b) rectangular grid iron system
- (c) Rectangular combined with radial street system
- (d) Organic street system

43. Indian city with rectangular grid iron pattern is

- chandigarh
- (b) Gandhinagar (c) Jaipur
- (d) all of the above

: ANSWERS :

| | 1. (c) | 2. (a) | 3. (b) | 4. (c) | 5. (a) |
|-----|---------|---------|---------|---------|---------|
| | 6. (d) | 7. (c) | 8. (b) | 9. (d) | 10. (b) |
| | 11. (b) | 12. (c) | 13. (c) | 14. (b) | 15. (c) |
| | 16. (c) | 17. (b) | 18. (c) | 19. (a) | 20. (b) |
| | 21. (b) | 22. (b) | 23. (c) | 24. (c) | 25. (b) |
| | 26. (d) | 27. (a) | 28. (b) | 29. (d) | 30. (c) |
| | 31. (d) | 32. (c) | 33. (b) | 34. (b) | 35. (c) |
| | 36. (a) | 37. (c) | 38. (a) | 39. (c) | 40. (d) |
| | 41. (b) | 42. (b) | 43. (d) | | |
| - 1 | l i | 1 | Í | Í | T . |



SYLLABUS

GATE -SYLLABUS

SECTION-1: ENGINEERING MATHEMATICS

Linear Algebra: Matrix algebra: systems of linear equations; Eigen values and Eigen vectors.

Calculus: Functions of single variable; Limit, continuity and differentiability; Mean value theorems, local maxima and minima, Taylor and Maclaurin series; Evaluation of definite and indefinite integrals, application of definite integral to obtain area and volume; Partial derivatives; Total derivative; Gradient, Divergence and Curt, Vector identities, Directional derivatives, Line, Surface and Volume integrals, stokes, Gauss and Green's theorems.

Ordinary Differential Equation (ODE): First order (linear and non-linear) equations; higher order linear equations with constant coefficients; Euler-Cauchy equations; Laplace transform and its application in solving linear ODEs; initial and boundary value problems.

Partial Differential Equation (PDE): Fourier series; separation of variables; solutions of one-dimensional diffusion equation; first and second order one-dimensional wave equation and two-dimensional Laplace equation.

Probability and statistics: Definitions of probability and sampling theorems; Conditional probability; Discrete Random variables; Poisson and Binomial distributions; Continuous random variables; normal and exponential distributions; Descriptive statistics — Mean, median, mode and standard deviation; Hypothesis testing.

Numerical MEthods: Accuracy and precision; error analysis, Numerical solutions of linear and non-linear algebraic equations; Least square approximation, NEwton's and Lagrange polynomials, numerical differentiation, integration by trapezoidal and simpson's rule, single and multi-step methods for first order differential equations.

SECTION-2: STRUCTURAL ENGINEERING

Engineering Mechanics: System of forces, free-body diagrams, equilibrium equations; internal forces in structures; friction and its applications, kinematics of point mass and rigid body; Centre of mass; Euler's equations of motion; impulse-momentum; Energy methods; Principles of virtual work.

Solid Mechanics: Bending moment and shear force in statically determinate beams; Simple stress and strain relationships; Theories of failures; Simple bending theory, flexural and shear stresse, shear centre; Uniform torsion, buckling of column, combined and direct bending stresses.

Structural Analysis: Statically determinate and indeterminate structures by force/energy/methods; Method of superposition; Analysis of trusses, arches, beams, cables and frames; Displacement methods: Slope deflection and moment distribution methods; Influence lines: stiffness and flexibility methods of structural analysis.

Construction Materials and Management: Construction materials: structural steel – composition, material properties and behaviour; Concrete – constituents, mix design, short-term and long-term properties; Brick and mortar; Timber; Bitumen. Construction Management: Types

of construction projects; Tendering and construction contracts; Rate analysis and standard specifications; Cost estimation; Project planning and network analysis—PERT and CPM.

Concrete Structures: Working stress, limit state and ultimate load design concepts; Design of beams, slabs, columns, Bond and development length; Prestressed concrete; Analysis of beam sections at transfer and service loads.

Steel structures: Working stress and limit state design concepts; Design of tension and compression members, beams and beam-columns, column bases; Connections – simple and eccentric, beam-column connections, plate girders and trusses; Plastic analysis of beams and frames.

SECTION 3: GEOTECHNICAL ENGINEERING:

Soil Mechanics: Original of soils, soil structure and fabric; Three-phase system and phase relationships index properties, Unified and Indian standard soil classification system; Permeability-one dimensional flow, Darcy's law; Seepage through soils - two-dimensional flow, flow nets, uplift pressure, piping; Principle of effective stress, capillarity, seepage force and quicksand condition; Compaction in laboratory and field conditions; One-dimensional consolidation, time rate of consolidation; Mohr's circle, stress paths, effective and total shear strength parameters, characteristics of clays and sand.

Foundation Engineering: Sub-surface investigations - scope, drilling bore holes, sampling, plate load test, standard penetration and cone penetration tests; Earth pressure theories - Rankine and Coulomb; stability of slopes - finite and infinite slopes, method of slices and Bishop's method; stress distribution in soils - Boussinesq's and Westergaard's theories, pressure bulbs; shallow foundations - Terzaghi's and Meyerhoff's bearing capacity theories, effect of water table; Combined footing and raft foundation; Contact pressure; Settlement analysis in ands and clays; Deep foundations - types of piles, dynamic and static formulae, load capacity of plles in sands and clays, pile load test, negative skin friction.

SECTION 4: WATER RESOURCES ENGINEERING

Fluid Mechanics: Properties of fluids, fluid statics; Continuity, momentum energy and corresponding equations; Potential flow, applications of momentum and energy equations; Laminar and turbulent flow; Flow in pipes, pipe networks; Concept of boundary layer and its growth.

Hydraulics: Forces on immersed bodies: Flow measurement in channels and pipes; Dimensional analysis and hydraulic similitude; kinematics of flow, velocity triangles; Basics of hydraulic machines, specific speed of pumps and turbines; Channel Hydraulics - Energy-depth relationships, specific energy, critical flow, slope profile, hydraulic jump, uniform flow and gradually varied flow.

Hydrologic cycle, precipitation, evaporation, evapo-transpiration, watershed, infiltration, unit hydrographs, hydrograph analysis, flood estimation and routing reservoir cpacity, reservoir and channel routing, surface run-off models, ground water hydrology - steady state well hydraulics and aquifers; Application of Darcy's law.

Irrigation: Duty, delta estimation of evapo-transpiration; Crop water requirements; Design of lined and unlined canals head works, gravity dams and spillways; Design of weirs on permeable Objective Civil Eng. \ 2016 \ 64

foundation Types of irrigation systems, irrigation methods; Water logging and drainage, Canal regulatory works, cross-drainage structures, outlets and escapes.

Water and Waste Water: Qualoty standards, basic unit processes and operations for water treatment. Drinking water standards, water requirements, basic unit operations and unit processes for surface water treatment, distribution of water, Sewage and sewerage treatment, quantity and characteristics of wastewater. Primary, secondary and fertiary treatment of wastewater effluent discharge standards. Domestic wastewater treatment, quantity of characteristics of domestic wastewater, primary and secondary treatment, Unit operations and unit processes of domestic wastewater, sludge disposal.

Air pollution: Types of pollutants, their sources and impacts, air pollution meteorology, air pollution control, air quality standards and limits.

Municipal Solid Wastes: Characteristics, generation, collection and transportation of solid wastes, engineered systems for solid waste management (reuse/recycle, energy recovery, treatment and disposal).

Noise pollution: Impacts of noise, permissible limits of noise pollution, measurement of noise and control of noise pollution.

SECTION 6: TRANSPORTATION ENGINEERING

Transportation Infrastructure: Highway alignment and engineering surveys; Geometric design of highways - cross - sectional elements, sight distances, horizontal and vertical alignments; Geometric design of railway track; Airport runway length, taxiway and exit taxiway design.

Highway Pavements: Highway materials - desirable properties and quality control tests; Design of bituminous paving mixes, Design factors for flexible and rigid pavements; Design of flexible pavement using IRC: 37-2012; Design of rigid pavements using IRC: 58-2011; Distresses in concrete pavements.

Traffic Engineering: Traffic studies on flow, speed, travel time - delay and O-D study, PCU, peak hour factor parking study, accident study and analysis, statistical analysis of traffic data; Microscopic and macroscopic parameters of traffic flow, fundamental relationships; Control devices, signal design by Webster's method; Types of intersections and channelization; Highway capacity and level of service of rural highways and urban roads.

SECTION 7: GEOMATICS ENGINEERING:

Principles of surveying; Errors and their adjustment; Maps - scale, coordinate system; Distance and angle measurement - Levelling and trigonometric levelling; Traversing and triangulation survey; Total station; Horizontal and vertical curves.

Photogrammetry - scale, flying height; Remote sensing - basics, platform and sensors, visual image interpretation; Basics of Geographical information system (GIS) and Geographical Positioning system (GPS).



GATE -2014, PAPER-1

Q.No. 1-5 carry one Mark Each

| | , | | · Calla subinat |
|------|--|------------------------|-----------------------|
| 1. | A student is required to demonstrate a high | level of comprehen | ision of the subject, |
| | especially in the social sciences. | | • |
| | The word closest in meaning to comprehens | sion is | |
| | (A) understanding (B) meaning | (C) concentration | (D) stability |
| Ans | swer: (A) | | |
| 2. C | Choose the most appropriate word from the opti | ons given below to co | implete the following |
| | sentence. | | |
| | One of his biggest was his ability to | o forgive. | |
| | (A) vice (B) virtues | | (D) strength |
| | swer: (B) | | |
| 3. R | Rajan was not happy that Sajan decided to do | the project on his ov | vn. On observing his |
| | unhanniness Sajan explained to Rajan that I | ne preferred to work i | ndependently. Which |
| | one of the statements below is logically v | alid and can be infe | rred from the above |
| | sentences? | 775 | |
| | (A) Rajan has decided to work only in a g | roup. | |
| | (B) Rajan and Sajan were formed into a gr | | shes. |
| | D. J | | |
| | (C) Sajan had decided to give in to Rajan | 1 | |

Answer: (D)

4. If $y = 5x^2 + 3$, then the tangent at x = 0, y = 3

(A) passes through x = 0, y = 0

(B) has a slope of +1

(C) is parallel to the x-axis

(D) has a slope of -1

Answer: (C)

Exp: $y = 5x^2 + 3, \frac{dy}{dx} = 10x$

Slope of tan gent = $\left(\frac{dy}{dx}\right)_{x=0, y=3} = 10 \times 0 = 0$

Slope = $0 \implies \text{tangent is parallel to x-axis.}$

5. A foundry has a fixed daily cost of Rs 50,000 whenever it operates and a variable cost of Rs 800Q, where Q is the daily production in tonnes. What is the cost of production in Rs per tonne for a daily production of 100 tonnes?

Answer: 1300 to 1300

508

Exp: Fixed cost = Rs. 50,000

Variable cost = Rs. 800Q

Q = daily production in tones

For 100 tonnes of production daily, total cost of production = $50,000 + 800 \times$

100 = 130,000

So, cost of production per tonne of daily production

$$=\frac{130,000}{100}$$
 = Rs. 1300.

Q.No. 6-10 carry one Mark Each

- 6. Find the odd one in the following group: ALRVX, EPVZB, ITZDF, OYEIK
 - (A) ALRVX
- (B) EPVZB
- (C) ITZDF
- (D) OYEIK

Answer: (D)

Exp: $ALRVX \rightarrow only one vowel$

EPVZB → only one vowel

ITZDF → only one vowel

OYEIK → three vowels

7. Anuj, Bhola, Chandan, Dilip, Eswar and Faisal live on different floors in a six-storeyed building (the ground floor is numbered 1, the floor above it 2, and so on). Anuj lives on an even-numbered floor. Bhola does not live on an odd numbered floor. Chandan does not live on any of the floors below Faisal's floor. Dilip does not live on floor number 2. Eswar does not live on a floor immediately above or immediately below Bhola. Faisal lives three floors above Dilip. Which of the following floor-person combinations is correct?

| omomanons | is concer. | | | | | |
|-----------|------------|-------|---------|-------|----------|----------|
| | Anuj | Bhola | Chandan | Dilip | Eswar | Faisal |
| (4) | 6 | 2 | 5 | 1 | 3 | 4 |
| (A) | 2 | 6 | 5 | 1 | 3 | 3 4 / 5) |
| (B) | | 2 | 6 | 3 | 1 | 5 |
| (C) | | 4 | 6 | 1 | 3 | 5 . |
| ן (ע) ן | 2 | • | | | <u> </u> | 1 |

Answer: (B)

Exp:

- (a) Anuj: Even numbered floor (2,4,6)
- (b) Bhola: Even numbered floor (2,4,6)
- (c) Chandan lives on the floor above that of Faisal.
- (d) Dilip: not on 2nd floor.
- (e) Eswar: does not live immediately above or immediately below Bhola From the options its clear, that only option (B) satisfies condition (e). So, correct Ans is (B).

8. The smallest angle of a triangle is equal to two thirds of the smallest angle of a quadrilateral. The ratio between the angles of the quadrilateral is 3:4:5:6. The largest angle of the triangle is twice its smallest angle. What is the sum, in degrees, of the second largest angle of the triangle and the largest angle of the quadrilateral?

Answer: 180 to 180

Exp: Let the angles of quadrilateral are 3x, 4x, 5x, 6x

So,
$$3x + 4x + 5x + 6x = 360$$

$$x = 20$$

Smallest angle of quadrilateral = $3 \times 20 = 60^{\circ}$

Smallest angle of triangle = $\frac{2}{3} \times 60^{\circ} = 40^{\circ}$

Largest angle of triangle = $2 \times 40^{\circ} = 60^{\circ}$

Three angles of triangle are 40°, 60°, 80°

Largest angle of quadrilateral is 120°

Sum (2nd largest angle of triangle + largest angle of quadrilateral)

$$= 60^{\circ} + 120^{\circ} = 180^{\circ}$$

9. One percent of the people of country X are taller than 6 ft. Two percent of the people of country Y are taller than 6 ft. There are thrice as many people in country X as in country Y. Taking both countries together, what is the percentage of people taller than 6 ft?

Answer: (D)

Exp: Let number of people in country y = 100

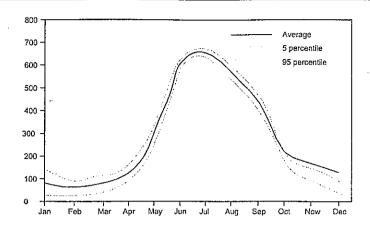
So, number of people in country x = 300

Total number of people taller than 6ft in both the countries

$$=300 \times \frac{1}{100} + 100 \times \frac{2}{100} = 5$$

% of people taller than 6ft in both the countries = $\frac{5}{400} \times 100 = 1.25$ %

10. The monthly rainfall chart based on 50 years of rainfall in Agra is shown in the following figure. Which of the following are true? (k percentile is the value such that k percent of the data fall below that value)



- (i) On average, it rains more in July than in December
- (ii) Every year, the amount of rainfall in August is more than that in January
- (iii) July rainfall can be estimated with better confidence than February rainfall
- (iv) In August, there is at least 500 mm of rainfall
- (A) (i) and (ii)
- (B) (i) and (iii)
- (C) (ii) and (iii)
- (D)(iii) and (iv)

Answer: (B)

Exp: In the question the monthly average rainfall chart for 50 years has been given.

Let us check the options.

- (i) On average, it rains more in July than in December correct.
- (ii) Every year, the amount of rainfall in August is more than that in January. may not be correct because average rainfall is given in the question.
- (iii) July rainfall can be estimated with better confidence than February rainfall.
 From chart it is clear the gap between 5 percentile and 95 percentile from average is higher in February than that in July correct.
- (iv) In August at least 500 mm rainfallMay not be correct, because its 50 year average.So correct option (B) (i) and (iii).

Q.No. 1-25 carry one Mark Each

1.
$$\lim_{x \to \infty} \left(\frac{x + \sin x}{x} \right)$$
 equals to

(A) - ∞

(B) 0

(C) 1

(D) ∞

Answer: (C)

Exp.
$$\lim_{X \to \infty} \left(\frac{x + \sin x}{x} \right) = \lim_{X \to \infty} \left(1 + \frac{\sin x}{x} \right)$$

2. Given the matrices $\mathbf{j} = \begin{bmatrix} 3 & 2 & 1 \\ 2 & 4 & 2 \\ 1 & 2 & 6 \end{bmatrix}$ and $\mathbf{K} = \begin{bmatrix} 1 \\ 2 \\ -1 \end{bmatrix}$, the product \mathbf{K}^T JK is ______

Answer: 23 to 23

Exp.
$$K^{T} JK = \begin{bmatrix} 1 & 2 & -1 \end{bmatrix} \begin{bmatrix} 3 & 2 & 1 \\ 2 & 4 & 2 \\ 1 & 2 & 6 \end{bmatrix} \begin{bmatrix} 1 \\ 2 \\ -1 \end{bmatrix}$$

$$= \begin{bmatrix} 6 & 8 & -1 \end{bmatrix} \begin{bmatrix} 1 \\ 2 \\ -1 \end{bmatrix} = 23$$

3. The probability density function of evaporation on any day during a year in a watershed is given by

$$f(E) = \begin{cases} \frac{1}{5} & 0 \le E \le 5 \text{ mm / day} \\ 0 & \text{otherwise} \end{cases}$$

The probability that E lies in between 2 and 4 mm/day in a day in the watershed is (in decimal)

Answer: 0.4 to 0.4

Exp.

4. The sum of Eigen values of the matrix, [M] is

Answer: (A)

Exp. Sum of the eigen values = Trace of the matrix =
$$215 + 150 + 550$$
 = 915

5. With reference to the conventional Cartesian (x, y) coordinate system, the vertices of a triangle have the following coordinates: $(x_1, y_1) = (1, 0)$; $(x_2, y_2) = (2, 2)$; and $(x_3, y_3) = (4, 3)$. The area of the triangle is equal to

(A)
$$\frac{3}{2}$$

(B)
$$\frac{3}{4}$$

(C)
$$\frac{4}{5}$$

(D)
$$\frac{5}{2}$$

Answer: (A)

Exp. Area of triangle $=\frac{1}{2}\begin{vmatrix} x_1 & x_2 & x_3 \\ y_1 & y_2 & y_3 \\ 1 & 1 & 1 \end{vmatrix} = \frac{1}{2}\begin{vmatrix} 1 & 2 & 4 \\ 0 & 2 & 3 \\ 1 & 1 & 1 \end{vmatrix}$ $=\frac{1}{2}|1(2-3)-2(0-3)+4(0-2)|$

$$=\frac{1}{2}|1-3|=\frac{3}{2}$$

6. Match the information given in Group - I with those in Group II.

| | Group - I | Group - II |
|-----|---|------------------------------------|
| (p) | Factor to decrease ultimate strength to design strength | (1) Upper bound on ultimate load |
| (q) | Factor to increase working load to ultimate load for design | (2) Lower bound on ultimate load |
| (r) | Statical method of ultimate load analysis | (3) Material partial safety factor |
| (s) | Kinematical mechanism method of ultimate load analysis | (4) Load factor |

(A)
$$P-1$$
; $Q-2$; $R-3$; $S-4$

(B)
$$P - 2$$
; $Q - 1$; $R - 4$; $S - 3$

(C)
$$P-3$$
; $Q-4$; $R-2$; $S-1$

(D)
$$P - 4$$
; $Q - 3$; $R - 2$; $S - 1$

Answer: (C)

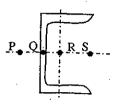
Exp. Static method Upper bound method of ultimate load analysis

Kinematic method → Lower bound on ultimate load

$$Fa = F \times \gamma_f$$

$$f_d = \frac{f}{\gamma_m}$$

The possible location of shear centre of the channel section, shown below, is



(A) P

(B) Q

(C) R

(D) S

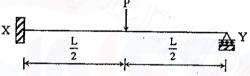
Answer: (A)

Exp. $F.e = H \times h$

$$\Rightarrow F = \frac{H \times h}{e}$$

Only possible location is P

The ultimate collapse load (P) in terms of plastic moment Mp by kinematic approach for a propped cantilever of length L with P acting at its mid-span as shown in the figure, would be



Answer: (C)

Exp.

$$D_s = 1 D_s = 4 - 3 = 1$$

So, no. of plastic hinges = $D_S + 1 - 2$ External work done = $P \cdot \Delta = P \cdot \left(\frac{L}{2} \cdot \theta\right)$

Internal work done $M_P : \theta + M_P : 2\theta = 3M_P : \theta$ By principal of virtual work

= P.
$$\frac{L}{2}$$
. θ = 3M_p. $\theta \Rightarrow$ P = $\frac{6M_P}{L}$

9. While designing, for a steel column of Fe250 grade, a base plate resting on a concrete pedestal of M20 grade, the bearing strength of concrete (in N/mm2) in limit state method of design as per IS:456-2000 is

Objective Civil Eng. \ 2016 \ 65

514

Answer: 9 to 9

=
$$0.45 f_{ck}$$

= $0.45 \times 20 = 9N / mm^3$

10. A steel section is subjected to a combination of shear and bending actions. The applied shear force is V and the shear capacity of the section is Vs. For such a section, high shear force (as per IS:800-2007) is defined as

(A)
$$V > 0.6VS$$

(B)
$$S V > 0.7V$$

(C)
$$S V > 0.8V$$

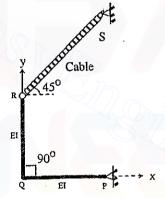
(D)
$$S V > 0.9V$$

Answer: (A)

As per clause 9.2.1 (IS: 800-2007) for combined shear and bending: Factored Exp. value of applied shear force is greater than or equal to shear strength for high

i.e., V>0.6V

The degree of static indeterminacy of a rigid jointed frame PQR supported as shown in the figure is



(A) Zero

(D) Unstable

Answer: (A)

Exp.
$$D_s = Ds_e + D_{si}$$
 $D_{se} = R - r$
 $= (r_e - 3) + 3c - r_r$ $D^{si} = 3c - rr$
 $= (4 - 3) + 3 \times 0 - 1 = 0$

$$D_{se} = R - r$$

$$6i = 3c - rr$$
 $R = 4, r =$

rr = no. of members connected to pin-1

12. In a beam of length L, four possible influence line diagrams for shear force at a section located at a distance of $\frac{L}{4}$ of from the left end support (marked as P, Q, R and S) are shown below. The correct influence line diagram is







(B) Q

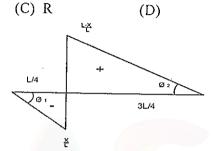


(A) P

Answer: (A)

 $\frac{X}{L} = \frac{1}{4L} = 0.25$ Exp.

$$\frac{L-X}{L} = \frac{3L}{4L} = 0.75$$



13. The degree of disturbance of the sample collected by the sampler is expressed by a term called the "area ratio". If the outer diameter and inner diameter of the sampler are Do and Di respectively, the area ratio is given by

(A)
$$\frac{D_0^2 - D_i^2}{D_i^2}$$

(B)
$$\frac{D_i^2 - D_0^2}{D_i^2}$$
 (C) $\frac{D_0^2 - D_i^2}{D_0^2}$ (D) $\frac{D_i^2 - D_0^2}{D_0^2}$

(C)
$$\frac{D_0^2 - D_i^2}{D_0^2}$$

(D)
$$\frac{D_i^2 - D_0^2}{D_0^2}$$

Answer: (A)

14. For a saturated cohesive soil, a triaxial test yields the angle of internal friction φ as zero. The conducted test is

- (A) Consolidated Drained (CD) test
- (B) Consolidated Undrained (CU) test
- (C) Unconfined Compression (UC) test
- (D) Unconsolidated Undrained (UU) test

Answer: (D)

Exp. Unconsolidated undrained test is used for completely saturated cohesive soil. In this test, no drainage is permitted during the first state as well as in the second stage.

15. The action of negative skin friction on the pile is to

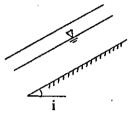
- (A) increase the ultimate load on the pile
- (B) reduce the allowable load on the pile
- (C) maintain the working load on the pile
- (D) reduce the settlement of the pile

Answer: (B)

16. A long slope is formed in a soil with shear strength parameters: c'=0 and $\phi'=34^\circ$. A firm stratum lies below the slope and it is assumed that the water table may occasionally rise to the surface, with seepage taking place parallel to the slope. Use $\gamma_m=18 k N/m^3$ and $\gamma_w=10 k N/m^3$. The maximum slope angle (in degrees) to ensure a factor of safety of 1.5, assuming a potential failure surface parallel to the slope, would be

Answer: (D)

Exp. c'= 0, $\phi' = 34^{\circ}$ $\gamma_{sat} = 18 \text{k N/m}^3$ $\gamma_{sat} = 10 \text{kN/m}^3$ F.O.S = 1.5 $\gamma'_{sub} = \gamma_{sat} - \gamma_{w}$



When water table rises to surface = $3 \cdot 18 - 10 = 8kN/m^3$

F.O.S. =
$$\frac{\gamma'_{\text{sub}}}{\gamma_{\text{sat}}} \frac{\tan \phi}{\tan i}$$
$$\Rightarrow 1.5 = \frac{8}{18} \times \frac{\tan 34^{\circ}}{\tan i}$$

$$\Rightarrow \tan i = 0.199 \Rightarrow i = 11.30^{\circ}$$
7 An incompressible homogeneous flu

17. An incompressible homogeneous fluid is flowing steadily in a variable diameter pipe having the large and small diameters as 15 cm and 5 cm, respectively. If the velocity at a section at the 15 cm diameter portion of the pipe is 2.5 m/s, the velocity of the fluid (in m/s) at a section falling in 5 cm portion of the pipe is _____

Answer: 22 to 23

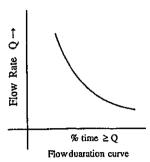
Exp. By continuity equation between two sections

$$A_1 V_1 = A_2 V_2$$

 $\Rightarrow \frac{\pi}{4} \times (d_1)^2 \cdot V_1 = \frac{\pi}{4} (d_2)^2 \cdot V_2$
 $\Rightarrow (15)^2 \times 2.5 = (5)^2 \times V_2$
 $\Rightarrow V_2 = \frac{225 \times 2.5}{25} = 22.5 \text{ m/s}$

- 18. A conventional flow duration curve is a plot between
 - (A) Flow and percentage time flow is exceeded
 - (B) Duration of flooding and ground level elevation
 - (C) Duration of water supply in a city and proportion of area receiving supply exceeding this duration

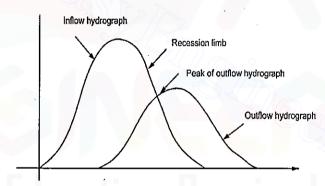
(D) Flow rate and duration of time taken to empty a reservoir at that flow rate Answer: (A)



- 19. In reservoirs with an uncontrolled spillway, the peak of the plotted outflow hydrograph
 - (A) lies outside the plotted inflow hydrograph
 - (B) lies on the recession limb of the plotted inflow hydrograph
 - (C) lies on the peak of the inflow hydrograph
 - (D) is higher than the peak of the plotted inflow hydrograph

Answer: (B)

Exp.



- 20. The dimension for kinematic viscosity is
 - (A) $\frac{L}{MT}$
- (B) $\frac{L}{T^2}$
- (C) $\frac{L^2}{T}$
- (D) $\frac{ML}{T}$

Answer: (C)

Exp. Kinematic viscosity (V) \rightarrow cm²/s : $\frac{L^2}{T}$

- 21. Some of the nontoxic metals normally found in natural water are
 - (A) arsenic, lead and mercury
- (B) calcium, sodium and silver
- (C) cadmium, chromium and copper
- (D) iron, manganese and magnesium

Answer: (D)

518

22. The amount of CO2 generated (in kg) while completely oxidizing one kg of CH4 to the end products is _____

Answer: 2.7 to 2.8

Exp.
$$CH_4 + 20_2 \rightarrow CO_2 + 2H_2O_3$$

16 g 44g

When completely oxidized, 16g CH4 liberates = 44g CO₂

So, 1 kg.CH₄ will liberate =
$$\frac{44}{16} \times 1 = 2.75$$
kg

23. The minimum value of 15 minute peak hour factor on a section of a road is

(D) 0.33

Answer: (C)

Exp. 15 minute peak hour factor is used to design traffic intersections

$$PHF = \frac{\left(\frac{V}{4}\right)}{V_{15}}$$

Where V= peak hour volume (veh/hr)

 V_{15} = Maximum 15 minute volume within the peak hour.

Minimum value is 0.25

- 24. The following statements are related to temperature stresses developed in concrete pavement slabs with free edges (without any restraint):
 - (P) The temperature stresses will be zero during both day and night times if the pavement slab is considered weightless
 - (Q) The temperature stresses will be compressive at the bottom of the slab during night time if the self-weight of the pavement slab is considered
 - (R) The temperature stresses will be compressive at the bottom of the slab during day time if the self-weight of the pavement slab is considered

The TRUE statement(s) is(are)

(C) P and Q only (D) P and R only

Answer: (C)

Exp. Temperature stress = 0 if pavement is weightless

Temperature stress = Tensile at bottom of slab in day time

- = Compressive at bottom of slab in night time
- 25. The Reduced Levels (RLs) of the points P and Q are +49.600 m and + 51.870 m respectively. Distance PQ is 20 m. The distance (in m from P) at which the +51.000 m contour cuts the line PQ is

(D) 2.27

Answer: (B)

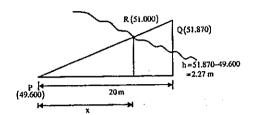
Gate - 2014, Paper-1

519

Exp.

$$\frac{h}{20} = \frac{51 - 49.6}{x}$$

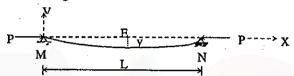
$$\Rightarrow x = \frac{1.4 \times 20}{2.27} = 12.33 \text{ m}$$



Q.No. 26-55 carry Two Marks

26. If the following equation establishes equilibrium in slightly bent position, the midspan deflection of a member shown in the figure is

$$\frac{D^2 y}{dx^2} + \frac{P}{EI} y = 0$$



If α is amplitude constant for y, then

(A)
$$y = \frac{1}{P} \left(1 - \alpha \cos \frac{2\pi x}{L} \right)$$

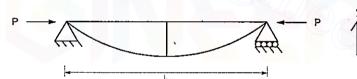
(B)
$$y = \frac{1}{P} \left(1 - \alpha \sin \frac{2\pi x}{L} \right)$$

(C)
$$y = \alpha \sin \frac{n\pi x}{I}$$

(D)
$$y = \alpha \cos \frac{n\pi x}{L}$$

Answer: (C)

Exp.



$$\frac{d^2y}{dex^2} = \frac{-P}{EI} \cdot y = -k^2 \cdot y$$

Solution of this differential equation is

$$y = a \sin kx + b \cos kx$$

at
$$x = 0$$
, $y = 0 \implies b = 0$

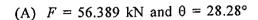
at
$$x = L$$
, $y = 0 \implies 0 = a \sin KL$

$$\Rightarrow$$
 KL = $n\pi \Rightarrow$ K = $\frac{n\pi}{T}$

$$\therefore y = a \sin \frac{n\pi}{L} x$$

520

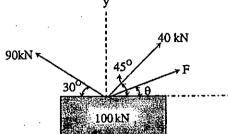
27. A box of weight 100 kN shown in the figure is to be lifted without swinging. If all forces are coplanar, the magnitude and direction (θ) of the force (F) with respect to x-axis should be



(B)
$$F = -56.389$$
 kN and $\theta = -28.28^{\circ}$

(C)
$$F = 9.055 \text{ kN and } \theta = 1.414^{\circ}$$

(D)
$$F = -9.055 \text{ kN} \text{ and } \theta = -1.414^{\circ}$$



Answer: (A)

Exp. For no swinging, $\Sigma F_H = 0$ $\Rightarrow 90 \cos 30^{\circ} F \cos \theta + 40 \cos 45^{\circ}$

$$\Rightarrow F \cos \theta = 49.658 \dots (i)$$

$$\Rightarrow$$
 F cos $\theta = 49.658$ (1)

Also,
$$\Sigma F_v = 0$$

$$\Rightarrow$$
 100 = 90sin30° 40sin 45° Fsin θ

$$\Rightarrow$$
 F sin $\theta = 26.715$ (ii)

solving(i) and (ii)

$$F = 56.389 \text{ kN}, \theta = 28.28^{\circ}$$

28. A particle moves along a curve whose parametric equations are: $x = t^3 + 2t$, $y = -3e^{-2t}$ and $z = 2\sin(5t)$, where x, y and z show variations of the distance covered by the particle (in cm) with time t (in s). The magnitude of the acceleration of the particle (in cm/s²) at t = 0 is ______

Answer: 12 to 12

Exp.
$$x = t^3 + 2t$$
, $y = -3e^{-2t}$, $z = 2 \sin (5t)$

$$a_{x} = \frac{\partial^{2} x}{\partial t^{2}} = 6t$$

$$a_{y} = \frac{\partial^{2} y}{\partial t^{2}} = -12.e^{-2t}$$

$$a_{z} = \frac{\partial^{2} z}{\partial t^{2}} = -50\sin 5t$$

$$a = a_x \hat{i} + a_y \hat{j} + a_z \hat{k} = (6t) \hat{i} - (12e^{-2t}) \hat{j} - (50\sin 5t) \hat{k}$$

$$a (t = 0) = -12 \hat{j}$$

so,
$$|a|_{t=0} = 12 \text{ cm/s}^2$$

29. A traffic office imposes on an average 5 number of penalties daily on traffic violators. Assume that the number of penalties on different days is independent and follows a Poisson distribution. The probability that there will be less than 4 penalties in a day is

Answer: 0.26 to 0.27

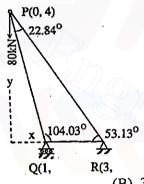
Exp.
$$P(x) = \frac{e^{-\lambda} \lambda^{x}}{x!}, \lambda = 5$$

$$P(x < 4) = P(x = 0) + P(x = 1) + P(x = 2) + P(x = 3)$$

$$= \frac{e^{-5} 5^{0}}{|0|} + \frac{e^{-5} 5^{1}}{|1|} + \frac{e^{-5} 5^{2}}{|2|} + \frac{e^{-5} 5^{3}}{|3|}$$

$$=e^{-5}\left[1+5+\frac{25}{2}+\frac{125}{6}\right]=0.265$$

30. Mathematical idealization of a crane has three bars with their vertices arranged as shown in the figure with a load of 80 kN hanging vertically. The coordinates of the vertices are given in parentheses. The force in the member QR, FQR will be

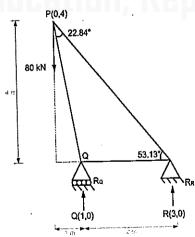


- (A) 30 kN Compressive
- (C) 50 kN Compressive

- (B) 30 kN Tensile
- (D) 50 kN Tensile

Answer: A

Exp.



Objective Civil Eng. \ 2016 \ 66

ደብኒላ

522

$$\Sigma F_v = 0 \implies R_Q + R_R = 80$$
(i)

Taking moment about Q=0

$$\Rightarrow$$
 80 × 1 + R_R × 2 = 0

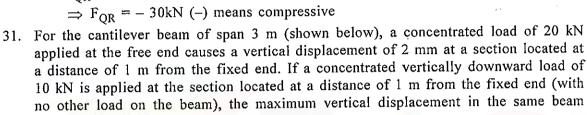
$$\Rightarrow$$
 RR = -40 kN \downarrow

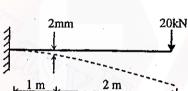
From (i), we get $R_O = 120 \text{kN}$

Consider section x-x

Taking moment about P = 0, LHS in equilibrium

$$F_{OR} \times 4 + 120 \times 1 = 0$$

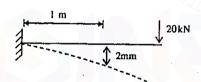




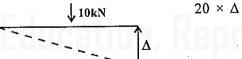
Answer: 1 to 1

(in mm) is

Exp.

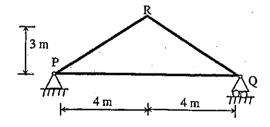


By Bettis theorem,



 $20 \times \Delta = 10 \times 2 \Delta = 1$ mm.

32. For the truss shown below, the member PQ is short by 3 mm. The magnitude of vertical displacement of joint R (in mm) is _____

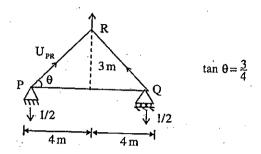


Answer: 1.0 to 2.5

Exp. Since deflection at R is required. So, let us apply a virtual unit load at point R in upwards direction.

At point P
$$\Sigma F_{v} = 0 \Rightarrow U_{PR} \sin \theta = \frac{1}{2}$$

 $\Sigma FH = 0 \Rightarrow U_{PR} \cos \theta + U_{PQ} = 0$
 $\Rightarrow U_{PQ} = -U_{PR} \cos \theta$
 $= \frac{1}{2} \times \frac{1}{\tan \theta} = \frac{1}{2} \times \frac{4}{3} = \frac{-2}{3}$



$$\delta_{\rm R} = U_{\rm PQ} \times \lambda_{\rm PQ}$$

$$= \frac{-2}{3} \times (-3) \ (\because PQ \text{ is 3 mm short}] = 2mm$$

So, deflection at R = 2mm (upwards)

33. A rectangular beam of width (b) 230 mm and effective depth (d) 450mm is reinforced with four bars of 12 mm diameter. The grade of concrete is M20 and grade of steel is Fe500. Given that for M20 grade of concrete the ultimate shear strength, $2 \approx 0.36$ N/mm $\tau =$ for steel percentage, p = 0.25, and $2 \approx 0.48$ N/mm $\tau =$ for p = 0.50. For a factored shear force of 45kN, the diameter (in mm) of Fe500 steel two legged stirrups to be used at spacing of 375 mm, should be

Answer: (A)

Exp.
$$\tau_{\rm v} = \frac{V_{\rm U}}{\rm bd} = \frac{45 \times 1000}{230 \times 450} = 0.434 \,\text{N/mm}^2$$

% tensile reinforcement (p) = $\frac{4 \times \frac{\pi}{4} \times (12)^2}{230 \times 450} \times 100 = 0.437 \%$

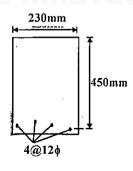
$$\tau_c = 0.36 + \frac{0.12}{0.25} \times (0.437 - 0.25) = 0.45 \text{ N/mm}^2$$

$$\tau_{\rm v} < \tau_{\rm c}$$

So, minimum shear reinforcement is required Minimum shear reinforcement

So, adopt 8mm

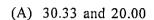
$$\frac{A_{sv}}{b \times S_v} = \frac{0.4}{0.87 f_y}$$



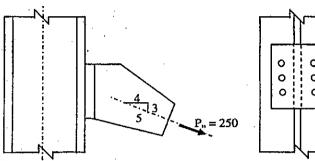
$$\Rightarrow A_{sv} = \frac{0.4 \times b \times S_v}{0.87 \times f_v}$$

$$\Rightarrow 2 \times \frac{\pi}{4} \times \phi^2 = \frac{0.4 \times 230 \times 375}{0.87 \times 500} \Rightarrow \phi = 7.10 \text{ mm}$$

34. The tension and shear force (both in kN) in each bolt of the joint, as shown below, respectively are



- (C) 33.33 and 20.00
- (D) 33.33 and 25.00



Answer: (D)

Exp.
$$\sin \theta = \frac{3}{5}, \cos \theta = \frac{4}{5}$$

$$P_{\rm U}\cos\theta = \frac{4}{5}.P_{\rm U}$$

$$P_{\rm U} \sin \theta = \frac{3}{5} \cdot P_{\rm U}$$

$$\begin{array}{c}
4 \\
0 \\
5
\end{array}$$

$$\begin{array}{c}
P_0 = 250 \text{kN}
\end{array}$$

Tension in each bolt =
$$\frac{P_U \cos \theta}{6} = \frac{4P_U}{5 \times 6} = \frac{4}{30} \times 250 = 33.33 \text{ kN}$$

Shear in each bolt =
$$\frac{P_U \sin \theta}{6} = \frac{3}{5} \times \frac{P_U}{6} = \frac{3}{5 \times 6} \times 250 = 25 \text{ kN}$$

35. For a beam of cross-section, width = 230 mm and effective depth = 500 mm, the number of rebars of 12 mm diameter required to satisfy minimum tension reinforcement requirement specified by IS:456-2000 (assuming grade of steel reinforcement as Fe500) is

Answer: 2 to 2

Exp.
$$\frac{\left(A_{st}\right)_{min}}{bd} = \frac{0.85}{f_y}$$

$$\Rightarrow$$
 $(A_{st})_{min} = \frac{0.85}{500} \times 230 \times 500 = 195.5 \text{ mm}^2$

$$n \times \frac{\pi}{4} d^2 = 195.5 \Longrightarrow n = 1.73$$

So, take n = 2

36. In a reinforced concrete section, the stress at the extreme fibre in compression is 5.80 MPa. The depth of neutral axis in the section is 58 mm and the grade of concrete is M25. Assuming linear elastic behavior of the concrete, the effective curvature of the section (in per mm) is

(A)
$$2.0 \times 10^{-6}$$

(B)
$$3.0 \times 10^{-6}$$

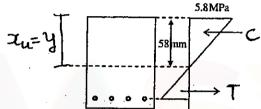
(C)
$$4.0 \times 10^{-6}$$

(D)
$$5.0 \times 10^{-6}$$

Answer: (C)

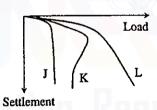
Exp. E = $5000 \sqrt{f_{ck}} = 5000 \times \sqrt{25} = 25000 \text{ N/mm}^2$

$$\frac{M}{I} = \frac{f}{y} = \frac{E}{R}$$



curvature,
$$\frac{1}{r} = \frac{f}{yE} = \frac{5.8}{58 \times 25000} = 4 \times 10^{-6} \text{ per mm}$$

37. Group I contains representative load-settlement curves for different modes of bearing capacity failures of sandy soil. Group II enlists the various failure characteristics. Match the load-settlement curves with the corresponding failure characteristics.



| Group I | Group II |
|-------------|---|
| (p) Curve J | (1) No apparent heaving of soil around the footing |
| (q) Curve K | (2) Rankine's passive zone develops imperfectly |
| (r) Curve L | (3) Well defined slip surface extends to ground surface |

(A)
$$P - 1$$
, $Q - 3$, $R - 2$

(C)
$$P - 3$$
, $Q - 1$, $R - 2$

(D)
$$P - 1$$
, $Q - 2$, $R - 3$

Answer: (A)

Exp. $K \rightarrow$ General shear failure

L → Local shear failure

J → Punching shear failure

Objective Civil Engineering

526

General shear failure (Q):

Well define failure pattern(3)

Local shear failure (L):

Rankine passive zone developes(2)

Punching shear failure:

No heaving of soil around footing

38. A given cohesionless soil has $e_{max} = 0.85$ and $e_{max} = 0.50$. In the field, the soil is compacted to a mass density of 1800 kg/m3 at a water content of 8%. Take the mass density of water as 1000kg/m^3 and G, as 2.7. The relative density (in %) of the soil is

(B) 60.25

(C) 62.87

(D) 65.7

 $\rho_{d} = \frac{G.\rho_{w}}{1+e}, \rho_{d} = \frac{\rho_{b}}{1+\omega}$ $\therefore \rho_{b} = \frac{G(1+\omega)\rho_{w}}{1+e}$

Answer: (D)

Exp.

$$e_{max} = 0.85$$
, $e_{min} = 0.5 \rho_{field} = 1800 \text{ kg/m}^3$
 $w = 8 \%$, $\rho_{w} = 1000 \text{ kg/m}^3$, $G_s = 27$

$$\rho_{b} = \frac{G(1+w)}{1+e} \rho_{w}$$

$$\Rightarrow$$
 1 + e = G (1 + w) $\frac{\rho_{\text{w}}}{\rho_{\text{b}}} = \frac{2.7(1+.08)}{1800} \times 1000$

$$\Rightarrow$$
 e = 0.62

Relative Density =
$$\frac{e_{max} - e}{e_{maqx} - e_{min}} \times 100 = \frac{0.85 - 0.62}{0.85 - 0.5} \times 100 = 65.1 \%$$

39. The following data are given for the laboratory sample.

$$\sigma_0' = 175 \text{kPa}$$
; $e_0 = 1.1$; $\sigma_0' + \Delta \sigma_0 = 300 \text{kPa}$; $e = 0.9$

If thickness of the clay specimen is 25mm, the value of coefficient of volume compressibility is $___ \times 10^{-4} \text{ m}^2/\text{kN}$

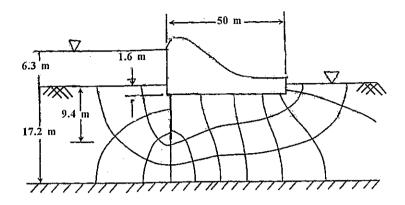
Answer: 7.6 to 8.0

Exp. Volume compressibility, $m_v = \frac{a_v}{1 + e_0}$, $a_v = \frac{\Delta_e}{\Delta \sigma'}$

$$\therefore m_{v} = \frac{\Delta_{e}}{(1 + e_{0})\Delta\sigma'}$$

$$m_v = \frac{0.2}{1 + 1.1} \times \frac{1}{125} = 7.62 \times 10^{-4} \text{ m}^2 / \text{kN}$$

40. The flow net constructed for the dam is shown in the figure below. Taking the coefficient of permeability as 3.8 × 10⁻⁶ m/s, the quantity of flow (in cm³/s) under the dam per meter of dam is



Answer: 7.10 to 7.85

Exp.
$$Q = K.H. \frac{N_f}{N_d}$$

 $N_f = No.$ of flow channels = 3

 $N_d = No.$ of equipotential drops = 10

$$K = 3.8 \times 10^{-4} \text{ s/m}$$

$$H = 6.3 m$$

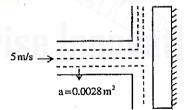
$$Q = 3.8 \times 10^{-6} \times 6.3 \times \frac{3}{10} = 7.182 \times 10^{-6} \text{ m}^3/\text{s/m} \ 7.182\text{cm}^3 /\text{s/m}$$

41. A horizontal set of water with its cross-sectional area of 0.0028 m2 hits a fixed vertical plate with a velocity of 5 m/s. After impact the jet splits symmetrically in a plane parallel to the plane of the plate. The force of impact (in N) of the jet on the plate is

Answer: (C)

Exp. Force on plate, $F = \rho.a.v^2$

$$1000 \times 0 \times 0028 \times 5^2 = 70N$$



42. A venturimeter, having a diameter of 7.5 cm at the throat and 15 cm at the enlarged end, is installed in a horizontal pipeline of 15 cm diameter. The pipe carries an incompressible fluid at a steady rate of 30 litres per second. The difference of pressure head measured in terms of the moving fluid in between the enlarged and the throat of the venturimeter is observed to be 2.45 m. Taking the acceleration due to gravity as 9.81 m/s², the coefficient of discharge of the venturimeter (correct up to two places of decimal) is

Answer: 0.93 to 0.96

→ Q=30ℓ/s

D,=15cm

$$Q = \frac{\text{cd.a}_1 \text{ a}_2}{\sqrt{\text{a}_1^2 - \text{a}_2^2}} \cdot 2\text{gh}$$

$$a_1 = \frac{\pi}{4} \times (.15)^2 = 0.0176 \,\mathrm{m}^2$$

$$a_2 = \frac{\pi}{4} \times (.75)^2 = 0.044 \text{ m}^2$$

$$\Rightarrow 30 \times 10^{-3} = C_d \cdot \frac{0.0176 \times 0.0044}{\sqrt{(0.0176)^2 - (0.0044)^2}} \times \sqrt{2 \times 9.81 \times 2.45}$$

$$\Rightarrow$$
 C_d = 0.95

43. A rectangular channel having a bed slope of 0.0001, width 3.0 m and Manning's coefficient 'n' 0.015, carries a discharge of 1.0 m³/s. Given that the normal depth of flow ranges between 0.76 m and 0.8 m. The minimum width of a throat (in m) that is possible at a given section, while ensuring that the prevailing normal depth is not exceeded along the reach upstream of the contraction, is approximately equal to (assume negligible losses)

Answer: (B)

Exp. 3

3 n = 0.015, Q =
$$1 \text{m}^3$$
 /s, B = 3.0m

Normal depth of flow ranges between 0.76m to 0.8 m

If prevailing normal depth of flow is not exceeded, there must not be choking of the section or it should be at boundary condition of choking.

So, width of section should be such that there should be critical flow corresponding to the prevailing specific energy.

i.e.
$$\frac{3}{2} \left(\frac{q^2}{g}\right)^{1/3} = E_c = E_{initial}$$

$$q = \frac{Q}{B_{min}}$$
, So $\frac{3}{2} \left[\frac{\left(\frac{Q}{B_{min}}\right)^2}{g} \right]^{\frac{1}{3}} = E_{initial}$

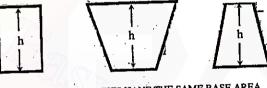
$$E_{initial} = y + \frac{q^2}{2gy^2}$$

$$\Rightarrow 1 = \frac{1}{0.015} \cdot (3y) \cdot \left(\frac{3y}{3+2y}\right) \cdot (0.0001)^{1/2} \Rightarrow y = 0.78 \text{ m}$$

So, E_{initial} = 0.78 +
$$\frac{\left(\frac{1}{3}\right)^2}{2 \times 9.81 \times (0.78)^2}$$
 = 0.789 m

So,
$$\frac{3}{2} \left(\frac{Q}{g \cdot (B_{min})^2} \right)^{\frac{1}{3}} = 0.789 \Rightarrow B_{min} = 0.84 \text{ m}$$

44. Three rigid buckets, shown as in the figures (1), (2) and (3), are of identical heights and base areas. Further, assume that each of these buckets have negligible mass and are full of water. The weights of water in these buckets are denoted as W₁, W₂, and W3 respectively. Also, let the force of water on the base of the bucket be denoted as F₁, F₂, and F₃ respectively. The option giving an accurate description of the system physics is



ALL THREE BUCKETS HAVE THE SAME BASE AREA
(1) (2) (3)

(A)
$$W_2 = W_1 = W_3$$
 and $F_2 > F_1 > F_3$

(B)
$$W_2 > W_1 > W_3$$
 and $F_2 > F_1 > F_3$

(C)
$$W_2 = W_1 = W_3$$
 and $F_1 = F_2 = F_3$

(D)
$$W_2 > W_1 > W_3$$
 and $F_1 = F_2 = F_3$

Answer: (D)

Exp.

Force on base of Bucket, w base $F = \gamma_w h.A_{base}$

... Base area of all buckets is same.

So,
$$F_1 = F_2 = F_3$$

Weight of water, $W = \gamma_w$. V
Since $V_2 > V_1 > V_3$
so, $W_2 > W_1 > W_3$







45. An incompressible fluid is flowing at a steady rate in a horizontal pipe. From a section, the pipe divides into two horizontal parallel pipes of diameters d_1 and d_2 (where $d_1 = 4d_2$) that run for a distance of L each and then again join back to a pipe of the original size. For both the parallel pipes, assume the head loss due to friction only and the Darcy-Weisbach friction factor to be the same. The velocity ratio between the bigger and the smaller branched pipes is _____

Answer: 2 to 2

Darcy's formula,
$$h_f = \frac{fLV^2}{2gD}$$
 $f = Darcy's friction factor$

Objective Civil Eng. \ 2016 \ 67

530

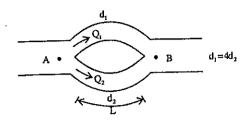
Exp. Since pipes are in parallel,

so, head loss will be same

$$\frac{\text{fL}}{d_2} \cdot \frac{V_1^2}{2g} = \frac{\text{fL}}{d_2} \cdot \frac{V_2^2}{2g}$$

$$\Rightarrow \frac{V_1^2}{d_1} = \frac{V_2^2}{d_2} \Rightarrow \frac{V_1^2}{V_2^2} = \frac{d_1}{d_2} = 4$$

$$\Rightarrow \frac{V_1}{V_2} = 2$$



46. 16 MLD of water is flowing through a 2.5 km long pipe of diameter 45 cm. The chlorine at the rate of 32 kg/d is applied at the entry of this pipe so that disinfected water is obtained at the exit. There is a proposal to increase the flow through this pipe to 22 MLD from 16 MLD. Assume the dilution coefficient, n = 1. The minimum amount of chlorine (in kg per day) to be applied to achieve the same degree of disinfection for the enhanced flow is

(D) 23.27

Answer: (A)

Exp. For disinfection, we have n to =K

Where t = time required to kill all organisms

c = concentration of disinfectant

n = dilution coefficient

k = constant

So,
$$t_1 c_1^n = t_2 c_2^n$$

here,
$$n = 1$$

$$t_1c_1 = t_2 c_2$$

$$t_1 = \frac{L}{V_1}; L = Length of pipe$$

 $V_1 = V_1; V_1 = V_1$

$$=\frac{L}{Q_{l/A}} = \frac{L.A}{Q_{l}}$$
; $Q_{l} = \text{disch arg e per day}$

$$C_1 = \frac{W_1}{Q_1}$$
, $W_1 = \text{weight of disinfectant per day}$

so,
$$\frac{LA}{Q_1} \frac{W_1}{Q_1} = \frac{LA}{Q_2} \cdot \frac{W_2}{Q_2}$$

$$W_2 = \left(\frac{Q_2}{Q_1}\right)^2$$
. $W_1 = \left(\frac{22}{16}\right)^2 \times 32 = 60.5 \text{ kg/d}$

- 47. The potable water is prepared from turbid surface water by adopting the following treatment sequence.
 - (A) Turbid surface water → Coagulation → Flocculation → Sedimentation → Filtration → Disinfection → Storage & Supply
 - (B) Turbid surface water → Disinfection → Flocculation → Sedimentation → Filtration → Coagulation → Storage & Supply
 - (C) Turbid surface water → Filtration → Sedimentation → Disinfection → Flocculation → Coagulation → Storage & Supply
 - (D) Turbid surface water → Sedimentation → Flocculation → Coagulation → Disinfection → Filtration . Storage & Supply

Answer: (A)

48. For a sample of water with the ionic composition shown in the figure below, the carbonate and non-carbonate hardness concentrations (in mg/l as CaCO³), respectively are:

| meq/I 0 | | 4 5 7 |
|---------|------------------|----------------------------------|
| | Ca ²⁺ | Mg ²⁺ N ^{a+} |
| | HCO ₃ | SO ₄ ⁻² |
| meq/1 0 | | 3.5 |

(A) 200 and 50

(B) 175 and 75

(C) 75 and 175

(D) 50 and 200

Answer: (B)

Exp. Carbonate hardness $CH = 3.5 \times 50 \text{ mg/l}$ as $CaCO_3 = 175 \text{mg/l}$ Non Carbonate hardness, $NCH = Total \ Hardness - Carbonate hardness$ $= 5.0 \times 50 - 175$ = 75 mg/l as $CaCO_3$

49. A straight 100 m long raw water gravity main is to carry water from an intake structure to the jack well of a water treatment plant. The required flow through this water main is 0.21 m³/s. Allowable velocity through the main is 0.75 m/s. Assume f = 0.01, g = 9.81 m/s². The minimum gradient (in cm/100 m length) to be given to this gravity main so that the required amount of water flows without any difficulty is ___

Answer: 4.7 to 4.9

Exp.
$$Q = 0.21 \text{ m}^3/\text{s}$$

 $V_a = 0.75 \text{ m/s}, f = 0.01, g = 9.81 \text{ m/s}^2$
 $A = \frac{Q}{V} \Rightarrow \frac{\pi}{4} \cdot d^2 = \frac{0.21}{0.75}$
 $\Rightarrow d = 0.60 \text{ m}$
 $h_f = \frac{f \times L}{d} \times \frac{V^2}{2g} = \frac{0.01 \times 100 \times (0.75)^2}{0.60 \times 2 \times 9.81} = 0.047 \text{ m} = 4.7 \text{ cm}$

Minimum gradient =
$$\frac{h_f}{L} = \frac{4.7 \text{ cm}}{100 \text{ m}}$$

50. A traffic survey conducted on a road yields an average daily traffic count of 5000 vehicles. The axle load distribution on the same road is given in the following table:

| Axle load (tonnes) | Frequency of traffic (%) | | |
|--------------------|--------------------------|--|--|
| 18 | 10 | | |
| 14 | 20 | | |
| 10 | 35 | | |
| Q | 15 | | |
| 6 | 20 | | |

The design period of the road is 15 years, the yearly traffic growth rate is 7.5% and the load safety factor (LSF) is 1.3. If the vehicle damage factor (VDF) is calculated from the above data, the design traffic (in million standard axle load, MSA) is

Answer: 307 to 310

Exp. Vehicle damage factor

$$VDF = \frac{V_1 \times \left(\frac{W_1}{W_s}\right)^4 + V_2 \cdot \left(\frac{W_2}{W_s}\right)^4 + \dots \cdot V_s \left(\frac{W_s}{W_s}\right)^4}{V_1 + V_2 + V_3 + V_4 + V_5}$$

$$= \frac{10 \times \left(\frac{18}{8.2}\right)^4 + 20 \times \left(\frac{14}{8.2}\right) + 35 \times \left(\frac{10}{8.2}\right)^4 + 15 \times \left(\frac{8}{8.2}\right)^4 + 20 \times \left(\frac{16}{8.2}\right)^4}{10 + 20 + 35 + 15 + 20} = 4.99$$

$$N = \frac{365 \times A \left[(1+r)^{n} - 1 \right]}{r} \times VDF$$

$$= \frac{365 \times 5000 \times \left[(1.075)^{n} - 1 \right]}{0.075} \times 4.99 = 237.785 \text{ MSA}$$

So, Design traffic = $237.785 \cdot 1.3 = 309.21$ MSA

51. The perception-reaction time for a vehicle travelling at 90 km/h, given the coefficient of longitudinal friction of 0.35 and the stopping sight distance of 170 m (assume $g = 9.81 \text{ m/s}^2$), is _____ seconds.

Answer: 3.1 to 3.2

Exp.
$$V = 90 \text{ km/h}$$
 $\therefore v = \frac{90 \text{ m} \times 1000}{3600} = 25 \text{ m/s}$

$$SSD = v.t + \frac{v^2}{2gf}$$

$$170 = 25t + \frac{25^2}{2 \times 9.81 \times 0.35}$$

t = 3.16 see

52. The speed-density (u-k) relationship on a single lane road with unidirectional flow is u = 70 - 0.7k, where u is in km/hr and k is in veh/km. The capacity of the road (in veh/hr) is _____

Answer: 1750 to 1750

Exp.
$$U = 70 - 0.7 \text{ K}$$

capacity =
$$\frac{U_1 \times K_j}{4} \frac{U_f}{K_j} = \text{free velocity}$$

 $K_j = \text{jam density}$

At
$$K_j$$
, $U = 0$
So $k_j = \frac{70}{7} = 100 \text{ veh/km}$
At $K = 0$, $U = U_f = 70 \text{ km/hr}$

So, capacity =
$$\frac{70 \times 100}{4}$$
 = 1750 veh/hr

53. An isolated three-phase traffic signal is designed by Webster's method. The critical flow ratios for three phases are 0.20, 0.30, and 0.25 respectively, and lost time per phase is 4 seconds. The optimum cycle length (in seconds) is ______

Answer: 90 to 95

Exp. Total time lost in a cycle, $L = 4 \times 3 = 12sec$

$$C = \frac{1.5L + 5}{1 - y} = \frac{1.5 \times 12 + 5}{1 - (0.2 + 0.3 + 0.25)} = 92s$$

54. A levelling is carried out to establish the Reduced Levels (RL) of point R with respect to the Bench Mark (BM) at P. The staff readings taken are given below.

| the petter w | Iurk (2312) | 7 | | | |
|--------------|-------------|----|----------|-----------|--|
| Staff | BS | IS | FS | RL | |
| Station | | | | 100 000 m | |
| P P | 1.655 m | | • | 100.000 m | |
| 0 | -0.950 m | | -1.500 m | | |
| | | | 0.750 m | ? | |
| K | <u> </u> | | | <u> </u> | |

If RL of P is +100.000 m, them RL (in m) of R is

(A) 103.355

(B) 103.155

(C) 101..455

(D) 100.355

Answer: (C)

Exp.

RL of P = 100.0 m

HI at P = 100 + 1.655 = 101.655 m

RL of Q = 101.655 - (-1.5) = 103.155 m

HI at Q = 103.155 + (-0.95) = 102.205 m

RL of R = 102.205 - 0.75 = 101.455 m

55. Group I lists tool/instrument while Group II lists the method of surveying. Match the tool/instrument with the corresponding method of surveying.

| Group I | Group II |
|-----------------|---------------------------|
| (p) Alidade | (1) Chain surveying |
| (q) Arrow | (2) Levelling |
| (r) Bubble tube | (3) Plain table surveying |
| (s) Stadia hair | (4) Theodolite surveying |

(A)
$$P-3$$
; $Q-2$; $R-1$; $S-4$

(B)
$$P - 2$$
; $Q - 4$; $R - 3$; $S - 1$

(C)
$$P-1$$
; $Q-2$; $R-4$; $S-3$

(D)
$$P - 3$$
; $Q - 1$; $R - 2$; $S - 4$

Answer: (D)

Exp. Alidade

→ used in plane table surveying

Arrow

→ chain surveying

Bubble tube

→ leveling

Stadia hair

→ Theodolite surveying



GATE -2014, PAPER-2

| O. No. 1-5 carry one Mark E | acl | \mathbf{E} | rk | Mai | one | carry | 1-5 | No. | O. |
|-----------------------------|-----|--------------|----|-----|-----|-------|-----|-----|----|
|-----------------------------|-----|--------------|----|-----|-----|-------|-----|-----|----|

| | O. No. 1-3 Carry one Mark Each |
|-----|---|
| 1. | Choose the most appropriate word from the options given below to complete the following sentence. |
| | A person suffering from Alzheimer's disease short-term memory*loss. |
| | (A) experienced (B) unexperienced (C) is experiencing (D) experiences |
| Ans | swer: (D) |
| 2. | Choose the most appropriate word from the options given below to complete the following sentence is the key to their happiness; they are satisfied with what they have. |
| | (A) Contentment (B) Ambition (C) Perseverance (D) Hunger |
| Ans | swer: (A) |
| 3. | Which of the following options is the closest in meaning to the sentence below? |
| | "As a woman, I have no country." |
| | (A) Women have no country |
| | (B) Women are not citizens of any country. |
| | (C) Women's solidarity knows no national boundaries |
| | (D) Women of all countries have equal legal rights. |
| An | swer: (C) |
| 4. | In any given year, the probability of an earthquake greater than Magnitude 6 occurring |
| | in the Garhwal Himalayas is 0.04. The average time between successive occurrences |
| | of such earthquakes is years. |
| | swer: 25 to 25 |
| Exp | _ |
| | $P = 0.04 = \frac{4}{100}$ |
| | For 1 earth quake Reverse probability |
| | $\frac{100}{4}$ P = 1 earth quake |
| | 25 years |

5. The population of a new city is 5 million and is growing at 20% annually. How many years would it take to double at this growth rate?

Answer: (A)

Exp:

$$\frac{20}{140} \times 8$$

After 1 year

$$P = 6$$

$$2 \text{ years} = 7.2$$

After
$$3 = \frac{20}{100} \times 1.2$$

= 8.65

After 4 years =
$$\frac{20}{100} \times 8.65$$

Time will be in between 3-4 years.

Q. No. 6-10 carry one Mark Each

6. In a group of four children, Som is younger to Riaz. Shiv is elder to Ansu. Ansu is youngest in the group. Which of the following statements is/are required to find the eldest child in the group?

Statements:

- 1. Shiv is younger to Riaz.
- 2. Shiv is elder to Som.
- (A) Statement 1by itself determines the eldest child.
- (B) Statement 2 by itself determines the eldest child.
- (C) Statement 1 and 2 are both required to determine the eldest child.
- (D) Statement 1 and 2 are not sufficient to determine the eldest child.

Answer: (A)

7. Moving into a world of big data will require us to change our thinking about the merits of exactitude. To apply the conventional mindset of measurement to the digital, connected world of the twenty-first century is to miss a crucial point. As mentioned earlier, the obsession with exactness is an artefact of the information-deprived analog era. When data was sparse, every data point was critical, and thus great care was taken to avoid letting any point bias the analysis. From "BIG DATA" Viktor Mayer-Schonberger and Kenneth Cukier

The main point of the paragraph is:

- (A) The twenty-first century is a digital world
- (B) Big data is obsessed with exactness
- (C) Exactitude is not critical in dealing with big data
- (D) Sparse data leads to a bias in the analysis

Answer: (C)

8. The total exports and revenues from the exports of a country are given in the two pie charts below. The pie chart for exports shows the quantity of each item as a percentage of the total quantity of exports. The pie chart for the revenues shows the percentage of the total revenue generated through export of each item. The total quantity of exports of all the items is 5 lakh tonnes and the total revenues are 250 crore rupees. What is the ratio of the revenue generated through export of Item 1 per kilogram to the revenue generated through export of Item 4 per kilogram?





(A) 1:2

(B) 2:1

(C) 1:4

(D) 4:1

Answer: (D)

Exp: re

revenue generated through export of item 1 Kg

$$\Rightarrow \frac{\text{Item}}{\text{quantity}} = \frac{11}{100} \times 5 = \frac{11}{20} \text{ (lakhs tows)}$$

revenue gen
Item!
$$\frac{12}{100} \times 6 \times 250 \times (C) = \frac{30 \text{cr}}{11} \times 20 \quad ...(1)$$

Re venue gen Item $4 = \frac{6}{100} \times 250 (C)$

Objective Civil Eng. \ 2016 \ 68

$$= \frac{15 \text{ cr}}{22} \times 20 \text{ Lt.} \quad ...(2)$$
1:2
$$\frac{30}{11} \times \frac{20 \times 22}{15 \times 20} = 4:1$$

9. X is 1 km northeast of Y. Y is 1 km southeast of Z. W is 1 km west of Z. P is 1 km south of W. Q is 1 km east of P. What is the distance between X and Q in km?

(A) 1

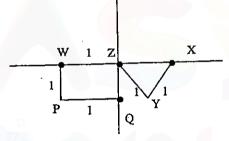
- (B) 2
- (C) 3
- (D) 2

Answer: (C)

Exp: From the fig: $zx = \sqrt{2}$. [Pythagoras theorem] zQ = 1 Given

⇒ Considering ZQX, which is right angle, is

$$\Rightarrow Qx^2 = ZQ^2 + Zx^2$$
$$= \sqrt{1+2}$$



$$=\sqrt{3}$$

10. 10% of the population in a town is HIV+. A new diagnostic kit for HIV detection is available; this kit correctly identifies HIV+ individuals 95% of the time, and HIV" individuals 89% of the time. A particular patient is tested using this kit and is found to be positive. The probability that the individual is actually positive is _____

Answer: 0.48 to 0.49

Exp: Let total population = 100

$$HIV + patients = 10$$

For the patient to be +Ve, should be either +Ve and test is showing +Ve or the patient should

be - Ve but rest is showing +Ve

$$\Rightarrow \frac{0.1 \times 0.95}{0.1 \times 0.95 + 0.9 \times 0.11}$$

Q. No. 1-25 carry one Mark Each

- A fair (unbiased) coin was tossed four times in succession and resulted in the following outcomes: (i) Head, (ii) Head, (iii) Head, (iv) Head. The probability of obtaining a 'Tail' when the coin is tossed again is
 - (A) 0

- (B) $\frac{1}{2}$ (C) $\frac{4}{5}$
- (D) $\frac{1}{5}$

Answer: (B)

Exp.

$$P(T) = \frac{1}{2}$$

The determinant of matrix $\begin{bmatrix} 0 & 1 & 2 & 3 \\ 1 & 0 & 3 & 0 \\ 2 & 3 & 0 & 1 \\ 3 & 0 & 1 & 2 \end{bmatrix}$ is

Answer: 88 to 88

Exp.

$$\begin{vmatrix} 0 & 1 & 2 & 3 \\ 1 & 0 & 3 & 0 \\ 2 & 3 & 0 & 1 \\ 3 & 0 & 1 & 2 \end{vmatrix} = -1 \begin{vmatrix} 1 & 2 & 3 \\ 3 & 0 & 1 \\ 0 & 1 & 2 \end{vmatrix} - 3 \begin{vmatrix} 0 & 1 & 3 \\ 2 & 3 & 1 \\ 3 & 0 & 2 \end{vmatrix}$$
$$= [1(0 - 1) - 2 (6 - 0) + 3 (3 - 0)]$$
$$= -3 [0 - 1 (4 - 3) + 3 (0 - 9)] = 88$$

- $z = \frac{2-3i}{-5+i}$ can be expressed as
 - (A) 0.5 0.5i
- (B) -0.5 + 0.5i
- (C) 0.5 0.5i
- (D) 0.5 + 0.5i

Exp.
$$z = \frac{2-3i}{-5+i}$$

$$z = \frac{2-3i}{-5+i} \times \frac{-5-i}{-5-i}$$

$$= \frac{-10-2i+15i+3i^2}{5^2-i^2} = \frac{-10+13i-3}{25+1}$$

$$= \frac{13i-13}{26} = 13 \frac{(i-1)}{26} = \frac{i-1}{2} = \frac{i}{2} - \frac{i}{2} = 0.5i = 0.5.$$

4. The integrating factor for the differential equation $\frac{dp}{dt} + k_2 P = k_1 L_0 e^{-k_1 t}$ is

(A)
$$e^{-k_1t}$$

(B)
$$e^{-k_2t}$$

(C)
$$e^{k_1t}$$

(D)
$$e^{k_2t}$$

Answer: (D)

Exp.
$$\frac{dp}{dt} + k_2 P = k_1 L_0 e^{k_2 t}$$

The standard form of Linear differential equations is

$$\frac{dy}{dx} + py = Q; l.F = e^{\int pdx}$$

$$\Rightarrow \frac{\mathrm{dp}}{\mathrm{dt}} + k_2 P = \left(K_1 L_0 e^{-k_1 t} \right)$$

Integrating factor, I.F. = $e^{\int k_2 dt} = e^{k_2 t}$

5. If $\{x\}$ is a continuous, real valued random variable defined over the interval $(-\infty, +\infty)$ and its occurrence is defined by the density function given as: $f(x) = \infty$

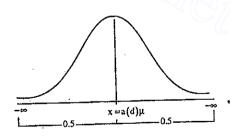
 $\frac{1}{\sqrt{2\pi * b}} e^{\frac{1}{2} \left(\frac{x-a}{b}\right)^2}$ where 'a' and 'b' are the statistical attributes of the random variable

{x}. The value of the integral $\int_{-\infty}^{a} \frac{1}{\sqrt{2\pi * b}} e^{\frac{1}{2} \left(\frac{x-a}{b}\right)^{2}} dx \text{ is}$

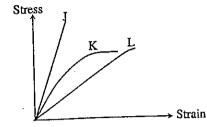
(D)
$$\frac{\pi}{2}$$

Answer: (B)

Exp. We have $\int_{-\infty}^{\infty} f(x) dx = 1$ $\Rightarrow \int_{-\infty}^{a} f(x) dx + \int_{a}^{-\infty} f(x) dx = 1$ $\Rightarrow \int_{a}^{a} f(x) dx = 0.5$



6. Group I contains representative stress-strain curves as shown in the figure, while Group II gives the list of materials. Match the stress-strain curves with the corresponding materials.



| Gro | up I | | Group II |
|-----|---------|-----|------------------|
| (p) | Curve J | (1) | Cement paste |
| (q) | Curve K | (2) | Coarse aggregate |
| (r) | Curve L | (3) | Concrete |

(A) P - 1; Q - 3; R - 2

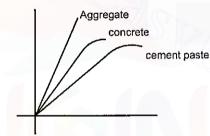
(B) P - 2; Q - 3; R - 1

(C) P-3; Q-1; R-2

(D) P - 3; Q - 2; R - 1

Answer: (B)

Exp.



So,
$$P = 2$$
, $Q = 3$, $R = 1$

- 7. The first moment of area about the axis of bending for a beam cross-section is
 - (A) moment of inertia

(B) section modulus

(C) shape factor

(D) polar moment of inertia

Answer: (B)

Exp. \therefore Section modulus, $z = \frac{I}{r} = \frac{A \cdot r^2}{r} = A \cdot r$, i.e. Moment of Area

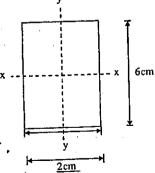
8. Polar moment of inertia (I_p) , in cm4, of a rectangular section having width, b = 2 cm and depth, d = 6 cm is _____

Answer: 40.00 to 40.00

Exp.
$$I_p = I_{xx} + I_{yy}$$

= $\frac{2 \times (6)^3}{12} + \frac{6(2)^3}{12}$
= 40 cm^4

M.I. about axis perpendicular to the plane is called polar M.I.



The target mean strength f_{cm} for concrete mix design obtained from the characteristic strength f_{ck} and standard deviation s, as defined in IS:456-2000, is

(A)
$$f_{ck} + 1.35\sigma$$

(B)
$$f_{ck} + 1.45\sigma$$

(C)
$$f_{ck} + 1.55c$$

(C)
$$f_{ck} + 1.55\sigma$$
 (D) $f_{ck} + 1.65\sigma$

Answer: (D)

 $f_m = f_{ck} + 1.65\sigma$ As per IS: 456.2000

The flexural tensile strength of M25 grade of concrete, in N/mm², as per IS:456-2000

Answer: 3.5 to 3.5

Flexural tensile strength, $f_{cr} = 0.7 \sqrt{f_{ck}} \text{ N/mm}^2$ IS: 456 - 2000 $= 0.7 \times \sqrt{25}$ P.16 $= 3.5 N / mm^2$

11. The modulus of elasticity, ck E = 5000 f where f_{ck} is the characteristic compressive strength of concrete, specified in IS:456-2000 is based on

(A) tangent modulus

(B) initial tangent modulus

(C) secant modulus

(D) chord modulus

Answer: (B)

12. The static indeterminacy of the two-span continuous beam with an internal hinge, shown below, is



Answer: 0 to 0

Exp.
$$D_{se} = R - r$$

= 4 - 4 = 0

one extra condition is provided by internal hinge.

13. As per Indian Standard Soil Classification System (IS: 1498 - 1970), an expression for A-line is

(A)
$$I_p = 0.73 (w_L - 20)$$

(B)
$$I_p = 0.70 \text{ (w}_L - 20)$$

(C)
$$I_p = 0.73 (w_L - 10)$$

(D)
$$I_p = 0.70 \text{ (w}_L - 10)$$

Answer: (A)

Exp.
$$I_p = 0.73 \text{ (W}_L - 20)$$

- The clay mineral primarily governing the swelling behavior of Black Cotton soil is
 - (A) Halloysite
- (B) I llite
- (C) Kaolinite
- (D) Montmorillonite

Answer: (D)

- 15. The contact pressure for a rigid footing resting on clay at the centre and the edges are respectively
 - (A) maximum and zero

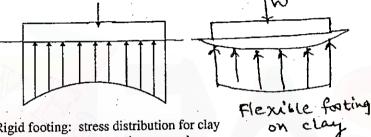
(B) maximum and minimum

(C) zero and maximum

(D) minimum and maximum

Answer: (D)

Exp.



Rigid footing: stress distribution for clay minimum at centre, maximum at edge.

$$G_s = 2.71, n = 40\% = 0.40,$$

 $W = 20\%, S = ?$

16. A certain soil has the following properties: Gs = 2.71, n = 40% and w = 20%. The degree of saturation of the soil (rounded off to the nearest percent) is

Answer: 81.0 to 81.5

Exp.
$$G_s = 2.71$$
, $n = 40\% = 0.40$, $w = 20\%$, S ? $e = \frac{n}{1-n} = \frac{0.4}{0.6} = 0.67$ $S = \frac{wG}{e} = \frac{0.20 \times 2.71}{0.67} = 0.808 = 81\%$

- 17. A plane flow has velocity components $u = \frac{x}{T_1}$, $v = -\frac{y}{T_2}$ and w = 0 and x, y and z directions respectively, where $T_1 (\neq 0)$ and $T_2 (\neq 0)$ are constants having the dimensions of time. The given flow is incompressible if

 - (A) $T_1 = -T_2$ (B) $T_1 = -\frac{T_2}{2}$ (C) $T_1 = \frac{T_2}{2}$ (D) $T_1 = T_2$

Answer: (D)

Exp.
$$U = \frac{x}{T_1}$$
, $V = \frac{-y}{T_2}$, $w = 0$

For incompressible flow

$$\frac{2v}{2x} + \frac{2v}{2y} + \frac{2w}{2z} = 0$$

$$\Rightarrow \frac{1}{T_1} - \frac{1}{T_2} = 0 \Rightarrow T_1 = T_2$$

18. Group I lists a few devices while Group II provides information about their uses.

Match the devices with their corresponding use.

| Group I | Group II |
|-----------------|---|
| (p) Anemometer | (1) Capillary potential of soil water |
| (q) Hygrometer | (2) Fluid velocity at a specific point in the flow stream |
| (r) Pitot Tube | (3) Water vapour content of air |
| (s) Tensiometer | (4) Wind speed |

(D)
$$P - 4$$
; $Q - 3$; $R - 2$; $S - 1$

Answer: (D)

Exp. Aneometer → wind speed

Hygrometer → water vapour content of air

Pilot tube → flow velocity at a specific point in the flow stream

Tensionmeter → Capillary potential of soil water

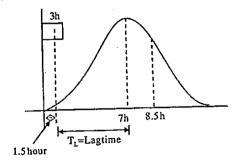
19. An isolated 3-h rainfall event on a small catchment produces a hydrograph peak and point of inflection on the falling limb of the hydrograph at 7 hours and 8.5 hours respectively, after the start of the rainfall. Assuming, no losses and no base flow contribution, the time of concentration (in hours) for this catchment is approximately

Answer: (D)

Exp.

For small catchment, time of concentration is equal to lag time of peak flow.

$$T_c = 7 - 1.5 = 5.5h$$



20. The Muskingum model of routing a flood through a stream reach is expressed as $O_2 = K_0 I_2 + K_1 I_1 + K_2 O_1$, where K_0 , K_1 and K_2 are the routing coefficients for the concerned reach, I_1 and I_2 are the inflows to reach, and O_1 and O_2 are the are the outflows from the reach corresponding to time steps 1 and 2 respectively. The sum of K_0 , K_1 and K_2 of the model is

(A) -1

- (B) -0.5
- (C) 0.5
- (D) ·1

Answer: (D)

Exp. $K_0 + K_1 + K_2 = 1$ 1.5 hour

- 21. The dominating microorganisms in an activated sludge process reactor are
 - (A) aerobic heterotrophs
- (B) anaerobic heterotrophs
- (C) autotrophs
- (D) phototrophs

Answer: (A)

- 22. The two air pollution control devices that are usually used to remove very fine particles from the flue gas are
 - (A) Cyclone and Venturi Scrubber
- (B) Cyclone and Packed Scrubber
- (C) Electrostatic Precipitator and Fabric Filter (D) Settling Chamber and Tray Scrubber

Answer: (C)

23. The average spacing between vehicles in a traffic stream is 50 m, then the density (in veh/km) of the stream is _____

Answer: 20.0 to 20.0

Exp. Density,
$$K = \frac{1000}{s} = \frac{1000}{50} = 20 \text{ veh/km}$$

- 24. A road is being designed for a speed of 110 km/hr on a horizontal curve with a super elevation of 8%. If the coefficient of side friction is 0.10, the minimum radius of the curve (in m) required for safe vehicular movement is
 - (A) 115.0
- (B) 152.3
- (C) 264.3
- (D) 528.5

Answer: (D)

Exp. Ruling gradient = $\frac{V^2}{127(e+f)}$

$$= \frac{110 \times 110}{127 (0.08 + 0.10)} = 529.30 \text{ m}$$

Objective Civil Eng. \ 2016 \ 69

- 25. The survey carried out to delineate natural features, such as hills, rivers, forests and manmade features, such as towns, villages, buildings, roads, transmission lines and canals is classified as
 - (A) engineering survey

(B) geological survey

(C) land survey

(D) topographic survey

Answer: (D)

Exp. Topographic survey is carried out to delineate natural features.

O. No. 26-55 carry Two Marks

26. The expression $\lim_{\alpha\to 0} \frac{x^{\alpha}-1}{\alpha}$ is equal to

- (A) log x
- (B) 0
- (C) $x \log x$
- (D) ∞

Answer: (A)

Exp. By L' Hospital rule,

$$L = \frac{\frac{d}{da}(x^{\alpha-1})}{\frac{d}{d\alpha}(\alpha)}$$

$$= \alpha \stackrel{\lim}{\to} 0 \frac{x^{\alpha} \cdot \log x}{1} = \log x$$

27. An observer counts 240 veh/h at a specific highway location. Assume that the vehicle arrival at the location is Poisson distributed, the probability of having one vehicle arriving over a 30- second time interval is ______

Answer: 0.25 to 0.28

Exp. Average no. of vehicles per hour,

$$\lambda = 240 / hour$$

$$=\frac{240}{60}$$
 / min

= 4 / min 2 / 30sec

$$P(x = 1) = \frac{e^{-2}.2!}{1!} 0.27$$

28. The rank of matrix
$$\begin{bmatrix} 6 & 0 & 4 & 4 \\ -2 & 14 & 8 & 8 \\ 14 - 14 & 0 - 10 \end{bmatrix}$$
 is _____

Answer: 2.0 to 2.0

Exp.
$$\begin{bmatrix} 6 & 0 & 4 & 4 \\ -2 & 14 & 8 & 8 \\ 14 & -14 & 0 & -10 \end{bmatrix} R2 \rightarrow 3R_2 + R_1 ; R_3 \rightarrow 6R_3 - 14R_1$$

$$\sim \begin{bmatrix} 6 & 0 & 4 & 4 \\ 0 & 42 & 28 & 58 \\ 0 - 84 - 56 & 0 \end{bmatrix} R_3 \rightarrow R_3 + 2R_2$$

$$\sim \begin{bmatrix}
6 & 0 & 4 & 4 \\
0 & 42 & 28 & 58 \\
0 & 0 & 0 & 0
\end{bmatrix}$$

$$\rightarrow$$
 Rank = No. of non-zero rows = 2

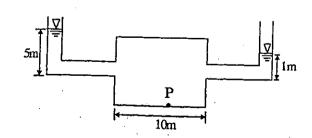
29. Water is flowing at a steady rate through a homogeneous and saturated horizontal soil strip of 10 m length. The strip is being subjected to a constant water head (H) of 5 m at the beginning and 1 m at the end. If the governing equation of flow in the soil strip

is
$$\frac{d^2 H}{dx^2} = 0$$
 = (where x is the distance along the soil strip), the value of H (in m) at the middle of the strip is _____

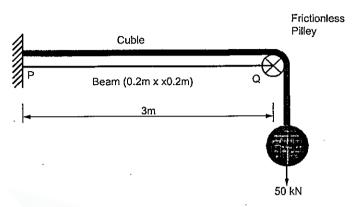
Answer: 3.0 to 3.0

Exp.
$$\frac{d^2 H}{dx^2} = 0$$

$$\Rightarrow \frac{dH}{dX} = K \Rightarrow H = Kx + C$$
at $x = 0$, $H = 5 \Rightarrow C = 5$
at $x = 10$, $H = 1 \Rightarrow K = -0.4$
so, $H = -0.4x + 5$
At $x = 5$, $H = -0.4 \times 5 + 5 = 3$ m



30. The values of axial stress (σ) in kN/m², bending moment (M) in kNm, and shear force (V) in kN acting at point P for the arrangement shown in the figure are respectively



- (A) 1000, 75 and 25
- (C) 1500, 225 and 75

- (B) 1250, 150 and 50
- (D) 1750, 300 and 100

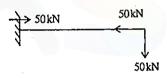
Answer: (B)

Exp. Loading after removing of cable

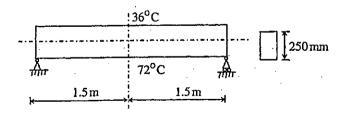
Axial stress =
$$\frac{50}{0.2 \times 0.2} = 1250 \text{ kN/m}^2$$

B.M. =
$$50 \times 3 = 150 \text{ kNm}$$

S.F = 50kN



31. The beam of an overall depth 250 mm (shown below) is used in a building subjected to two different thermal environments. The temperatures at the top and bottom surfaces of the beam are 36°C and 72°C respectively. Considering coefficient of thermal expansion (α) as 1.50 × 10⁻⁵ per °C, the vertical deflection of the beam (in mm) at its mid-span due to temperature gradient is _____



Answer: 2.38 to 2.45

Exp. From properties of circle,

$$(2R - \Delta) \cdot \Delta = \frac{L}{2} \times \frac{L}{2}$$

 \therefore Δ is very small, neglect Δ^2

$$2R.\Delta = \frac{L^2}{4}$$

$$\Rightarrow \Delta = \frac{L^2}{8R} \qquad \text{But } R = \frac{h}{\alpha T}$$

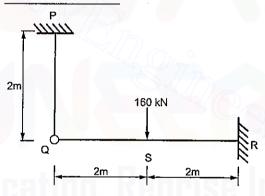
$$=\frac{L_2}{8.h}.\alpha T$$

Here,
$$L = 3m$$
, $\alpha = 1.50 \times 10^{-5} / {}^{\circ}C$,

$$T = 72^{\circ} - 36^{\circ} = 36^{\circ} C$$

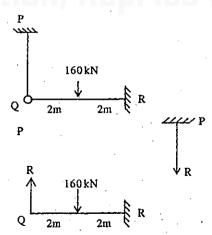
$$\Delta = \frac{\left(3\right)^2 \times 1.5 \times 10^{-5} \times 36}{8 \times 0.250} = 0.00243 \text{ m } 2.43 \text{ mm}$$

32. The axial load (in kN) in the member PQ for the arrangement/assembly shown in the figure given below is _____



Answer: 50.0 to 50.0

Exp.



Taking PQ to be rigid; so, $\Delta_Q = 0$

$$\Rightarrow = \frac{R(4)^3}{3EI} = \frac{160(2)^3}{3EI} + \frac{160(2)^2}{2EI} \times 2$$

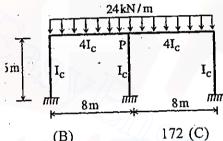
def. at φ due to R = def. at φ due to 160 kN

$$\Rightarrow 64R = 160 \times 8 + 160 \times 4 \times 3$$

$$\Rightarrow$$
 R = 50kN

So, Tension in PQ = 50 kN

Considering the symmetry of a rigid frame as shown below, the magnitude of the 33 bending moment (in kNm) at P (preferably using the moment distribution method) is



(A) 170

(B)

176(D) 178

Answer: (C)

Axis of symmetry is passing through a column; hence it can be treated as Exp.

D.F

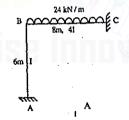
Member

Stiffness

FE11:

BA

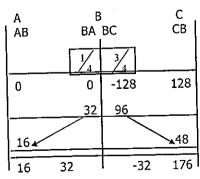
BC



$$M_{BC} = \frac{-WL^2}{12} = \frac{-24 \times 8 \times 8}{12} = -128kNM$$

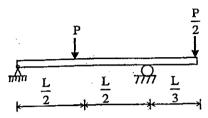
$$M_{CB} = + 128 \text{ kNM}$$

B.M. at C = 176 kN.m



Gate - 2014, Paper-2

34. A prismatic beam (as shown below) has plastic moment capacity of Mp, then the collapse load P of the beam is



- (A) $\frac{2M_{\rho}}{I}$
- (B) $\frac{4M_{\rho}}{L}$
- (C) $\frac{6M_{\rho}}{L}$
- (D) $\frac{8M_{\rho}}{L}$

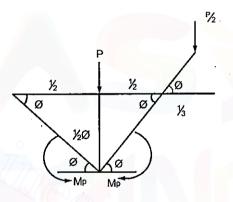
Answer: (C)

Exp. Degree of static indeterminacy $D_s = 0$ Number of plastic hinges = $D_s + 1 = 1$

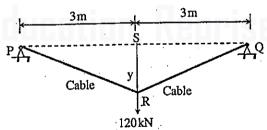
From principal of virtual work

$$Mp.\theta + Mp.\theta = P = \left(\frac{L}{2}\theta\right) - \frac{P}{2} \cdot \left(\frac{L}{3} \cdot \theta\right) = 0$$

$$\Rightarrow P = \frac{6M_P}{L}$$



35. The tension (in kN) in a 10 m long cable, shown in the figure, neglecting its self-weight is



- (A) 120
- (B) 75
- (C) 60
- (D) 45

Answer: (B)

Exp. Taking moment about Q = 0

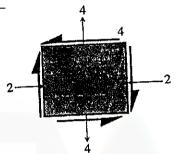
$$R \times 6 - 120 \times 3 = 0 \implies R_A = 60kN$$

Taking moment about R = 0

$$R \times 3 - H \times 4 = 0 \implies H = \frac{3}{4} \times 60 = 45 \text{kN}$$

 $T = \sqrt{R^2 + H^2} = \sqrt{(60)^2 + (45)^2} = 75 \text{kN}$

36. For the state of stresses (in MPa) shown in the figure below, the maximum shear stress (in MPa) is ______



Answer: 5.0 to 5.0

Exp.
$$\sigma_{x} = -2, \ \sigma_{y} = 4, \ \tau = 4$$

Max shear stress,
$$t_{max} = \frac{\sigma_1 - \sigma_2}{2}$$

Where,
$$\sigma_1 = \frac{\sigma_x - \sigma_y}{2} + \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + \tau^2}$$

$$= \frac{-2+4}{2} + \sqrt{\left(\frac{-2-4}{2}\right)^2 + \left(4\right)^2} = 1+5=6 \text{ MPa}$$

$$\sigma_1 = \frac{\sigma_x - \sigma_y}{2} + \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + \tau_2} = 1 - 5 = -4 \text{ MPa}$$

So,
$$\tau_{\text{max}} = \frac{6 - (-4)}{2}$$
 5 MPa

37. An infinitely long slope is made up of a c- φ soil having the properties: cohesion (c) = 20 kPa, and dry unit weight (γ_d) = 16kN/m³. The angle of inclination and critical height of the slope are 40° and 5 m, respectively. To maintain the limiting equilibrium, the angle of internal friction of the soil (in degrees) is ______

Answer: 21.0 to 23.0

Exp. Given, C = 20KPa,
$$\gamma d = 16 \text{ kN / m}^3$$

 $\beta = 40^\circ$, H = 5m, $\phi = ?$

$$H = \frac{C}{\gamma_d (\tan \beta - \tan \phi) \cos^2 \beta}$$

$$\Rightarrow 5 = \frac{20}{16 \left(\tan 40^{\circ} - \tan \phi \right) \cdot \cos^2 40^{\circ}}$$
$$\Rightarrow \phi = 22.44^{\circ}$$

38. Group I enlists in-situ field tests carried out for soil exploration, while Group II provides a list of parameters for sub-soil strength characterization. Match the type of tests with the characterization parameters

| Group I | Group II |
|---|-----------------------------|
| (P) Pressuremeter Test (PMT) | (1) Menard's modulus (Em) |
| (Q) Static Cone Penetration Test (SCPT) | (2) Number of blows (N) |
| (R) Standard Penetration Test (SPT) | (3) Skin resistance (fc) |
| (S) Vane Shear Test (VST) | (4) Undrained cohesion (cx) |

(D)
$$P - 4$$
; $Q - 1$; $R - 2$; $S - 3$

Answer: (A)

39. A single vertical friction pile of diameter 500 mm and length 20 m is subjected to a vertical compressive load. The pile is embedded in a homogeneous sandy stratum where: angle of internal friction $(fd) = 30^{\circ}$, dry unit weight $(fd) = 20 \text{ kN/m}^3$ and angle of wall friction $(f\tilde{N}) = 2f\tilde{N}/3$. Considering the coefficient of lateral earth pressure (K) = 2.7 and the bearing capacity factor (Nq) = 25, the ultimate bearing capacity of the pile (in kN) is ______

Answer: 6150 to 6190

$$Q_U = f_s \cdot A_s$$

Where,
$$f_s = \frac{1}{2} \sigma_v \cdot K \cdot \tan \delta$$

$$\sigma_{\rm v} = \gamma_{\rm d} \times L$$
 = 20 × 20 = 400kN / m²

$$K = 2.7$$

$$\tan \delta = \tan \left(\frac{2}{3}\phi\right) = \tan \left(\frac{2}{3}\times 30\right) = 30 \ 0.364$$

So,
$$f_s = \frac{1}{2} \times 400 \times 2.7 \times 0.364 = 196.56 \text{ kN } / \text{ m}^2$$

So,
$$Q_u = (196.56) \times \pi D.L = 196.56 \times \pi \times 0.5 \times 20 = 6175kN$$

Objective Civil Eng. \ 2016 \ 70

A circular raft foundation of 20 m diameter and 1.6m thick is provided for a tank that 40. applies a bearing pressure of 110 kPa on sandy soil with Young's modulus, E,' = 30 MPa and Poisson's ration, $v_s = 0.3$. The raft is made of concrete $E_c = 30$ GPa and $v_c = 0.15$. Considering the raft as rigid, the elastic settlement (in mm) is

Answer: (B)

Elastic settlement of rigid footing Exp.

$$S = 0.8 \left[\frac{qB\left(1 - \mu^2\right)}{E} \right]$$

Given, $q = 110 \text{kN/m}^2$, B = 20 m, $\mu = 0.30$

$$E_s = 30GPa = 30 \times 10^3 \text{ kN/m}^2$$

E_s = 30GPa = 30 × 10³ kN/m²
S = 0.8
$$\left[\frac{110 \times 20 \times (1 - 0.09)}{30 \times 10^3} \right]$$
 = 53.38 mm

41. A horizontal nozzle of 30 mm diameter discharges a steady jet of water into the atmosphere at a rate of 15 litres per second. The diameter of inlet to the nozzle is 100 mm. The jet impinges normal to a flat stationary plate held close to the nozzle end. Neglecting air friction and considering the density of water as 1000 kg/m³, the force exerted by the jet (in N) on the plate is

Answer: 318 to 319

Velocity of jet, $v = \frac{Q}{A} = \frac{15 \times 10^{-3} \text{ m}^3/\text{s}}{\frac{\pi}{4} \times (0.03)^2} = 21.22 \text{m/s}$

Force on plate,
$$F = \rho a.v^2$$

=
$$1000 \times \frac{\pi}{4} \times (0.03)^2 \times (21.22)^2$$

= 318.29 N

42. A venturimeter having a throat diameter of 0.1 m is used to estimate the flow rate of a horizontal pipe having a diameter of 0.2 m. For an observed pressure difference of 2 m of water head and coefficient of discharge equal to unity, assuming that the energy losses are negligible, the flow rate (in m³/s) through the pipe is approximately equal to

(A) 0.500

Answer: (C)

Exp.
$$Q = C_d \cdot \frac{a_1 \times a_2}{\sqrt{a_1^2 - a_2^2}} \cdot \sqrt{2gh}$$

$$a_1 = \frac{\pi}{4} \times (0.2)^2 = 0.0314 \text{ m}^2$$

$$a_2 = \frac{\pi}{4} \times (0.1)^2 = 0.0078 \text{ m}^2$$
so, Q =
$$\frac{1 \times 0.0314 \times 0.0078}{\sqrt{(0.314)^2 - (0.0078)^2}} \times \sqrt{2 \times 9.81 \times 2} = 0.050 \text{ m}^3/\text{s}$$

43. A rectangular channel of 2.5 m width is carrying a discharge of 4 m3/s. Considering that acceleration due to gravity as 9.81 m/s², the velocity of flow (in m/s) corresponding to the critical depth (at which the specific energy is minimum) is _____

Answer: 2.45 to 2.55

Exp. B =2.5m, Q = 4m³ / s , g = 9.81/ s²

Critical depth,
$$y_c = \left(\frac{q^2}{g}\right)^{1/3}$$

So, $y_c = \left[\frac{(1.6)^2}{9.81}\right]^{1/3} = 0.64m$

Now, Q = A.v_c

$$\Rightarrow v_c = \frac{Q}{B.y_c} = \frac{4}{2.5 \times 0.64} = 2.5m/s$$

44. Irrigation water is to be provided to a crop in a field to bring the moisture content of the soil from the existing 18% to the field capacity of the soil at 28%. The effective root zone of the crop is 70 cm. If the densities of the soil and water are 1.3 g/cm³ and 1.0 g/cm³ respectively, the depth of irrigation water (in mm) required for irrigating the crop is _____

Answer: 91 to 91

Exp. Depth of irrigation water

$$d = \frac{\gamma_d}{\gamma_w} d (F.C - W.P)$$

$$= \frac{1.3}{1} \times 70 \times (0.28 - 0.18)$$

$$= 9.1 \text{cm} \implies 91 \text{mm}$$

45. With reference to a standard Cartesian (x, y) plane, the parabolic velocity distribution profile of fully developed laminar flow in x-direction between two parallel, stationary and identical plates that are separated by distance, h, is given by the expression

$$u = -\frac{h^2}{8\mu} \frac{dp}{dx} \left[1 - 4 \left(\frac{y}{h} \right)^2 \right]$$

In this equation, the y = 0 axis lies equidistant between the plates at a distance h/2from the two plates, p is the pressure variable and μ is the dynamic viscosity term. The maximum and average velocities are, respectively

(A)
$$u_{\text{max}} = -\frac{h^2}{8\mu} \frac{dp}{dx}$$
 and $u_{\text{average}} = \frac{2}{3} u_{\text{max}}$ (B) $u_{\text{max}} = \frac{h^2}{8\mu} \frac{dp}{dx}$ and $u_{\text{average}} = \frac{2}{3} u_{\text{max}}$

(B)
$$u_{\text{max}} = \frac{h^2}{8\mu} \frac{dp}{dx}$$
 and $u_{\text{average}} = \frac{2}{3} u_{\text{max}}$

(C)
$$u_{\text{max}} = -\frac{h^2}{8\mu} \frac{dp}{dx}$$
 and $u_{\text{average}} = \frac{3}{8} u_{\text{max}}$ (D) $u_{\text{max}} = \frac{h^2}{8\mu} \frac{dp}{dx}$ and $u_{\text{average}} = \frac{3}{8} u_{\text{max}}$

(D)
$$u_{\text{max}} = \frac{h^2}{8\mu} \frac{dp}{dx}$$
 and $u_{\text{average}} = \frac{3}{8} u_{\text{max}}$

Answer: (A)

Exp.

$$U = \frac{h^2}{8\mu} \left(\frac{dp}{dx} \right) \left[1 - 4 \left(\frac{y}{h} \right)^2 \right]$$

Maximum velocity is at y = 0

$$U_{\text{max}} = \frac{-h^2}{8\mu} \left(\frac{dp}{dx}\right)$$

$$U_{avg} = \frac{Q}{A} = \frac{\int U.dA}{A} = \frac{\int U_{max} \left(1 - \frac{4y^2}{h^2}\right). dA}{A}$$

$$= \frac{2 \int_{0}^{h/2} U_{\text{max}} \left(1 - \frac{4y^{2}}{h^{2}}\right) \cdot dy \times 1}{h \times 1} = \frac{2U_{\text{max}}}{h} \left[y - \frac{4y^{3}}{3h^{2}} \Big|_{0}^{h/2} \right]$$

$$= \frac{2U_{\text{max}}}{h} \left[\frac{h}{2} - \frac{4h^3}{24h^2} \right] = \frac{2}{3} U_{\text{max}}$$

46. A suspension of sand like particles in water with particles of diameter 0.10 mm and below is flowing into a settling tank at 0.10 m³/s. Assume g = 9.81 m/s², specific gravity of particles = 2.65, and kinematic viscosity of water =1.0105×10-2 cm²/s. The minimum surface area (in m2) required for this settling tank to remove particles of size 0.06 mm and above with 100% efficiency is

Answer: 31.0 to 32.0

Particle size = 0.06m < 0.1 mm Exp.

So, Stoke's law is valid

 $\frac{Q}{A}$ = overflow rate \leq settling velocity of 0.06mm particle

$$\Rightarrow \frac{Q}{A} \le V_s \Rightarrow A = \frac{Q}{V_s}$$

$$V_{s} = \frac{1}{18} \cdot \frac{d^{2}g}{V} (G_{s} - 1) = \frac{1}{18} \times \frac{(0.06 \times 10^{-3})^{2} \times 9.81}{1.0105 \times 10^{-6}} \times (2.65 - 1) = 3.20 \times 10^{-3} \text{ m/s}$$

$$\Rightarrow A \ge \frac{0.1}{3.20 \times 10^{-3}} = 31.21 \text{ m}^{2}$$

47. A surface water treatment plant operates round the clock with a flow rate of 35 m³/min. The water temperature is $15f\tilde{N}$ and jar testing indicated an alum dosage of 25 mg/l with flocculation at a Gt value of 4×10^4 producing optimal results. The alum quantity required for 30 days (in kg) of operation of the plant is ______

Answer: 37800 to 37800

Exp. G
$$I = 35m^3 / min = 35 \times 10^3 \times 60 \times 24 l / day$$

Alum dosage = 25 mg/l

Alum quantity required for 30 days

$$= 35 \times 10^3 \times 60 \times 24 \times 25 \times 10^{-6} \times 30 = 37800$$
kg.

48. An effluent at a flow rate of 2670 m³/d from a sewage treatment plant is to be disinfected. The laboratory data of disinfection studies with a chlorine dosage of 15 mg/l yield the model $N_t = N_0 e^{-0.145t}$ where $N_t =$ number of micro-organisms surviving at time t (in min.) and $N_0 =$ number of micro-organisms present initially (at t = 0). The volume of disinfection unit (in m³) required to achieve a 98% kill of micro-organisms is

Answer: 49.0 to 51.0

Exp.
$$Q = 2760 \text{ m}^3 / \text{day}$$
, chlorine dose $15 \text{mg} / I$

$$N_b = N_0 \cdot e^{-0.145t}$$

If 98% micro organisms are killed, 2% are surviving.

So,
$$N_t = 0.02$$

 N_0 = number of microorganisms present at t = 0

ie.,
$$100\% = 1$$

So,
$$0.02 e^{-0.145t}$$
 \Rightarrow $t = 26.98min utes$

$$V = Q \times t_d = \left(2670 \frac{m^3}{d}\right) \times \left(\frac{26.98}{60 \times 24}\right) = 50.02 m^3$$

49. A waste water stream (flow = 2 m³/s, ultimate BOD = 90 mg/l) is joining a small river (flow = 12 m³/s, ultimate BOD = 5 mg/l). Both water streams get mixed up instantaneously. Crosssectional area of the river is 50 m². Assuming the de-oxygenation rate constant, k' = 0.25/day, the BOD (in mg/l) of the river water, 10 km downstream of the mixing point is

| Objective | Civil | Engineering |
|-----------|-------|-------------|
|-----------|-------|-------------|

(A) 1.68

(B) 12.63

(C) 15.46

(D) 1.37

Answer: (C)

Exp.

 $Q_s = 2m^3$ / s, BOD ultimate, $L_s = 90mg / l$ $Q_s = 12m^3 / s$, $L_R = 5mg / l$

 $(BOD)_{mix} = \frac{90 \times 2 + 12 \times 5}{2 + 12} = 17.14 \text{mg} / l$

Velocity of River flow, $V_R = \frac{Q}{A} = \frac{12+2}{50} = 0.28 \text{ m/s}$

Time taken to travel $10 \text{km} = \frac{1000}{0.28} = 35714.28 \text{s} = 0.41 \text{d}$

 $L_t = L_0 \times e^{-kt} = 17.14 \times e^{-0.25 \times 0.41} = 15.46 \text{ mg} / l$

50. In a Marshall sample, the bulk specific gravity of mix and aggregates are 2.324 and 2.546 respectively. The sample includes 5% of bitumen (by total weight of mix) of specific gravity 1.10. The theoretical maximum specific gravity of mix is 2.441. The void filled with bitumen (VFB) in the Marshall sample (in %) is

Answer: 62 to 66

Exp.

$$VFB = \frac{V_h}{V_{MA}} \times 100$$

Where, V_b = voids filled with bitumen = $\frac{G_m}{G_4} \times W_4 = \frac{2.324}{1.10} \times 5 = 10.564$

 $V_{MA} = V_v + V_b$

 $V_V = \text{Volume of voids} = \frac{G_t - G_m}{G_t} \times 100 = \frac{2.441 - 2.324}{2.441} \times 100 \ 4.79\%$

 $V_{MA} = 10.56 + 4.79 = 15.35$

So, $V_{FB} = \frac{10.564}{15.35} \times 100 = 68.82\%$

51. A student riding a bicycle on a 5 km one-way street takes 40 minutes to reach home. The student stopped for 15 minutes during this ride. 60 vehicles overtook the student (assume the number of vehicles overtaken by the student is zero) during the ride and 45 vehicles while the student stopped. The speed of vehicle stream on that road (in km/hr) is

(A) 7.5

(B) 12

(C) 40

(D) 60

Answer: (D)

On a section of a highway the speed-density relationship is linear and is given by $v = \left| 80 - \frac{2}{3} k \right|$; where V is in km/h and K is in veh/km. The capacity (in veh/h) of this section of the highway would be

(A) 1200

(B) 2400

(C) 4800

(D) 9600

Answer: (B)

Exp.

$$V = 80 - \frac{2}{3}k$$

Capacity =
$$\frac{V_f \times k_j}{4}$$
. where

 V_f = free mean velocity

$$K_i$$
 is when, $V = 0$

K_i = Jam density

$$K_j = 80 \times \frac{3}{2} = 120 \text{veh / km}$$

$$V_f$$
 is at $K = 0$, $V_f = 80$ km/h

So, Capacity =
$$\frac{80 \times 120}{4}$$
 = 2400 veh / h

53. A pre-timed four phase signal has critical lane flow rate for the first three phases as 200, 187 and 210 veh/hr with saturation flow rate of 1800 veh/hr/lane for all phases. The lost time is given as 4 seconds for each phase. If the cycle length is 60 seconds, the effective green time (in seconds) of the fourth phase is

Answer: 14.0 to 18.0

Exp.

Total time lost, $t = 4 \times = 16s$; $C = \frac{1.5L + 5}{1 - V}$

$$C = \frac{1.5L + 5}{1 - y}$$

$$Y = y_1 + y_2 + y_3 + y_4$$

$$= \frac{q_1}{s_1} + \frac{q_2}{s_2} + \frac{q_3}{s_3} + \frac{q_4}{s_4}$$

$$= \frac{200}{1800} + \frac{187}{1800} + \frac{210}{1800} + y_4 = 0.332 + y_4$$

$$60 = \frac{1.5 \times + 5}{1 - (0.332 + y_4)} \Rightarrow .668 - y_4 = \frac{29}{60} \Rightarrow y_4 = 0.185$$

Effective green time for 4th phase

$$= \frac{(\text{Co} - \text{L}) \times \text{y}_4}{\text{y}_1 + \text{y}_2 + \text{y}_3 + \text{y}_4} = \frac{(60 - 16) \times 0.185}{0.332 + 0.185} = 15.74\text{s}$$

A tacheometer was placed at point P to estimate the horizontal distances PQ and PR. 54. The corresponding stadia intercepts with the telescope kept horizontal, are 0.320 m and 0.210 m, respectively. The ∠QPR is measured to be 61° 30' 30". If the stadia multiplication constant = 100 and stadia addition constant = 0.10 m, the horizontal distance (in m) between the points Q and R is



Answer: 28.0 to 29.0

Exp. Tacheometric equation,

$$D = kS + C$$

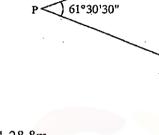
$$D_{PQ} = 100 \times 0.32 + 0.10 = 32.1 m$$

$$D_{PR} = 100 \times 0.210 + 0.10 = 21.1m$$

From Cosine rule in triangle PQR

$$QR^2 = (PQ)^2 + (PR)^2 - 2 (PQ) (PR) \cos 61^{\circ} 30'30''$$

=
$$(32.1)^2$$
 + $(21.1)^2$ - 2 × 32.1 × 21.1 cos61° 30'30" 28.8m



55. The chainage of the intersection point of two straights is 1585.60 m and the angle of intersection is 1400. If the radius of a circular curve is 600.00 m, the tangent distance (in m) and length of the curve (in m), respectively are

(A) 418.88 and 1466.08

(B) 218.38 and 1648.49

(C) 218.38 and 418.88

(D) 418.88 and 218.38

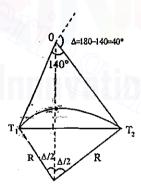
Answer: (C)

Exp. Length of curve, $L = \frac{\pi}{180} \times \Delta \times R$

$$= \frac{\pi}{180} \times 40 \times 600 = 418.88 \text{m}$$

Tangent distance, $T = R \tan (\Delta/2)$

$$= 600 \tan \left(\frac{40}{2}\right) = 218.38 m$$





Gate -2015, Paper-1

GATE 2015 -CE ON 8th FEBRUARY, 2015 - (FORENOON SESSION)

GENERAL APTITUDE QUESTIONS

| | | Q. No. 1-5 carry | y one | Mark Ea | <u>ch</u> | |
|------------|---|--|---------|----------------------------|---------------------------|-------------------------|
| 1. Se | elect the pair that nildren: Pediatricia | best expresses a rela | ationsl | iip similar to | o that expresse | ed in the pair: |
| (<i>A</i> | .) Adult: Orthopa | edist | (B) | Females: Gy | ynaecologist | |
| (0 |) Kidney: Nephro | ologist | (D) | Skin: Derma | atologist | |
| Answe | r: (B) | | | | | |
| Exp: | Community of | people: Doctor | | | | |
| of | treme focus on sy Indian students quirements of the | llabus and studying that this has closed exam | for te | st has becon r minds to | ne such a dom anything | inant concern to the |
| (A |) related | (B) extraneous | (C) | outside | (D) use | eful 💮 💮 |
| Answei | : (B) | | | | | |
| Exp: | extraneous -irre | elevant or unrelated | to the | subject bein | ng dealt with. | |
| 3. If | ROAD is written | as URDG, then SWA | AN sho | ould be writ | ten as: | |
| (A |) VXDQ | (B) VZDQ | (C) | VZDP | (D) UX | (DQ |
| Answei | : (B) | | | | | |
| Exp: | | +3 = R, A + 3 = D | • | , | | |
| | S + 3 = V, W + | -3 = Z, A + 3 = D, | N + 1 | 3 = Q | | |
| the am | censor board with ong the exhibitors | John Ab h no cuts last week, s for a release in Ta | but th | e film's dist | tributors | cleared by no takers |
| (A | Mr., was, found | , on | (B) | a, was foun | d, at | |
| (C) | the, was, found, | on z | (D) | a, being, fir | nd, at | |
| Answer | : (C) | | | | | |
| rul opt | | er Madras Café talk takers' is not the co | | | | |

- 5. A function f(x) is linear and has a value of 29 at x = -2 and 39 at x = 3. Find its value at x = 5.
 - (A) 59
- (B) 45
- (C) 43
- (D) 35

Answer: (C)

Exp:

f(x) = 2x + 33

Q. No. 6-10 carry Two Mark Each

6. The head of a newly formed government desires to appoint five of the six selected members P,Q,R,S,T and U to portfolios of Home, Power, Defense, Telecom and Finance. U does not want any portfolio if S gets one of the five. R wants either Home or Finance or no portfolio. Q says that if S gets either Power or Telecom, then she must get the other one. T insists on a portfolio if P gets one.

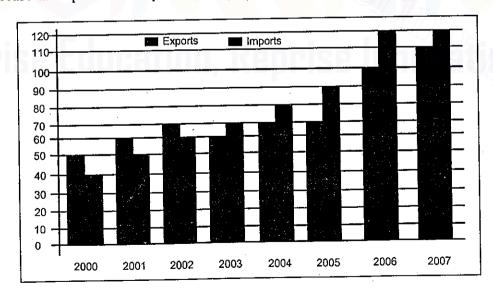
Which is the valid distribution of portfolio?

- (A) P-Home, Q-Power, R-Defense, S-Telecom, T-Finance
- (B) R-Home, S-Power, P-Defense, Q-Telecom, T-Finance
- (C) P-Home, Q-Power, T-Defense, S-Telecom, U-Finance
- (D) Q-Home, U-Power, T-Defense, R-Telecom, P-Finance

Answer: (B)

Exp: Since U does not want any portfolio, (C) and (D) are ruled out. R wants Home, or Finance or No portfolio, (A) is not valid. Hence option (B) is correct

7. The exports and imports (in crores of Rs.) of a country from the year 2000 to 2007 are given in the following bar chart. In which year is the combined percentage increase in imports and exports the highest?



Answer: 2006

Exp: Increase in exports in $2006 = \frac{100 - 70}{70} = 42.8 \%$

Increase in imports in $2006 = \frac{120 - 90}{90} = 33.3 \%$

which is more than any other year

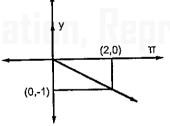
8. Most experts feel that in spite of possessing all the technical skills required to be a batsman of the highest order., he is unlikely to be so due to lack of requisite temperament. He was guilty of throwing away his wicket several times after working hard to lay a strong foundation. His critics pointed out that until he addressed to this problem, success at the highest level will continue to elude him.

Which of the statement (s) below is/are logically valid and can be inferred from the above passage?

- (i) He was already a successful batsman at the highest level
- (ii) He has to improve his temperament in order to become a great batsman
- (iii) He failed to make many of his good starts count
- (iv) Improving his technical skills will guarantee success
- (A) (iii) and (iv) (B) (ii) and (iii)
- (C) (i), (ii) and (iii) (D) (ii) only

Answer: (B)

9. Choose the most appropriate equation for the function drawn as a thick line, in the plot below.



$$(A) x = y-|y|$$

(B)
$$x = -(y-|y|)$$

$$(C) \quad x = y + |y|$$

(D)
$$x = -(y+|y|)$$

Answer: (B)

10. Alexander turned his attention towards India, since he had conquered Persia.

Which one of the statements below is logically valid and can be inferred from the above sentence?

- (A) Alexander would not have turned his attention towards India had he not conquered Persia.
- (B) Alexander was not ready to rest on his laurels, and wanted to march to India
- (C) Alexander was completely in control of his army and could command it to move towards India.
- (D) Since Alexander's kingdom extended to Indian borders after the conquest of Persia, he was keen to move further.

Answer: (A)

Section Name: Civil Engineering Q.No. 1-25 cary one Mark Each

- Consider the following statements for air-entrained concrete:
 - Air-entrainment reduces the water demand for a given level of workability
 - (ii) Use of air-entrained concrete is required in environments where cyclic freezing and thawing is expected.

Which of the following is TRUE?

- (A) Both (i) and (ii) are True
- (B) Both (i) and (ii) are False
- (C) (i) is True and (ii) is False
- (D) (i) is False and (ii) is True

Answer: (A)

- Which of the following statements is TRUE for the relation between discharge velocity and seepage velocity?
 - (A) Seepage velocity is always smaller than discharge velocity
 - (B) Seepage velocity can never be smaller than discharge velocity
 - (C) Seepage velocity is equal to the discharge velocity
- (D) No relation between seepage velocity and discharge velocity can be established Answer: (B)

Exp:

since $A_v < A$ $\therefore V_s > V$.

q = discharge per unit time

So, seepage velocity (Vs) can never by smaller than discharge velocity

3. The integral $\int_{x_1}^{x_2} x^2 dx$ with $x_2 > x_1 > 0$ is evaluated analytically as well as numerically using a single application of the trapezoidal rule. If I is the exact value of the integral obtained analytically and J is the approximate value obtained using the trapezoidal rule, which of the following statements is correct about their relationship?

(A) J > I

(B) J < I

(C) J = I

(D) Insufficient data to determine the relationship

Answer: (A)

Exp. We know that the approximated value of $\int_a^b f(x) dx$ obtained by trapezoidal rule is always greater than the analytical value.

J > I where J = approximate value

I = analytical value

4. A circular pipe has a diameter of 1m, bed slope of 1 in 1000, and Manning's roughness coefficient equal to 0.01. It may be treated as an open channel flow when it is flowing just full, i.e., the water level just touches the crest. The discharge in this condition is denoted by $Q_{\rm full}$. Similarly, the discharge when the pipe is flowing halffull, i.e., with a flow depth of 0.5m, is denoted by $Q_{\rm half}$. The ratio $Q_{\rm full}/Q_{\rm half}$ is:

(A) 1

(B) $\sqrt{2}$

(C) 2

(D) 4

Answer: (C)

For pipe running full,

$$R = \frac{A}{p} = \frac{\pi}{4} D^2 = \frac{D}{4}$$

For pipe running half,

$$R = A/P = \frac{\pi}{8} D^2 / (\pi D/2) = D/4$$

Exp:

$$Q = \frac{1}{n} \cdot AR^{\frac{2}{3}} \frac{1}{S^{\frac{1}{2}}} \Rightarrow Q_{\text{full}} = \frac{1}{n} \cdot \frac{\pi}{4} \cdot D^{2} \cdot \left(\frac{D^{2}}{4}\right)^{\frac{2}{3}} \frac{1}{S^{\frac{1}{2}}} \dots Manning's formula$$

$$Q_{half} = \frac{1}{n} \cdot \frac{\pi D^2}{8} \cdot \left(\frac{D}{4}\right)^{\frac{2}{3}} S^{\frac{1}{2}}$$

$$\frac{Q_{\text{full}}}{Q_{\text{half}}} = 2$$

- 5. Which of the following statements is NOT correct?
 - (A) Loose sand exhibits contractive behavior upon shearing
 - (B) Dense sand when sheared under undrained condition, may lead to generation of negative pore pressure
 - (C) Black cotton soil exhibits expansive behavior
 - (D) Liquefaction is the phenomenon where cohesionless soil near the downstream side of dams or sheet-piles loses its shear strength due to high upward hydraulic gradient

Answer: (D)

Exp: Liquefaction is due to cyclic loads, not due to high hydraulic gradient

Liquefaction is the phenomenon in which loose, saturated, granular soil, under

dynamic load loses all its shear strength and behaves like liquid.

The statement (D) indicates boiling of sand (quick sand).

6. A fine-grained soil has 60% (by weight) silt content. The soil behaves as semi-solid when water content is between 15% and 28%. The soil behaves fluid-like when the water content is more than 40%. The 'Activity' of the soil is

(A) 3.33

- (B) 0.42
- (C) 0.30
- (D) 0.20

Answer: (C)

Exp: $I_P = W_L - W_P = 40_P \ 28 = 12\%$ $Activity = \frac{I_P}{F} = \frac{12}{100 - 60} = 0.3$

F = clay fraction = 100 - Silt content = 100 - 60

- 7. For steady incompressible flow through a closed-conduit of uniform cross-section, the direction of flow will always be:
 - (A) from higher to lower elevation
- (B) from higher to lower pressure
- (C) from higher to lower velocity
- (D) from higher to lower piezometric head

Answer: (B)

8. Two triangular wedges are glued together as shown in the following figure. The stress acting normal to the interface, n is _____ MPa.

Answer: 0

Exp:
$$\sigma_{\text{n}} = \frac{\sigma_{\text{x}} + \sigma_{\text{y}}}{2} + \frac{\sigma_{\text{x}} - \sigma_{\text{y}}}{2} \cos 2\theta$$
 $\sigma_{\text{x}} = 100 \text{ MPa (T)}$

$$= \frac{100 - 100}{2} + \frac{100 - (-100)}{2} \cos (-90^{\circ}) = 0 \quad \sigma_{\text{y}} = -100 \text{ MPa (C) } \theta = 45^{\circ} \text{ (anticlockwise)}$$

$$100 \text{MPa} \longrightarrow 100 \text{MPa}$$

- 9. In a closed loop traverse of 1 km total length, the closing errors in departure and latitude are 0.3 m and 0.4 m, respectively. The relative precision of this traverse will be;
 - (A) 1:5000
- (B) 1:4000
- (C) 1:3000
- (D) 1:2000

Answer: (D)

Exp:
$$e = \sqrt{\ell^2 + d^2} = \sqrt{(0.3)^5 + (0.4)^2} = 0.5 \text{ m}$$

Relative precision =
$$\frac{0.5}{1000}$$
 = 1:2000

10. Solid waste generated from an industry contains only two components, X and Y as shown in the table below

Component

Composition

Density

%weight kg m³

X

c₁

 ρ_1

Y

 c_2

 ρ_2

Assuming $(c_1 + c_2) = 100$, the composite density of the solid waste (ρ) is given by:

(A)
$$\frac{100}{\left(\frac{c_1}{\rho_1} + \frac{c_2}{\rho_2}\right)}$$
 (B) $100\left(\frac{\rho_1}{c_1} + \frac{\rho_2}{c_2}\right)$ (C) $100 (c_1\rho_1 + c_2\rho_2)$ (D) N77° 50'E

Answer: (A)

Exp: Let density of sludge is ρ

$$\frac{c_1 + c_2}{\rho} = \frac{c_1}{\rho_1} + \frac{c_2}{\rho_2}$$

$$\Rightarrow \rho = \frac{100}{\frac{c_1}{\rho_1} + \frac{c_2}{\rho_2}}$$

11. The two columns below show some parameters and their possible values.

| | 1 | | |
|---------------------------|----------------------|--|--|
| Parameter | Value | | |
| P-Gross Command Area | I-100 hectares/cumec | | |
| Q-Permanent Wilting Point | II-6 °C | | |
| R-Duty of canal water | III-1000 hectares | | |
| S-Delta of wheat | IV-1000 cm | | |
| | V-40 cm | | |
| | VI-0.12 | | |

Which of the following options matches the parameters and the values correctly?

- (A) P-I, Q-II, R-III, S-IV
- (B) P-III, Q-VI, R-I, S-V
- (C) P-I, Q-V, R-VI, S-II
- (D) P-III, Q-II, R-V, S-IV

Answer: (B)

Exp:

P-Gross Command Area=1000 ha

Q-Permanent Wilting Point=0.12

R-Duty of canal water=100 ha/cumec

S-Delta of wheat=40 cm

- 12. In an unconsolidated undrained triaxial test, it is observed that an increase in cell pressure from 150 kPa to 250 kPa leads to a pore pressure increase of 80 kPa. It is further observed that, an increase of 50 kPa in deviatoric stress results in an increase of 25 kPa in the pore pressure. The value of Skempton's pore pressure parameter B is;
 - (A) 0.5
- (B) 0.625
- (C) 0.8
- (D) 1.0

Answer: (C)

Exp:
$$B = \frac{\Delta U}{\Delta \sigma_3} = \frac{80}{100} = 0.8$$

- 13. Which of the following statements is TRUE for degree of disturbance of collected soil sample?
 - (A) Thinner the sampler wall, lower the degree of disturbance of collected soil sample
 - (B) Thicker the sampler wall, lower the degree of disturbance of collected soil sample
 - (C) Thickness of the sampler wall and the degree of disturbance of collected soil sample are unrelated
 - (D) The degree of disturbance of collected soil sample is proportional to the inner diameter of sampling tube

Answer: (A)

Exp: As thickness of sampler increases, disturbance increases

- 14. Which of the following statements is FALSE?
 - (A) Plumb line is along the direction of gravity
 - (B) Mean Sea Level (MSL) is used as a reference surface for establishing the horizontal control
 - (C) Mean Sea Level (MSL) is a simplification of the Geoid
 - (D) Geoid is an equi-potential surface of gravity

Answer: (B)

Exp: Mean Sea Level (MSL) is used as a reference surface for establishing the vertical control and not horizontal control

15. For what value of p the following set of equations will have no solution?

$$2x + 3y = 5$$
$$3x + py = 10$$

Answer: 4.5

Exp: Given
$$2x + 3y = 5$$

 $3x + py = 10$

$$\Rightarrow \begin{bmatrix} 2 & 3 \\ 3 & p \end{bmatrix} \begin{bmatrix} x \\ y \end{bmatrix} = \begin{bmatrix} 5 \\ 10 \end{bmatrix}$$

$$AX = B$$

Augmented matrix [A / B] =
$$\begin{bmatrix} 2 & 3 & 5 \\ 3 & P & 10 \end{bmatrix}$$

Objective Civil Eng. \ 2016 \ 72

$$R_2 \rightarrow 2R_2 - 3R_1 \begin{bmatrix} 2 & 3 & 5 \\ 0 & 2p - 9 & 5 \end{bmatrix}$$

system will have no solution if $\rho(A / B) \neq \rho(A)$

$$\Rightarrow 2p - 9 = 0$$

$$\Rightarrow$$
 p = $\frac{9}{2}$ = 4.5

16. In a two-dimensional steady flow field, in a certain region of the x-y plane, the velocity component in the x-direction is given by $v_x = x^2$ and the density varies as

 $\rho = \frac{1}{2}$. Which of the following is a valid expression for the velocity component in the y-direction, v_v?

(A)
$$v_y = -x / y$$
 (B) $v_y = x / y$ (C) $v_y = -xy$

(B)
$$v_v = x / y$$

(C)
$$v_y = -xy$$

(D)
$$v_y = xy$$

Answer: (C)

Continuity equation Exp:

$$\frac{\partial}{\partial x} (\rho Y) + \frac{\partial}{\partial y} (\rho V) = 0$$

$$\Rightarrow \frac{\partial}{\partial x}(x) + \frac{\partial}{\partial y}\left(\frac{1}{x}, V\right) = 0$$

$$\Rightarrow 1 + \frac{\partial}{\partial y} \left(\frac{V}{x} \right) = 0 \Rightarrow \frac{\partial}{\partial y} \left(\frac{V}{x} \right) = -1$$

$$\Rightarrow$$
V = -xy

- 17. Workability of concrete can be measured using slump, compaction factor and Vebe time. Consider the following statements for workability of concrete:
 - As the slump increases, the Vebe time increases
 - (ii) As the slum increases, the compaction factor increases

Which of the following is TRUE?

- (A) Both (i) and (ii) are True
- (B) Both (i) and (ii) are False
- (C) (i) is True and (ii) is False
- (D) (i) is False and (ii) is True

Answer: (D)

As the slump increases, the Vebe time decreases Exp:

18. Consider the following probability mass function (p.m.f) of a random variable X:

$$p(x,q) = \begin{cases} q & \text{if} & X = 0\\ 1 - q & \text{if} & X = 1\\ 0 & \text{otherwise} \end{cases}$$

If q = 0.4, the variance of X is _____.

Answer: 0.24

Exp:
$$p(x, q) = q$$
 if $X = 0$
 $= 1 - q$ if $X = 1$
 0 otherwise

given
$$q = 0.4$$

$$\Rightarrow p(x, q) = 0.4 \text{ if } X = 0$$

$$= 0.6 \text{ if } X = 1$$

$$= 0 \text{ otherwise}$$

$$\Rightarrow \begin{array}{c|c|c} X & 0 & 1 \\ \hline p(X=x) & 0.4 & 0.6 \end{array}$$

Required value=V (X) =E (X)² - {E (X)}²
E (X) =
$$\sum x_i p_i = 0 \times 0.4 + 1 \times 0.6 = 0.6$$

E (X)² = $\sum x_i^2 p_i = 0^2 \times 0.4 + 1^2 \times 0.6 = 0.6$
V (X) = 0.6 - (0.6)²
= 0.6 - 0.36
= 0.24

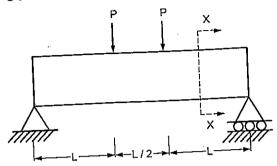
- 19. Which of the following statements CANNOT be used to describe free flow speed (u_f) of a traffic stream?
 - (A) uf is the speed when flow is negligible
 - (B) u_f is the speed when density is negligible
 - (C) uf is affected by geometry and surface conditions of the road
 - (D) uf is the speed at which flow is maximum and density is optimum

Answer: (D)

Exp: Free flow speed → speed when flow is negligible

- → speed when density is negligible
- → affected by Geometry, deriver's perception, roadway condition etc.

20. Consider the singly reinforced beam shown in the figure below:



At cross-section XX, which of the following statement is TRUE at the limit state?

- (A) The variation of stress is linear and that of strain is non-linear
- (B) The variation of strain is linear and that of stress is no-linear
- (C) The variation of both stress and strain is linear
- (D) The variation of both stress and strain is non-linear

Answer: (B)

Exp:

0.0035 Αt

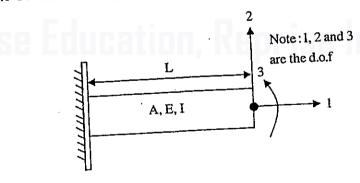




Strain variation

Stress variation

21. For the beam shown below, the stiffness coefficient K22 can be written as



Answer: (B)

Objective Civil Engineering

$$\frac{\text{sw} + \text{cover}}{\text{sw}} = \frac{\text{sw}}{\text{sw}} + \frac{\text{cover}}{\text{sw}} = 1.5$$

$$\Rightarrow \frac{\text{cover}}{\text{sw}} = 0.5$$

$$\Rightarrow \text{cover } 0.5 \times 1.46 \times 10^7 = 0.73 \times 10^7 \text{ m}^3$$
Total volume = $(1.46 + 0.73 \times 10^7) = 21.9 \times 10^6 \text{ m}^3$
= 21.9 million m³

- 55. The bearings of two inaccessible stations, S_1 (Easting 500 m, Northing 500 m) and S_2 (Easting 600 m, Northing 450 m) from a station S3 were observed as 225° and 153° 26' respectively. The independent Easting (in m) of station S_3 is:
 - (A) 450.000
- (B) 570.710
- (C) 550.000
- (D) 650.000

Answer: (C)

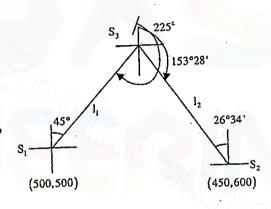
624

Let $s_1 s_3 = l_1$, $s_2 s_3 = l_2$ Exp: Northing of $S_3 = 500 + 1_1 \cos 45^\circ$ $= 450 + l_2 \cos 26^{\circ} 34^{\circ}$ $\Rightarrow 1_1 \cos 45^\circ - 1_2 \cos 26^\circ 34' = -50$

Easting of S₃

 $500 + l_1 \sin 45^\circ = 600 - l_2 \sin 26^\circ 34^\circ$ $l_1 \sin 45^\circ + l_2 \sin 26^\circ 34' = 100$ $\Rightarrow 1_1 = 70.71, 1_2 = 111.80$ Easting of $S_3 = 500 + 70.71 \times \sin 45$

= 549.99 m = 550 m





53. In a wastewater treatment plant, primary sedimentation tank (PST) designed at an overflow rate of 32.5 m₃/day/m² is 32.5 m long, 80 m wide and liquid depth of 2.25 m. If the length of the weir is 75 m, the weir loading rate (in m³/day/m) is

Answer: 112.67

Exp:

$$Q = 32.5m^3 / d / m^2$$

$$B = 8m$$

$$D = 2.25m$$

$$V_0 = \frac{Q}{BL}$$

$$Q = V_0 BL$$

$$= 32.5 \times 32.5 \times 8$$

$$= 8450 \text{m}^3 / \text{d}$$

Weir length =75m.

$$q = \frac{8450}{75} = 112.67 \text{ m}^3 / \text{d} / \text{m}$$

54. A landfill is to be designed to serve a population of 200000 for a period of 25 years. The solid waste (SW) generation is 2 kg/person/day. The density of the un-compacted SW is 100 kg/m3 and a compaction ratio of 4 is suggested. The ratio of compacted fill (i.e. SW + cover) to compacted SW is 1.5. The landfill volume (in million m3) required is _____.

Answer: 21.9

Exp:

Total solid waste generated =
$$2kg \times 2 \times 10^5$$

$$= 400000 \text{ kg/day}$$

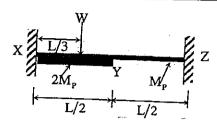
For 25 years =
$$400000 \times 365 \times 25$$

$$= 3.65 \times 10^9 \text{ kg}$$

Compaction ratio = $0.4 = \frac{\text{volume after compaction}}{\text{volume before compaction}}$

$$V = \frac{3.65 \times 10^9}{100} = 3.65 \times 10^7 \text{ m}^3$$

$$V' = 0.4 \times 3.65 \times 10^7 = 1.46 \times 10^7 \text{ m}^3$$



- (A) $16.5 \text{ M}_{p}/\text{L}$
- (B) $15.5 \text{ M}_{p}/\text{L}$
- (C) $15.0 \text{ M}_p/L$
- (D) $16.0 \text{ M}_{p}/\text{L}$

Answer: (C)

Plastic hinges formed =3 Exp:

$$\alpha = \frac{\theta}{2}$$

$$\theta = 2\alpha$$

$$\theta = 2\alpha$$

$$\theta = \frac{\Delta}{L/3}$$

$$\Delta = \frac{L}{3}\theta$$

$$\Delta = \frac{\alpha 2L}{3}$$

$$\theta = 2\alpha$$

$$2M_p\theta + 2M_p\theta + 2M_p\alpha + M_p\alpha = W\Delta$$

$$2M_p\theta + 2M_p\theta + M_p\theta + M_p \frac{\theta}{2} = W \times \frac{L}{3}\theta$$

$$5.5M_p = \frac{WL}{3} \Rightarrow W = \frac{16.5}{L}M_p$$

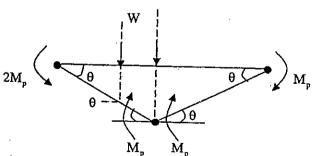
(2)

$$2M_{p}\theta + M_{p} \theta + M_{p} \Delta + M_{p}\theta = W \times \Delta$$

$$5M_p\theta = \frac{WL}{3}\theta$$

$$15 \frac{M_p}{L} = W$$

Lowest is collapse load $15 \frac{M_p}{I}$



621

Exp:

$$G_{t} = \frac{w_{1} + w_{2} + w_{3} + w_{4}}{\frac{w_{1}}{G_{1}} + \frac{w_{2}}{G_{2}} + \dots}$$

$$=\frac{100}{\frac{55}{2.6} + \frac{35.8}{2.7} + \frac{3.7}{2.65} + \frac{5.5}{1.01}} = 2.424$$

Eff 'G' of aggregates G (fine+coarse)

$$G = \frac{(55 \times 2.6) + (35.8 \times 2.7)}{55 + 35.8} = 2.64$$

51. In a system two connected rigid bars AC and BC are of identical length, L with pin supports at A and B. The bars are interconnected at C by a frictionless hinge. The rotation of the hinge is restrained by a rotational spring of stiffness, k. The system initially assumes a straight line configuration, ACB. Assuming both the bars as weightless, the rotation at supports, A and B, due to a transverse load, P applied at C is

(A)
$$\frac{PL}{4k}$$
 (B) $\frac{PL}{2k}$

(B)
$$\frac{PL}{2k}$$

$$(C) \frac{P}{4k}$$

(D)
$$\frac{Pk}{4L}$$

Answer: (A)

Exp:

External work done

$$=\frac{1}{2}\times\rho\times L.\theta....(i)$$

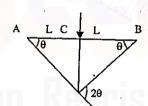
Strain energy stored in spring

$$= \frac{1}{2} \times k \times (2\theta) \times (2\theta)$$
$$= 2k.\theta^2 \dots (ii)$$
$$(i) = (ii)$$

$$\Rightarrow \frac{1}{2} PL\theta = 2k.\theta^2$$

$$\Rightarrow \theta = \frac{PL}{4k}$$

52. A fixed end beam is subjected to a load, W at 1/3rd span from the left support as shown in the figure. The collapse load of the beam is



Total no. of filters =
$$\frac{720}{60}$$
 = 12 filters.

2 out of services, total filters =10.

S.A of filters $2 = 60 \times 10 = 600 \text{ m}^2$.

The loading rate =
$$\frac{86400}{600}$$
 = 144 m³ / day/ m².

49. A pile of diameter 0.4 m is fully embedded in a clay stratum having 5 layers, each 5 m thick as shown in the figure below. Assume a constant unit weight of soil as 18 kN/m3 for all the layers.

Using λ -method ($\lambda = 0.15$ for 25 m embedment length) and neglecting the end bearing component, the ultimate pile capacity (in kN) is _____.

| | G | | | | S |
|--|-----|------|-------|-----------|---------------|
| | M | 5m∫ | 25 m | c=40 kPa | / <u>////</u> |
| γ =18kN/m ³ for all layers | YQ | . 5m | ,L= | c=50 kPa | |
| | | 5m | 0.4m | c=60 kPa | |
| | "-" | 5m | Dia≡ | c=70 kPa | |
| | | 5m | Pile] | c=80 kPa | 72 |

Answer: 1060.29

Exp: Ultimate Bearing capacity, Qu

=
$$\lambda (\sigma_{\text{v.avg}} + 2c_{\text{u}})A_{\text{s}}$$

= 0.15 [18 × 12.5 + 2C_u] × [π × 0.4 × 25]
• w = 0

$$\Rightarrow q_u = 1060.29 \text{ kN}$$

- 50. In Marshall method of mix design, the coarse aggregate, fine aggregate, fines and bitumen having respective values of specific gravity 2.60, 2.70, 2.65 and 1.01, are mixed in the relative proportions (% by weight) of 55.0, 35.8, 3.7 and 5.5 respectively. The theoretical specific gravity of the mix and the effective specific gravity of the aggregates in the mix respectively are
 - (A) 2.42 and 2.63 (B) 2.42 and 2.78 (C) 2.42 and 2.93

(D) 2.64 and 2.78

Answer: (A)

46. A field channel has cultivable commanded area of 2000 hectares. The intensities of irrigation for gram and wheat are 30% and 50% respectively. Gram has a kor period of 18 days, kor depth of 12 cm, while wheat has a kor period of 18 days and a kor depth of 15 cm. The discharge (in m3/s) required in the field channel to supply water to the commanded area during the kor period is _____.

Answer: 1.427

Exp: Rafi crops Gran and wheat

$$Q_1 = \frac{A_1}{D_1} = \frac{200 \times 0.3}{8.64 \times \frac{18}{0.12}} = 0.463 \text{ m}^3/\text{s} \text{ ; Duty} = 8.64 \frac{\text{B}}{\Delta}$$

$$Q_2 = \frac{A^2}{D_2} = \frac{2000 \times 0.5}{8.64 \times \frac{18}{0.15}} = 0.964 \text{ m}^3/\text{s}$$

$$\therefore Q_1 + Q_2$$
 is required = 0.964 + 0.463 = 1.427 m³ / s

47. The relation between speed u (in km/h) and density k (number of vehicles/km) for a traffic stream on a road is u = 70 - 0.7k. The capacity on this road is _____ vph (vehicles/hour).

Answer: 175

Exp: u = 70 - 7k

Capacity $u \times k$, q = uk

$$q = (70 - 0.7k) k$$

$$\frac{dq}{dk} = 70 - 0.7 \times 2k = 0 \implies k = 50 \text{ V / km}.$$

$$q = (70 - 0.7 \times 50) \times 50 = 175 \text{ V / hr.}$$

48. A water treatment plant of capacity, 1 m³/s has filter boxes of dimensions 6 m × 10 m. Loading rate to the filters is 120 m³/day/m². When two of the filters are out of service for back washing, the loading rate (in m³/day/m²) is ______

Answer: 144

Exp: Total water filters = $24 \times 3600 \times 1 = 86400 \text{ m}^3 / \text{day}$.

S.A. =
$$\frac{86400}{120}$$
 = 720 m²

Area of one filter $2 = 6 \times 10 = 60 \text{ m}^2$.

$$y_{p} = \frac{1}{D^{2}} (-12x^{2} + 24x - 20)$$

$$= -12\frac{x^{4}}{12} + 24 \cdot \frac{x^{3}}{6} - 20 \cdot \frac{x^{2}}{2!}$$

$$= -x^{4} + 4x^{3} - 10x^{2}$$

$$y = c_{1} + c_{2}x + 10x^{2} + 4x^{3} - x^{4}$$

$$y(0) = 5 \Rightarrow c1 = 5$$

$$y(2) = 21 \Rightarrow 21 = 5 + 2c_{2} + 40 + 32 - 16$$

$$21 = 2c_{2} + 61$$

$$c_{2} = -20$$

$$y = 5 - 20x + 10x^{2} + 4x^{3} - x^{4}$$

$$y(1) = 5 - 20 + 10 + 4 - 1$$

$$= -2$$

45. For step-size $\Delta x = 0.4$, the value of following int egral usin g Simpson 's 1 3 rule is

$$\int_{0}^{0.8} (0.2 + 25x + 200x^{2} + 675x^{3} + 900x^{4} + 400x^{5}) dx$$

Answer: -3.8293

Exp: Given
$$h = \Delta x = 0.4$$

$$f(x) = 0.2 + 25x \ 200x^2 \ 675x^3$$

$$-900x^4 + 400x^5$$

$$x_0 = 0 x_n \ 0.8 \Rightarrow n = \frac{0.8 - 0}{0.4} = 0$$

$$y = f(x)$$
 0.2 24.456 - 126.744

By Simpson's
$$\frac{1}{3}$$
 Rule

$$\int_{0}^{0.8} f(x) dx = \frac{0.4}{3} [(0.2 - 126.744) + 4(24.456)] = -3.8293$$

Ratio for p = 2

 \Rightarrow Characteristic equation λ_2 -4 λ +3 = 0 (by substituting p=2)

 $\Rightarrow \lambda=1,3$

If we take
$$p = \frac{14}{3}$$
 then $A = \begin{bmatrix} 2 & 1 \\ 1 & \frac{14}{3} \end{bmatrix}$

$$\Rightarrow \lambda^2 - \left(2 + \frac{14}{3}\right)\lambda + \left(\frac{28}{3} - 1\right) = 0$$

$$\Rightarrow \lambda^2 - \frac{20}{3}\lambda + \frac{25}{3} = 0$$

$$\Rightarrow 3\lambda^2 - 20\lambda + 25 = 0$$

$$\lambda = 5, \frac{5}{3}$$

Eigen values $5, \frac{5}{3}$ are in ratio 3:1

$$\therefore p = \frac{14}{3}$$

44. Consider the following second order linear differential equation

$$\frac{d^2y}{dx^2} = -12x^2 + 24x - 20$$

The boundary conditions are at x = 0, y = 5 and at x = 2, y = 21

The value of at x = 1 is _____

Answer: -2

Exp: Given

$$\frac{d^2y}{dx^2} = -12x^2 + 24x - 20$$

$$y(0) = 5 y(2) = 21$$

$$y(1) = ?$$

Auxillary equation $m^2 = 0$

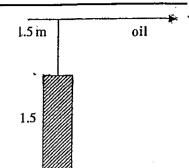
$$m = 0, 0$$

$$y_c = (c_1 + c_2 x) e^{0x} = c_1 + c_2 x$$

Objective Civil Eng. \ 2016 \ 78

Objective Civil Engineering

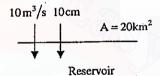
Exp: Force on gate $=\frac{1}{2} \times 1.5 \times 2 \times G \gamma_{\omega} \left(1.5 + \frac{2}{3} \times 1.5\right)$ = $0.8 \times 9810 \times 2.5 \times 1.5$ = 29.43 kN

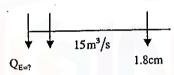


- 42. The average surface area of a reservoir in the month of June is 20 km². In the same month, the average rate of inflow is 10 m3/s, outflow rate is 15 m³/s, monthly rainfall is 10 cm, monthly seepage loss is 1.8 cm and the storage change is 16 million m³. The evaporation (in cm) in that month is
 - (A) 46.8
- (B) 136.0
- (C) 13.6
- (D) 23.4

Answer: (B)

Exp:





: changeinstorage inflow-outflow

$$\Delta S = Q_1 + Q_R - Q_0 - Q_S - Q_E$$

$$\Rightarrow 16 \times 10^6 = (10 \times 86400 \times 30) + (0.1 \times 20 \times 10^6)$$

$$- (15 \times 86400 \times 30) - (1.8 \times 10^{-2} \times 20 \times 10)$$

$$\Rightarrow$$
 Q_E = $\frac{27320000}{20 \times 10^6}$ = 1.366m 136.6cm

- 43. The two Eigen values of the matrix have a ratio of 3:1 for p=2. What is another value of p for which the Eigen values have the same ratio of 3:1?
 - (A) -2
- (B) 1
- (C) 7/3
- (D) 14/3

Answer: (D)

Exp: Let $A = \begin{bmatrix} 2 & 1 \\ 1 & p \end{bmatrix}$

Given that two eigen values of A are in 3:1

$$\phi = \sin^{-1} \cos 7 \ 30^{\circ}$$

$$C \cos 30 = 10 \sqrt{3}$$

$$C = 20$$

40. The velocity components of a two dimensional plane motion of a fluid are

$$u = \frac{y^3}{3} + 2x - x^2$$
 y and $v = xy^2 - 2y - \frac{x^3}{3}$

The correct statement is:

- (A) Fluid is incompressible and flow is irrotational
- (B) Fluid is incompressible and flow is rotational
- (C) Fluid is compressible and flow is irrotational
- (D) Fluid is compressible and flow is rotational

Answer: (A)

Exp:

For incompressible flow $\frac{\partial u}{\partial x} + \frac{\partial v}{\partial y} = 0$

...(i)

For irrotational flow.
$$\frac{1}{2} \left(\frac{\partial \mathbf{u}}{\partial \mathbf{x}} + \frac{\partial \mathbf{v}}{\partial \mathbf{y}} \right) = 0$$

(1)
$$\frac{\partial}{\partial x} \left(\frac{y^3}{3} + 2x - x^2 y \right) + \frac{\partial}{\partial y} \left(xy^2 - 2y - \frac{x^3}{3} \right) = 0$$

$$2 - 2xy + 2xy - 2 = 0$$

(2)
$$\frac{\partial}{\partial x} \left(xy^2 - 2y - \frac{x^3}{3} \right) - \frac{\partial}{\partial y} \left(\frac{y^3}{3} + 2x - x^2 y \right)$$

$$y^2 - x^2 - y^2 + x^2 = 0$$

41. A triangular gate with a base width of 2 m and a height of 1.5 m lies in a vertical plane. The top vertex of the gate is 1.5 m below the surface of a tank which contains oil of specific gravity 0.8. Considering the density of water and acceleration due to gravity to be 1000 kg/m3 and 9.81 m/s², respectively, the hydrostatic force (in kN) exerted by the oil on the gate is

Answer: 29.43

Objective Civil Engineering

614

Exp:
$$\Delta H = \frac{C_c}{1 + e_0} H_0 \log_{10} \left(\frac{\sigma_0 + \Delta \sigma}{\sigma_0} \right).$$

$$= \frac{0.6}{1 + 1.3} \times 4 \log_{10} \left(\frac{\sigma_0 + \sigma_0}{\sigma_0} \right)$$

$$= 0.314 \text{ m}$$

$$\Delta_{H} = 314 \text{mm}$$

37. According to the concept of Limit State Design as per IS456: 2000, the probability of failure of a structure is ______.

Answer: 0.097

38. Two pegs A and B were fixed on opposite banks of a 50 m wide river. The level was set up at A and the staff readings on Pegs A and B were observed as 1.350 m and 1.550 m, respectively. Thereafter the instrument was shifted and set up at B. The staff readings on Pegs B and A were observed as 0.750 m and 0.550 m, respectively. If the R.L. of Peg A is 100.200 m, the R.L. (in m) of Peg B is _____

Answer: 100

Exp: Reciprocal leveling

$$\Delta_{h} = \frac{\left(b_{1} - a_{1}\right) + \left(b_{2} - a_{2}\right)}{2} = \frac{\left(1.55 - 1.35\right) + \left(10.75 - 0.55\right)}{2}$$

 $\Delta h = 0.20$

$$RL ext{ of } B = RL ext{ of } A - 0.20 = 100m$$

From the reading we can see A is at higher level than B.

- 39. Stress path equation for tri-axial test upon application of deviatoric stress is, $q = 10\sqrt{3} + 0.5p$. The respective values of cohesion, c (in kPa) and angle of internal friction, are:
 - (A) 20 and 20°
- (B) 20 and 30°
- (C) 30 and 30°
- (D) 30 and 20°

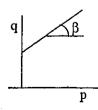
Answer: (B)

Exp: Stress path equations

$$\frac{\sigma_1 - \sigma_3}{2} = C\cos\phi + \frac{\sigma_1 + \sigma_3}{2}\sin\phi$$

$$p = 10\sqrt{3} + 0.5 p$$

$$C\cos \phi = 10 \sqrt{3}$$



34. A 588 cm3 volume of moist sand weighs 1010 gm. Its dry weight is 918 gm and specific gravity of solids, G is 2.67. Assuming density of water as 1 gm/cm3, the void ratio is ______.

Answer: 0.71

Exp: e = ?

$$\gamma_{\text{moist sand}} = \frac{1010}{588} = 1.717 \text{ g/cc}$$

$$\gamma_{\text{d}} = \frac{G\gamma_{\omega}}{1 + \text{e}}$$

$$\gamma_{\text{d}} = \frac{918}{588} = 1.561 \text{ g/cc}$$

$$1.561 = \frac{2.67 \times 1}{1 + \text{e}}$$

$$\text{e} = 0.71$$

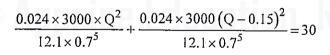
35. A pipe of 0.7 m diameter has a length of 6 km and connects two reservoirs A and B. The water level in reservoir A is at an elevation 30 m above the water level in reservoir B. Halfway along the pipe line, there is a branch through which water can be supplied to a third reservoir C. The friction factor of the pipe is 0.024. The quantity of water discharged into reservoir C is 0.15 m³/s. Considering the acceleration due to gravity as 9.81 m/s² and neglecting minor losses, the discharge (in m³/s) into the reservoir B is

Answer: 0.5716

Exp:

$$h_f = \frac{f1Q^2}{12.1d^5}$$

 $h_{f_1} + h_{f_2} = 30m \rightarrow in parallel$



$$Q = 0.7216 \text{ m}^3/\text{s}$$

$$Q_B = Q - 0.15 = 0.5716 \text{ m}^3/\text{s}$$

36. A 4 m thick layer of normally consolidated clay has an average void ratio of 1.30. Its compression index is 0.6 and coefficient of consolidation is 1 m²/yr. If the increase in vertical pressure due to foundation load on the clay layer is equal to the existing effective overburden pressure, the change in the thickness of the clay layer is _____ mm.

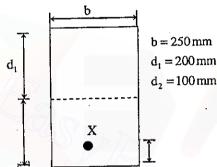
Answer: 314

$$= \frac{1}{4} \left[4 \cdot \frac{8}{3} - \frac{32}{5} \right]$$

$$= \frac{32}{4} \left[\frac{1}{3} - \frac{1}{5} \right]$$

$$= 8 \left[\frac{2}{15} \right] = \frac{16}{15} = 1.0667$$

33. In a pre-stressed concrete beam section shown in the figure, the net loss is 10% and the final prestressing force applied at X is 750 kN. The initial fiber stresses (in N/mm²) at the top and bottom of the beam were:



- (A) 4.166 and 20.833
- (C) 4.166 and -20.833
- (B) -4.166 and -20.833
- (D) -4.166 and 20.833

Answer: (D)

$$Loss = 10\%$$

Final force =
$$750 \text{ kN}$$

Initial force =
$$\frac{750}{0.9}$$
 = 833.33 kN

$$Z = \frac{bd^2}{6}$$

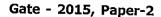
Top & Bottom stress =
$$\frac{P}{A} \pm m/z$$

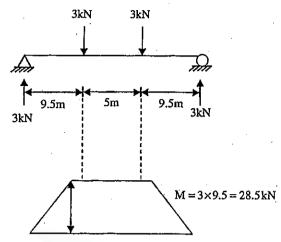
$$= \frac{833.33}{250 \times 400} \times 10^3 \pm \frac{833.33 \times 10^3 \times 100 \times 6}{250 \times 400^2}$$

$$= 8.33 \pm 12.5$$

$$Top = -4.166 (T)$$

Bottom =
$$20.833$$
 (C)





$$\sigma = \frac{M}{I}$$
. $y = \frac{M}{Z} = \frac{28.5 \times 10^6}{16.2 \times 10^3} = 1759.2 \text{ GPa}$

32. For probability density function of a random variable, x is

$$f(x) = \frac{x}{4}(4-x^2)$$
 for $0 \le x \le 2$

= 0 otherwise

The mean μ_z of the random variable is _____

Answer: 1.0667

Exp:

$$f(x) = \frac{x}{4}(4-x^2) \qquad 0 \le x \le 2$$

mean = μ_z = E (x)

$$= \int_{0}^{2} xf(x) dx$$

$$= \int_{0}^{2} x \left(\frac{x}{4}\right) (4 - x^{2}) dx$$

$$= \frac{1}{4} \int_{0}^{0} (4x^{2} - x^{2}) dx$$

$$= \frac{1}{4} \left[\frac{4x^{3}}{3} - \frac{x^{5}}{5}\right]_{0}^{2}$$

- 30. A 6 m high retaining wall having a smooth vertical back face retains a layered horizontal backfill. Top 3 m thick layer of the backfill is sand having an angle of internal friction, φ = 30° while the bottom layer is 3 m thick clay with cohesion, c = 20 kPa. Assume unit weight for both sand and clay as 18 kN/m³. The totalactive earth pressure per unit length of the wall (in kN/m) is:
 - (A) 150
- (B) 216
- (C) 156
- (D) 196

Answer: (A)

Exp:

A
$$\phi = 30$$
 $ka1 = 3$ $ka2 = 1$ $ka2 = 1$ $ka2 = 1$ $ka3 = 3$ $ka4$

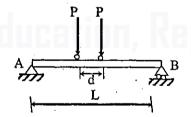
above B =
$$k_{a1} 18H = \frac{1}{3} \times 18 \times 3 = 18 \text{ kN/m}^2$$

below B =
$$ka_2 18H - 2c \sqrt{ka_2}$$

= $1 \times 18 \times 3 - 2 \times 20 \sqrt{1} = 14 \text{ kN /m}^2$
Pc = $Ka_2 .\gamma z - 2c \sqrt{ka_2} = 18 \times 6 - 2 \times 20$
= 68 kN/m^2

$$P_a = \frac{1}{2} \times 18 \times 3 \frac{1}{2} (14 + 68) \times 3 = 150 \text{kN/m}^2$$

31. A simply supported beam AB of span, L = 24 m is subjected to two wheel loads acting at a distance, d = 5 m apart as shown in the figure below. Each wheel transmits a load, P = 3 kN and may occupy any position along the beam. If the beam is an I-section having section modulus, S = 16.2 cm3, the maximum bending stress (in GPa) due to the wheel loads is ______



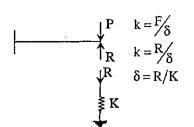
Answer: 1759.2

Exp: Maximum bending stress occurs at the point of maximum bending moment.

Maximum B.M. will occur under one of the point load such that resultant of the load system and point load under consideration is equidistant from the centre.

609

Exp:



δ for cantilever

$$\delta \text{ for cantilever}$$

$$\delta = \frac{F}{\delta}$$

$$\delta = \frac{(P-R)L^3}{3EI} = R/K$$

$$\delta = R/K$$

$$\frac{(50-R)200^3}{3 \times 200 \times 10^3 \times \frac{5 \times 10^3}{12}} = R/2$$

$$R = 3N$$

28. Match the information related to test on aggregates given in Group-I with that in Group-II.

| G | ro | 111 | n- | ĭ |
|---|----|-----|----|---|
| • | | 44 | ,- | _ |

P. Resistance to impact

Q. Resistance to wear

Resistance to weathering action

Group-II

- Hardness
- Strength
- Toughness

Answer: (B)

Exp: Resistance to impact → Toughness

Resistance to wear → Hardness

Resistance to weathering → Soundness

Resistance to crushing → Strength

29. A simply supported reinforced concrete beam of length 10 m sags while undergoing shrinkage. Assuming a uniform curvature of 0.004 m-1 along the span, the maximum deflection (in m) of the beam at mid-span is

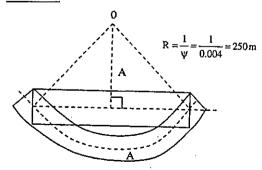
Answer: 0.0005

Exp:

$$OA = \sqrt{(250)^2 - \left(\frac{1}{2}\right)^2} = 249.9995 m$$

$$\Delta AA' = 0.0005m$$

Objective Civil Eng. \ 2016 \ 77



(iv) Average number of vehicles (by vehicle type) that would store in all lanes per cycle during the peak hour.

As per the IRC recommendations, the correct choice for design length of storage lanes is

- (A) Maximum of (ii and iii)
- (B) Maximum of (i and iii)
- (C) Average of (i and iii)
- (D) Only (iv)

Answer: (A)

Q. No. 26-55 carry Two Marks Each

- 26. Ultimate BOD of a river water sample is 20 mg/L. BOD rate constant (natural log) is 0.15 day⁻¹. The respective values of BOD (in %) exerted and remaining after 7 days are:
 - (A) 45 and 55
- (B) 55 and 45
- (C) 65 and 35
- (D) 75 and 25

Answer: (C)

Exp:

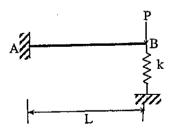
 $y_u = 20mg L$

After 7 days = $y_u e^{-kt} = 20 \times e^{0.15 \times 7} = 7$

% is =
$$\frac{7}{20}$$
 = 35%

exerted 100 35 65%

27. A steel strip of length, L = 200 mm is fixed at end A and rests at B on a vertical spring of stiffness, k = 2 N/mm. The steel strip is 5 mm wide and 10 mm thick. A vertical load, P = 50 N is applied at B, as shown in the figure. Considering E = 200 GPa, the force (in N) developed in the spring is _____.



Answer: 3

607

While minimizing the function f(x), necessary and sufficient conditions for a point, xo to be a minima are:

[A
$$_{0}$$
) > 0 and f "(x_{0}) = 0

(B)
$$f'(x_0) < 0$$
 and $f''(x_0) = 0$

(C)
$$f'(x_0) = 0$$
 and $f''(x_0) < 0$

(C)
$$f'(x_0) = 0$$
 and $f''(x_0) < 0$ (D) $f'(x_0) = 0$ and $f''(x_0) > 0$

Answer: (D)

22. The combined correction due to curvature and refraction (in m) for distance of 1 km on the surface of Earth is

Answer: (A)

Exp:
$$C = 0.0673d^2 = 0.0673 \times 1$$

23. Surcharge loading required to placed on the horizontal backfill of a smooth retaining vertical wall so as to completely eliminate tensile crack is:

(B)
$$2 \text{ ck}_a$$
 (C) $2\text{c}\sqrt{\text{k}_a}$

(D)
$$2c/\sqrt{k_a}$$

Answer: (D)

Exp: Surcharge load to be placed as =
$$\frac{2c}{\sqrt{k_a}}$$

24. A nozzle is so shaped that the average flow velocity changes linearly from 1.5 m/s at the beginning to 15 m/s at its end in a distance of 0.375 m. The magnitude of the convective acceleration (in m/s2) at the end of the nozzle is

Answer: 54

Exp: Convective acceleration =
$$u \frac{du}{dx} + v \frac{du}{dy} + w \frac{du}{dz}$$

$$=1.5\frac{(15-1.5)}{0.375}=54 \text{ m/s}^2$$

The following statements are made related to the lengths of turning lanes at signalised intersections

(i) 1.5 times the average number of vehicles (by vehicle type) that would store in turning lane per cycle during the peak hour.

(ii) 2 times the average number of vehicles (by vehicle type) that would store in turning lane per cycle during the peak hour.

(iii) Average number of vehicles (by vehicle type) that would store in the adjacent through lane per cycle during the peak hour.

so,
$$\Delta_{BC} = \frac{10 \times (.75)^3}{3E_1} + \frac{10 \times (.75)^2}{2E_1} \times 0.25$$
$$= \frac{2.11}{E_1} \downarrow$$

Deflection at C due to Redundant R_C

$$\Delta_{cc} = R_C \times \frac{(.75)^3}{3E_1} = \frac{0.141R_c}{E_1} \uparrow$$

$$\therefore \Delta_C = 0$$

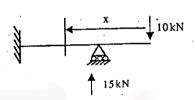
$$\Rightarrow \frac{2.11}{E_1} - \frac{.141R_C}{E_1} = 0$$

$$\Rightarrow R_C = 15 \text{ kN}$$

$$M_x = 10 \times x - 15 \times (x - .25) = 0$$

$$\Rightarrow 10x - 15x - 3.75 = 0$$

 \Rightarrow x = 0.75m



So, distance of point of contraflexure from end A = 1-0.75 = 0.25m

- 19. If the water content of a fully saturated soil mass is 100% the void ratio of the sample
 - (A) Less than specific gravity of soil (B) equal to specific gravity of soil
 - (C) greater than specific gravity of soil (D) independent of specific gravity of soil

Answer: (B)

Exp:

$$e = \frac{wG}{S_r}$$

$$S = 100\% = 1 \qquad \therefore e = \frac{1 \times G}{1} = G$$

$$w = 100\%$$

- 20. The relationship between porosity (), specific yield (Sy) and specific retention (Sr) of an unconfined aquifer is

- (A) $S_y + S_r =$ (B) $S_y + S_r = S_r$ (C) $S_r + S_y = S_y$ (D) $S_y + S_r + S_r = S_y$

Answer: (A)

605

- 17. A superspeedway in New Delhi has among the highest super-elevation rates of any track on the Indian Grand Prix circuit. The track requires drivers to negotiate turns with a radius of 335 m and 33° banking. Given this information, the coefficient of side friction required in order to allow a vehicle to travel at 320 km/h along the curve is
 - (A) 1.761
- (B) 0.176
- (C) 0.253
- (D) 2.530

Answer: 0.685

Exp:

$$V = 320 \text{ kmph}$$

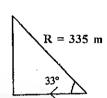
$$= 320 \times \frac{5}{18}$$

$$= \frac{800}{9} \text{ m/s}$$

$$\frac{V^2}{gR} = \frac{\tan \theta + f}{1 - f \tan \theta}$$

$$\Rightarrow \frac{(800/g)^2}{9.81 \times 335} = \frac{\tan 33^\circ + f}{1 - f \times \tan 33^\circ}$$

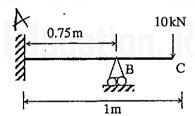
$$\Rightarrow 2.40 = \frac{0.64g + f}{1 + f \times \tan 33^\circ}$$



$$\Rightarrow 2.40 = \frac{0.64g + f}{1 - f \times 0.649}$$

$$\Rightarrow f = 0.685$$

18. A horizontal beam ABC is loaded as shown in the figure below. The distance of the point of contraflexure from end A (in m) is __



Answer: 0.25

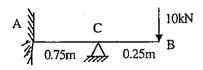
Exp:

Let us take R_c as redundant

Deflection at B due to load at C

Deflection at C due to load at B (ΔBC)

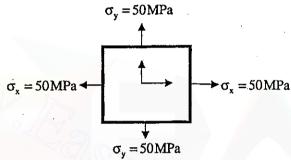
[By Marshall reciprocal theorem]



- 15. Prying forces are
 - (A) shearing forces on the bolts because of the joints
 - (B) tensile forces due to the flexibility of connected parts
 - (C) bending forces on the bolts because of the joints
 - (D) forces due the friction between connected parts

Answer: (B)

16. For the plane stress situation shown in the figure, the maximum shear stress and the plane on which it acts are



- (A) -50 MPa, on a plane 45° clockwise w.r.t. x-axis
- (B) -50 MPa, on a plane 45° anti-clockwise w.r.t. x-axis
- (C) 50 MPa, at all orientations
- (D) Zero, at all orientations

Answer: (D)

$$\sigma_1 = 50 \text{ N/mm}^2$$

$$\tau = 0$$

$$\sigma_2 = 50 \text{ N.mm}^2$$

$$\sigma_{\mathbf{n_1}} = \left(\frac{\sigma_1 + \sigma_2}{2}\right) + \sqrt{\left(\frac{\sigma_1 + \sigma_2}{2}\right) + \tau^2}$$

$$= \left(\frac{50 + 50}{2}\right) + \sqrt{0 + 0} = 50 \text{ N/mm}^2$$

$$\sigma_{\rm n_1} = \left(\frac{50 + 50}{2}\right) - 0 = 50 \text{ N/mm}^2$$

$$\tau_{\text{max}} = \frac{\sigma_{n_1} - \sigma_{n_2}}{2} = \frac{50 - 50}{2} = 0$$

603

- 12. Net ultimate bearing capacity of a footing embedded in a clay stratum
 - (A) increases with depth of footing only
 - (B) increases with size of footing only
 - (C) increases with depth and size of footing
 - (D) is independent of depth and size of footing

Answer: (D)

Exp: Because
$$q_u = CN_c + 8DN_q + 0.5 \gamma BN_r$$

It is clay $\therefore \phi = 0 \Rightarrow N_r = 0, N_q = 1$
 $q_u = CN_c + \gamma D$
 $q_{nu} = CN_c + \gamma D - \gamma D = CN_c$

13. A groundwater sample was found to contain 500 mg/L total dissolved solids (TDS). TDS (in %) present in the sample is

Answer: 0.05

Exp: TDS 500mg / lit 1 lit = 500 mg = 500 × 10⁻³ gm
% TDS =
$$\frac{500}{1000}$$
 × 10⁻³ × 100 1 lit = 1000 gm

14. In Newton-Raphson iterative method, the initial guess value (x_{ini}) is considered as zero while finding the roots of the equation: $f(x) = -2 + 6x - 4x^2 + 0.5x^3$. The correction, x_{ini} , to be added to xini in the first iteration is _____.

Answer: 0.3333

Exp:
$$f(x) = -2 + 6x - 4x^{2} + (0.5) x^{3}$$
$$x_{0} = 0$$
$$f'(x) = 6 - 8x + 1.5x^{2}$$
$$f(0) = -2 \qquad f'(0) = 6$$

By Newton-Raphson method

$$x_{1} = x_{0} - \frac{f(x_{0})}{f'(x_{0})} = 0 - \frac{(-2)}{6}$$

$$= \frac{2}{6}$$

$$= 0.3333$$

$$\Delta x = x_{1} - x_{0} = 0.3333 - 0 = 0.3333$$

- 8. A steel member 'M' has reversal of stress due to live loads, whereas another member 'N' has reversal of stress due to wind load. As per IS 800:2007, the maximum slenderness ratio permitted is
 - (A) less for member 'M' than that of member 'N'
 - (B) more for member 'M' than for member 'N'
 - (C) same for both the members
 - (D) not specified in the Code

Answer: (A)

Exp: M - due to live load

N - due to wind load

As per IS800.
$$\frac{M - \lambda - 180}{M - \lambda - 350}$$
 $M < N$

(A) e^{-2}

(B) e

(C) 1

 $(D) e^{2}$

Answer: (D)

Exp:

$$\lim_{x \to \infty} \left(1 + \frac{1}{x} \right)^{2x}$$

$$= \left(\lim_{x \to \infty} \left(x + \frac{1}{x}\right)^{x}\right)^{2}$$

$$= e^{2}$$

10. In a leveling work, sum of the Back Sight (B.S.) and Fore Sight (F.S.) have been found to be 3.085 m and 5.645 m respectively. If the Reduced Level (R.L.) of the starting station is 100.000 m, the R.L. (in m) of the last station is _____.

Answer: 97.440

Exp: $\Sigma BS = 3.085$

$$= 3.085$$
 $\Sigma F.s = 5.645 m$

Fall =
$$\Sigma Fs - \Sigma BS = 5.645 - 3.085 = 2.560$$

$$= -100 - 2.560 = 97.440m$$

11. In friction circle method of slope stability analysis, if r defines the radius of the slip circle, the radius of friction circle is

(A) r sin φ

(B) r

(C) r cos \$

(D) r tan ϕ

Answer: (A)

601

A guided support as shown in the figure below is represented by three springs 5. (horizontal, vertical and rotational) with stiffness kx, ky and k respectively. The limiting values of kx, ky and k are



(A)
$$\infty$$
, 0, ∞

(B)
$$\infty, \infty, \infty$$

(C)
$$0, \infty, \infty$$

(D)
$$\infty, \infty, 0$$

Answer: (A)

As rotation and horizontal deflection in zero as per given figure. Therefore its Exp: stiffness is

> Force '∞' as deflection = 0. stiffness = deflection

and stiffness is zero in y direction

Let $A = [\alpha_{ij}], \ l \le i, \ j \le n$ with $n \ge 3$ and $aij = i, \ j$. The rank of A is

(C)
$$n - 1$$

Answer: (B)

Exp: Given $A = [a_{ij}] \ 1 \le i, j \le n, n \ge 3$ and $a_{ii} = i.j$

$$\Rightarrow A = \begin{cases} 1 & 2 & 3 & --- \\ 2 & 4 & 6 & --- \\ 3 & 6 & 9 & --- \\ - & - & --- \end{cases}$$

If we apply R_2-2R_1 , R_3-3R_1

Every row will be zero row, except first row in echelon form

$$\therefore \rho(A) = 1$$

A hydraulic jump takes place in a frictionless rectangular channel. The pre-jump depth 7. is y_p . The alternate and sequent depths corresponding to y_p are y_a and y_s respectively. The correct relationship among y_p , y_a and y_s is

$$(A) y_a < y_s < y_p$$

(A)
$$y_a < y_s < y_p$$
 (B) $y_p < y_s < y_a$ (C) $y_p < y_s = y_a$ (D) $y_a = y_s = y_p$

$$(C) y_p < y_s = y_a$$

(D)
$$y_a = y_s = y_n$$

Answer: (B)

Objective Civil Eng. \ 2016 \ 76

Objective Civil Engineering

600

$$V\alpha\sqrt{y}$$

$$V\alpha\sqrt{L_r} \quad :: y \alpha L_r$$

- 3. Given $i = \sqrt{-1}$, the value of the definite integral, $I = \int_{0}^{\pi/2} \frac{\cos x + \sin x}{\cos x i \sin x} dx$ is:
 - (A) 1
- (B) -1
- (C) i

(D) -i

Answer: (B)

Exp:

$$I = \int_{0}^{\pi/2} \frac{\cos x + \sin x}{\cos x - i \sin x} dx$$

$$= \int_{0}^{\pi/2} \frac{e^{ix}}{e^{-ix}} dx = \int_{0}^{\pi/2} e^{2ix} dx$$

$$= \left(\frac{e^{2ix}}{2}\right)^{\pi/2}$$

$$= \frac{1}{2} \left[e^{\frac{2i\pi}{2}} - e^{0}\right]$$

$$= \frac{1}{2} \left[\cos \pi + i \sin \pi - 1\right]$$

4. SO₂ and CO adversely affect

 $=\frac{1}{2}[-1+0 -1]=-1$

- (A) oxygen carrying capacity of blood and functioning of lungs respectively
- (B) functioning of the respiratory system and brain respectively
- (C) functioning of the respiratory system and oxygen carrying capacity of blood respectively
- (D) functioning of air passages and chest respectively.

Answer: (C)

Exp: Carbon monoxide effects the bloods carrying capacity

599

Let capacity of tank B is x

$$\frac{70}{100} \times = 14000$$

 \Rightarrow x = 20000 gallons

Solution in tank A =
$$\frac{80}{100}$$
 × 14000 = 11200 gallons

Solution in tank B =
$$\frac{40}{100} \times 20000 = 8000$$
 gallons

Total solution = 11200 + 8000 = 19200 gallons

SECTION NAME: CIVIL ENGINEERING

Q. No. 1-25 carry one Mark Each

- 1. A column of size 450 mm × 600 mm has unsupported length of 3.0 m and is braced against side sway in both directions. According to IS 456:2000, the minimum eccentricities (in mm) with respect to major and minor principle axes are
 - (A) 20.0 and 20.0 (B) 26.0 and 21.0 (C) 26.0 and 20.0
- (D) 21.0 and 15.0

Answer: (B)

Exp:

$$e_{min} = \frac{L}{500} + \frac{D}{30}$$
 (or) 20mm which ever is minimum.

$$e_{xx} = \frac{3000}{500} + \frac{600}{30} = 26 \text{ mm}$$

$$e_{yy} = \frac{300}{500} + \frac{450}{30} = 21 \text{ mm}$$

The relationship between the length scale ratio (L_r) and the velocity scale ratio (V_r) in 2. hydraulic models, in which Froude dynamic similarity is maintained, is

(A)
$$V_r = L_r$$

(A)
$$V_r = L_r$$
 (B) $L_r = \sqrt{V_r}$ (C) $V_r = L_r^{1.5}$

(C)
$$V_r = L_r^{1.5}$$

(D)
$$V_r = \sqrt{L_r}$$

Answer: (D)

Exp: Fround number =
$$\frac{V}{\sqrt{gy}}$$

9. Read the following table giving sales data of five types of batteries for years 2006 to 2012

| Year | Туре | Туре | Type | Туре | Туре |
|------|------|------|------|------|------|
| 1001 | I | II | III | IV | V |
| 2006 | 75 | 144 | 114 | 102 | 108 |
| 2007 | 90 | 126 | 102 | 84 | 126 |
| 2008 | 96 | 114 | 75 | 105 | 135 |
| 2009 | 105 | 90 | 150 | 90 | 75 |
| 2010 | 90 | 75 | 135 | 75 | 90 |
| 2011 | 105 | 60 | 165 | 45 | 120 |
| 2012 | 115 | 85 | 160 | 100 | 145 |

Out of the following, which type of battery achieved highest growth between the years 2006 and 2012?

- (A) Type V
- (B) Type III
- (C) Type II
- (D) Type I

Answer: (D)

Exp: Type-I achieved a growth of 53% in the period which is higher than any other type of battery

10. The given question is followed by two statements: select the most appropriate option that solves the question Capacity of a solution tank A is 70% of the capacity of tank B. How many gallons of solution are in tank A and tank B?

Statements:

- Tank A is 80% full and tank B is 40% full
- II. Tank A if full contains 14,000 gallons of solution
- (A) Statement I alone is sufficient
- (B) Statement II alone is sufficient
- (C) Either statement I or II alone is sufficient
- (D) Both the statements I and II together are sufficient

Answer: (D)

Exp: Statement I can be used to solve the question if capacity of both tanks is already known

Statement-II can be used if it is known what quantity of each tank is full/empty. Therefore, by using both statements

5. Mr. Vivek walks 6 meters North-East, then turns and walks 6 meters South- East, both at 60 degrees to East. He further moves 2 meters South and 4 meters West. What is the straight distance in meters between the point he started from and the point he finally reached?

(A) 22

(B) 2

(C) 2

(D) 1/2

Answer: (A)

Q. No. 6-10 carry Two Marks Each

6. How many four digit numbers can be formed with the 10 digits 0, 1, 2, 9 if no number can start with 0 and if repetitions are not allowed?

Answer: 4536

Exp: In thousands place, 9 digits except 0 can be placed

In hundreds place, 9 digits can be placed (including 0, excluding the one used in thousands place)

In tens place, 8 digits can be placed (excluding the ones used in thousands and hundreds place)

In ones place, 7 digits can be placed (excluding the one used in thousands, hundreds and tens place)

Total number of combinations = 9.9.8.7 = 4536

7. The word similar in meaning to 'dreary' is

(A) cheerful

(B) dreamy

(C) hard

(D) dismal

Answer: (D)

Exp: dreary- depressingly dull and bleak or repetitive.

8. There are 16 teachers who can teach Thermodynamics (TD), 11 who can teach Electrical Sciences (ES), and 5 who can teach both TD and Engineering Mechanics (EM). There are a total of 40 teachers, 6 cannot teach any of the three subjects, i.e. EM, ES or TD. 6 can teach only ES. 4 can teach all three subjects, i.e. EM, ES and TD. 4 can teach ES and TD. How many can teach both ES and EM but not TD?

(A) 1

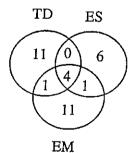
(B) 2

(C) 3

(D) 4

Answer: (A)

Exp:



GATE -2015, PAPER-2

GATE 2015 -CE on 8th February, 2015 - (Afternoon Session)

General Aptitude Questions

| Contract Appearance & account |
|--|
| Q. No. 1-5 carry one Mark Each |
| 1. Choose the most appropriate word from the options given below to complete the following sentence The official answered that the complaints of the citizen would be looked into. |
| (A) respectably (B) respectfully (C) reputably (D) respectively |
| Answer: (B) |
| 2. Choose the statement where underlined word is used correctly |
| (A) The minister insured the victims that everything would be all right. |
| (B) He ensured that the company will not have to bear any loss. |
| (C) The actor got himself ensured against any accident. |
| (D) The teacher insured students of good results |
| Answer: (B) |
| Exp: insured-the person, group, or organization whose life or property is covered by an insurance policy. |
| ensured- to secure or guarantee |
| 3. Four cards are randomly selected from a pack of 52 cards. If the first two cards are kings, what is the probability that the third card is a king? |
| (A) $4/52$ (B) $2/50$ (C) $1/52 \times (1/52)$ (D) $1/52 \times (1/52) \times (1/50)$ |
| Answer: (B) |
| Exp: There are 4 kings in a pack of 52 cards. |
| If 2 cards are selected and both are kings, remaining cards will be 50 out of which 2 will be kings. |
| 4. Which word is not a synonym for the word vernacular? |
| (A) regional (B) indigeneous (C) indigent (D) colloquial |
| Answer: (C) |
| Exp: vernacular- expressed or written in the native language of a place |
| indigent -deficient in what is requisite |

$$\int dv = \int (\alpha - \beta v_0) e^{-\beta T} dt$$

$$= \frac{(\alpha - \beta V_0) e^{-\beta T}}{-\beta}$$

$$t = 0, V = V_0$$

$$\Rightarrow V_0 = \frac{(\alpha - \beta V_0)}{-\beta} + C$$

$$C = V_0 = \frac{\alpha - \beta V_0}{-\beta} \Rightarrow C = \frac{\alpha}{\beta}$$

$$\Rightarrow V = \frac{\alpha - (\alpha \beta V_0) \times e^{-\beta T}}{\beta}$$

$$x = \frac{\alpha t_0}{\beta} + \frac{\alpha - \beta V_0}{\beta^2} \left(e^{-\beta t_0} - 1 \right)$$

$$\frac{dv}{dt} \Big|_{t=3} = (\alpha - \beta V_0) e^{-3\beta} = 1.3$$

$$\Rightarrow \alpha - \beta V_0 = \frac{1.3}{e^{-3\beta}}$$

$$x = \frac{\alpha t_0}{\beta} + \frac{1.3}{\beta^2 \left(e^{-\beta t} \right)} \left(e^{-\beta t_0} - 1 \right)$$

$$= \frac{2 \times 35}{0.05} + \frac{1.3 \left(e^{-35 \times 0.05} - 1 \right)}{(0.05)^2 \left(e^{-3 \times 0.05} \right)}$$

$$= 1400 - 499.17 = 900.83 \text{ m}$$

55. On a circular curve, the rate of super elevation is e. While negotiating the curve a vehicle comes to a stop. It is seen that the stopped vehicle does not slide inwards (in the radial direction). The coefficient of side friction is f. Which of the following is true?

(A)
$$e \le f$$

(B)
$$f < e < 2f$$

(C)
$$e \ge 2f$$

(D) None of the above

Answer: (A)

Exp:

$$f_N \ge mg \sin \theta$$

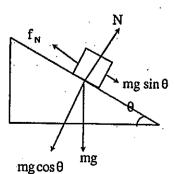
$$\Rightarrow$$
 f (mg cos θ) = mg sin θ

$$\Rightarrow$$
 $f \ge \tan \theta$

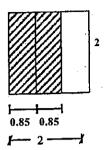
$$\Rightarrow$$
 f \geq e

$$\Rightarrow e \leq f$$





For one way shear (eccentricity) area to be reduced Reduced area of footing = $2 \times 1.7 = 3.4 \text{m}^2$ Load carrying capacity = $132.364 \times 3.4 = 450.\text{kN}$



53. In a catchment, there are four rain-gauge stations, P, Q, R, and S. Normal annual precipitation values at these stations are 780 mm, 850 mm, 920 mm, and 980 mm, respectively. In the year 2013, stations Q, R, and S, were operative but P was not. Using the normal ratio method, the precipitation at station P for the year 2013 has been estimated as 860 mm. If the observed precipitation at stations Q and R for the year 2013 were 930 mm and 1010 mm, respectively; what was the observed precipitation (in mm) at station S for that year?

Answer: 1076.2

Exp:

$$\frac{P_{s}}{N_{s}} = \frac{1}{3} \left[\frac{P_{p}}{N_{p}} + \frac{P_{Q}}{N_{Q}} + \frac{P_{R}}{N_{R}} \right]$$

$$\Rightarrow \frac{P_{s}}{980} = \frac{1}{3} \left[\frac{860}{780} + \frac{930}{850} + \frac{1010}{920} \right]$$

$$\Rightarrow P_{s} = 1076.20 \text{ mm}$$

54. The acceleration-time relationship for a vehicle subjected to non-uniform acceleration is, $\frac{dv}{dt} = (\alpha - \beta v_0)e^{-\beta t}$

Where, v is the speed in m/s, t is the time in s, α and β are parameters, and 0 v is the initial speed in m/s. If the accelerating behavior of a vehicle, whose drive intends to overtake a slow moving vehicle ahead, is described as,

$$\frac{\mathrm{d}\mathbf{v}}{\mathrm{d}\mathbf{t}} = (\alpha - \beta \mathbf{v})$$

Considering $\alpha = 2m/s^2$, $\beta = 0.05s^{-1}$ and $\frac{dv}{dt}$ 1.3 m./s² = at t = 3 s, the distance (in m) travelled by the vehicle in 35 s is ______.

Answer: 900.83

Exp:
$$\frac{dV}{dt} = (\alpha - \beta v_0)e^{-\beta T}$$

$$\Rightarrow \lambda^3 - 4\lambda^2 + 5\lambda - 2 = 0$$

$$(\lambda - 1) (\lambda^2 - 3\lambda + 2) = 0$$

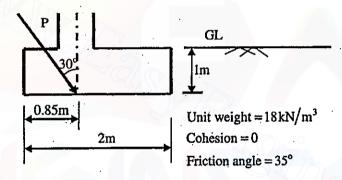
$$(\lambda - 1) (\lambda - 1) (\lambda - 2) = 0$$

$$\lambda = 1.2$$

52. A square footing (2mx 2m) is subjected to an inclined point load, P as shown in the figure below. The water table is located well below the base of the footing. Considering one-way eccentricity, the net safe load carrying capacity of the footing for a factor of safety of 3.0 is _____ kN.

The following factors may be used. Bearing capacity factors: Nq = 33.3, $N_v 37.16$; = Shape factors: $F_{qs} F_{ys} 1.314$; = Depth

factors: $F_{qd} = F_{\gamma d} = 1.113$; Inclination factors: $F_{qi} = 0.444$, $F_{\gamma i} = 0.02$



Answer: 450

$$q_{\text{safe}} = \frac{q_{\text{nu}}}{3}$$

$$q_{nu} = cN_c + qN_q + 0.5 \gamma BN_{\gamma} - 8 \Delta$$

$$q_{nu} = q (N_q - 1) + 0.5 \gamma BN_{\gamma}$$

$$q_{ns} = \frac{1}{3} (q (N_q - 1) F_{qs} \times F_{qd} + F_{qp} + 0.5\gamma BN_{\gamma} \times F_{\gamma s} \times F_{\gamma o} \times F_{\gamma p})$$

$$q_{ns} = \frac{1}{3} \left(\frac{18 \times 1(33.3 - 1) \times 1.314 \times 1.113 \times 0.444 + \frac{1}{2} \times 2 \times 18 \times }{37.16 \times 1.314 \times 1.113 \times 0.02} \right)$$

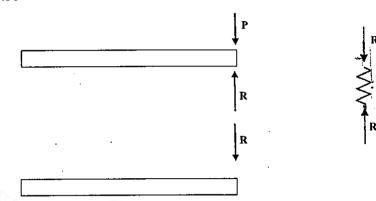
$$= \frac{397.03}{3} = 132.364 \text{ kN/m}^2$$

Objective Civil Eng. \ 2016 \ 75

Objective Civil Engineering

Answer: 33.33

Exp:



$$\Delta = \frac{R}{K}$$
$$= \frac{R}{3EI} \times 2L^{3}$$

Net deflection of upper beam = deflection of spring

$$\frac{PL^{3}}{3EI} - \frac{RL^{3}}{3EI} = \frac{2RL^{3}}{3EI}$$

$$\Rightarrow \frac{PL^{3}}{3EI} = \frac{3RL^{3}}{3EI} \Rightarrow R = \frac{P}{3} = 33.33\%$$

51. The smallest and largest Eigen values of the following matrix are:

$$\begin{bmatrix} 3 & -2 & 2 \\ 4 & -4 & 6 \\ 2 & -3 & 5 \end{bmatrix}$$

Answer: (D)

Exp:

$$Let A = \begin{bmatrix} 3 & -2 & 2 \\ 4 & -4 & 6 \\ 2 & -3 & 5 \end{bmatrix}$$

Characteristic equation is

$$|\mathbf{A} - \lambda \mathbf{I}| = 0$$

$$\Rightarrow \begin{vmatrix} 3 - \lambda & -2 & 2 \\ 4 & -4 - \lambda & 6 \\ 2 & -3 & 5 - \lambda \end{vmatrix} = 0$$

3m

3m

5kN

10kN

C

В

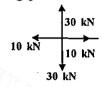
Answer: 5

Exp:

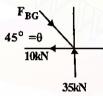
$$R_A \times 3 + 10 \times 9 = 0$$

 $\Rightarrow R_A = -30 \text{ kN}$
 $R_G = 35 \text{ kN}$

Taking joint A



Joint G



$$\Sigma H = 0 \quad F_{BG} \cos 45^{\circ} = 10$$

$$F_{BG} = \frac{10}{\cos 45} = 14.14 \text{ kN}$$

Joint B

$$\Sigma H = 0$$
,

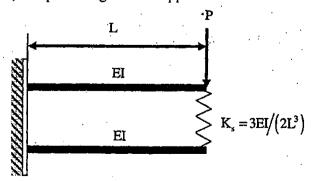
$$F_{BG} \cos 45^{\circ} = F_{BF}$$

$$\therefore F_{BF} = 10 \text{ kN}$$

$$U = \frac{F^2 \times L}{2A_E} = \frac{10 \times 10 \times 3}{2 \times 30} = 5kN - m$$

B B_{BF} 30 F_{BG}=14.14

50. Two beam are connected by linear spring as shown in the following figure. For a load P as shown in the figure, the percentage of the applied load P carried by the spring is



- 48. In a region with magnetic declination of 2°E, the magnetic Fore bearing (FB) of a line AB was measured as N79°50'E. There was a local attraction at A. To determine the correct magnetic bearing of the line, a point O was selected at which there was no local attraction. The magnetic FB of line AO and OA were observed to be ° S52 40'E and ° N50 20'W, respectively. What is the true FB of line AB?
 - (A) N81°50'E
- (B) N82°10'E
- (C) N84°10'E
- (D) N77°50'E

Answer: (C)

Exp:

 $\delta = 2^{\circ}E$

Magnetic F.B. of AB = $N79^{\circ}50$ 'E = $79^{\circ}50$.

Correct FB of $OA = N50^{\circ}20$ 'W = $309^{\circ}40$ '

- :. Correct B.B of OA = 129°40'
- observed F.B. of AO = observed BB of OA

$$= S 52°40'E = 127°20'$$

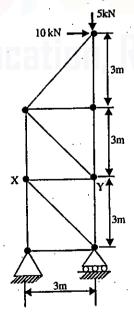
 $Error = M.V- T.V = -2^{\circ}20'$

Correction + 2°20'

T.B. of FB of AB = $N79^{\circ}50'E + 2 + 2^{\circ}20'$

 $= N84^{\circ}10'E$

49. For the 2D truss with the applied loads shown below, the strain energy in the member XY is _____ kN-m. For member XY, assume AE = 30 kN, where A is cross-section area and E is the modulus of elasticity.



47. A non-homogenous soil deposit consists of a silt layer sandwiched between a fine-sand layer at top and a clay layer below. Permeability of the silt layer is 10 times the permeability of the clay layer and one-tenth of the permeability of the sand layer. Thickness of the silt layer is 2 time the thickness of the sand layer and two-third of the thickness of the clay layer. The ratio of equivalent horizontal and equivalent vertical permeability of the deposit is ______.

Answer: 10.967

Exp:

$$\frac{H_1}{H_2}$$
 (1) K_1 fine sand $\frac{H_2}{H_3}$ (2) K_2 silt $\frac{H_3}{H_3}$ (3) K_3 clay $k_2 = 10k_3 = \frac{1}{10} k_1$

$$\Rightarrow k_1 = 10k_2$$
$$= 10 \times 10k_3$$

$$k_1 = 100 k_3$$

$$k_1 = 10k_2$$

$$H_2 = \frac{2}{3} H_3 \Rightarrow H_3 = \frac{3}{2} H_2 = \frac{3}{2} \times 2H_1 = 3H_1$$

$$H_3 = 3H_1$$

$$K_{x} = \frac{K_{1} H_{1} + K_{2} H_{2} + K_{3} H_{3}}{H_{1} + H_{2} + H_{3}} = \frac{k_{1} H_{1} + \frac{1}{10} K_{1} \times 2H_{1} + \frac{1}{100} K_{1} \times 3H_{1}}{H_{1} + 2H_{1} + 3H_{1}}$$

$$k_{x} = \frac{\left(1 + \frac{2}{10} + \frac{3}{100}\right) K_{1} H_{1}}{6H_{1}} = \frac{123}{100 \times 6} K_{1}$$

$$k_{y} = \frac{H_{1} + H_{2} + H_{3}}{\frac{H_{1}}{K_{1}} + \frac{H_{2}}{K_{2}} + \frac{H_{3}}{K_{3}}} = \frac{6H_{1}}{\frac{H_{1}}{K_{1}} + \frac{2H_{1} \times 10}{K_{1}} + \frac{3H_{1} \times 100}{K_{1}}} = \frac{6}{321} K_{1}$$

$$\frac{K_x}{K_y} = \frac{123}{100 \times 6} \times \frac{321}{6} = 10.967$$

Objective Civil Engineering

Answer: 156.20

Exp:

$$F_{D} = \frac{P}{n} = \frac{100}{5} = 20 \text{kN}$$

$$F_{R} = \sqrt{F_{D}^{2} + F_{t}^{2} + 2 \times F_{D} \times F_{t} \cos \theta}$$

$$= \sqrt{(20)^{2} + (141.42)^{2} + 2 \times 20 \times 141.42 \times \frac{1}{\sqrt{2}}}$$

$$= 156.20 \text{ kN}$$

$$\cos \theta = \frac{1}{\sqrt{2}}$$

$$\Rightarrow \theta = 45^{\circ}.$$

46. Consider a primary sedimentation tank (PST) in a water treatment plant with surface Overflow Rate (SOR) of 40 m³/m²/d. The diameter of the spherical particle which will have 90 percent theoretical removal efficiency in this tank is _____ μm. Assume that settling velocity of the particles in water is described by Stokes's Law. Given Density of water = 1000 kg/m³; Density of particle = 2650 kg/m³; g = 9.81 m²/s

Kinematic viscosity of water $(v) = 1.10 \times 10^{-6} \text{ m}^2/\text{s}$

Answer: 22.58

Exp:

p: % removal =
$$\frac{V_s}{V_s} \times 100$$

 $V_s' = 0.9 V_s$
= $\frac{0.9 \times 40}{86400}$ m/s

$$\Rightarrow \frac{1}{18} \times d^2 \times \frac{g}{\mu} (\rho_s - \rho_w) = \frac{0.9 \times 40}{86400}$$

$$\Rightarrow d = \sqrt{\frac{0.9 \times 40 \times 18 \times V \cdot \rho_w}{86480 (G_s - 1) \times \rho_w \times g}}$$

$$\Rightarrow d = 22.58...m$$

587

Where Tv is the time factor and U is the degree of consolidation in %.

If the coefficient of consolidation of the layer is 0.003 cm2/s, the deposit will experience a total of 50 mm settlement in the next ______ years.

Answer: 4.43

Exp:

$$T_{v} = \frac{C_{v.t}}{H^{2}} = \frac{0.003 \times (2 \times 365 \times 24 \times 3600)}{\left(\frac{20}{2} \times 100\right)^{2}}$$
$$= 0.189$$

$$T_v = \frac{\pi}{4} U^2 = 0.189 \Rightarrow U = 0.49 \le 60\%$$

Degree of consolidation for 50 mm settlement

$$= \frac{50}{30} \times 0.49 = 0.8166 = 81.7 \% > 60\%$$

$$\Rightarrow T_v = 1.784 = 0.933 \log_{10} (100 - U)$$

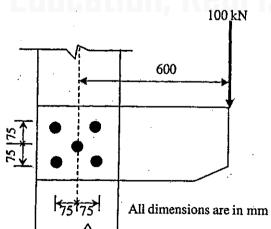
$$= 0.608 = \frac{C_v \times t}{d^2}$$

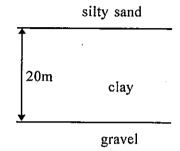
$$\Rightarrow t = \frac{0.608 \times H^2}{0.003 \times 10^{-4}} = \frac{0.608 \times (10)^2}{0.003 \times 10^{-4}} s$$

- = 202666667 s
- = 6.43 yr

Additional number of years 6.43 - 2 = 4.43 years

45. A bracket plate connected to a column flange transmits a load of 100 kN as shown in the following figure. The maximum force for which the bolts should be designed is kN.





$$S_{f} = 6.92 \times 10^{-3}$$

$$\frac{dy}{dx} = \frac{\frac{1}{1000} - 6.92 \times 10^{-3}}{1 - (1.79)^{2}} = 3.2 \times 10^{-3} = 0.0032$$

- 42. In a survey work, three independent angles X, Y and Z were observed with weights W_x , W_y , W_z , respectively. The weight of the sum of angles X, Y and Z is given by:
 - (A) $1 / \left(\frac{1}{W_x} + \frac{1}{W_y} + \frac{1}{W_z} \right)$
- (B) $\left(\frac{1}{W_x} + \frac{1}{W_y} + \frac{1}{W_z}\right)$

(C) $W_x + W_y + W_z$

(D) $W_x^2 + W_y^2 + W_z^2$

Answer: (A)

43. A hydraulic jump is formed in a 2m wide rectangular channel which is horizontal and frictionless. The post-jump depth and velocity are 0.8 m and 1 m/s, respectively. The prejump velocity is _____ m/s. (use = 10 m/s²).

Answer: 4.94

Exp: B

$$B = 2m, y_2 = 0.8m, U = 1m / s$$

$$F_2 = \frac{U_2}{\sqrt{g.y_2}} = \frac{1}{\sqrt{10 \times 0.8}} = 0.35$$

$$\frac{y_1}{y_2} = -\frac{1}{2} + \frac{1}{2} \cdot \sqrt{1 + 8F_2^2}$$

$$\Rightarrow \frac{y_1}{0.8} = -\frac{1}{2} + \frac{1}{2} \cdot \sqrt{1 + 8 \times (3.5)^2} = 0.203$$

$$\Rightarrow$$
 y₁ = 0.203 × 0.8 = 0.162 m

$$Q = B.y_2 \cdot V_2 = B.y_1 V_1$$

$$\Rightarrow$$
 0.8 × 1 = 0.162 × V_1

$$\Rightarrow$$
 V₁ = 4.94 m/s.

44. A 20 m thick clay layer is sandwiched between a silty sand layer and a gravelly sand layer. The layer experiences 30 mm settlement in 2 years.

Given

$$T_{v} = \begin{cases} \frac{\pi}{4} \left(\frac{U}{100}\right)^{2} & \text{for } U \leq 60\% \\ 1.781 - 0.933 \log_{10} \left(100 - U\right) & \text{for } U > 60\% \end{cases}$$

585

$$f(z) = \frac{9}{(z-1)(z+2)^2}$$

z = 1 is a simple pole

z = -2 is a pole of order 2

$$[\operatorname{Res} f(z)]_{z=1} = \lim_{z \to 1} (z-1) \frac{9}{(z-1)(z+2)^2}$$
$$= \frac{9}{9} = 1$$

$$[\text{Res f}(z)]_{z=2} = \frac{1}{1!} \lim_{z \to 2} \left[(z-2)^2 \frac{9}{(z-1)(z+2)^2} \right]$$

$$= \lim_{z \to -2} \frac{-9}{(z-1)^2}$$
$$= \frac{-9}{9} = -1$$

41. A short reach of a 2 m wide rectangular open channel has its bed level rising in the direction of flow at a slope of 1 m in 10000. It carries a discharge of 4 m3/s and its Manning's roughness coefficient is 0.01. The flow in this reach is gradually varying. At a certain section in this reach, the depth of flow was measured as 0.5m. The rate of change of the water depth with distance, dy/dx, at this section is _____ (use g = 10 m/s²).

Answer: 0.0032

Exp:

Adverse slope =
$$-\frac{1}{10000}$$

$$\theta = 4 \text{ m}^3 / \text{s}, \text{ n } 0.01, \text{ y} = 0.5 \text{m}$$

$$\frac{dy}{dx} = \frac{S_0 - S_f}{1 - F_c^2}$$

$$F_r = \frac{V}{\sqrt{gy}} = \frac{Q}{By\sqrt{gy}} = \frac{4}{2 \times 0.5 \times \sqrt{10 \times 5}} = 1.79$$

$$Q = \frac{1}{n} AR^{2/3} S_f^{1/2}$$

$$S_f^{1/2} = \frac{Q \times n}{A \times R^{2/3}} = \frac{4 \times 0.01}{2 \times 0.5 \times \left(\frac{2 \times 0.5}{2 + 1}\right)^{2/3}}$$

Objective Civil Eng. \ 2016 \ 74

The vehicle requires 174 m to slow down to 30 km/hr

So, minimum distance, X = 174 - 32 = 142 m.

39. The quadric equation $x^2 - 4x + 4 = 0$ is to be solved numerically, starting with the initial guess $x_0 = 3$. The Newton-Raphson method is applied once to get a new estimate and then the Secant method is applied once using the initial guess and this new estimate. The estimated value of the root after the application of the Secant method is ______.

Answer: 2.333

Exp: $f(x) = x^2 - 4x + 4$ $x_0 = 3$ f'(x) = 2x - 4

By Newton Raphson method $x_1 = x_0 - \frac{f(x_0)}{f'(x_0)}$ = $3 - \frac{1}{2} = 2.5$

For secant method let $x_0 = 2.5$ and $x_1 = 3$

By secant method $x_2 = x_1 - \frac{x_1 - x_0}{f(x_1) - f(x_0)} f(x_1)$ $= 3 - \frac{(3 - 2.5)}{f(3) - f(2.5)} f(3)$ $= 3 - \frac{0.5}{1 - (0.25)} \times 1$ $= 3 - \frac{0.5}{0.75}$ = 3 - 0.6667

= 2.333

40. Consider the following complex function:

$$f(z) = \frac{9}{(z-1)(z-2)^2}$$

Which of the following one of the residues of the above function?

(A) -1

(B) 9/16

(C) 2

(D) 9

Answer: (A)

37. Two reservoirs are connected through a 930 m long, 0.3 m diameter pipe, which has a gate valve. The pipe entrances is sharp (loss coefficient = 0.5) and the valve is half-open (loss coefficient = 5.5). The head difference between the two reservoirs is 20 m. Assume the friction factor for the pipe as 0.03 and g = 10 m/s². The discharge in the pipe accounting for all minor and major losses is _____ m³/s.

Answer: 0.1413

Exp: Total loss = 20 m

$$\Rightarrow 20 = \frac{0.5 \times v^2}{2g} + \frac{f \times L}{d} \times \frac{v^2}{2g} + \frac{v^2}{2g}$$

$$\Rightarrow 20 \times 2 \times 10 = 0.5 \text{ v}^2 + \frac{0.03 \times 930 \times \text{v}^2}{0.3} + 5.5 \text{v}^2 + \text{v}^2$$

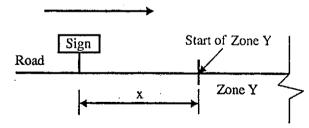
$$\Rightarrow v^2 = \frac{400}{100} = 4$$

$$\Rightarrow v = 2m/s$$

$$Q = \frac{\pi}{4} \times d^2 \times v = \frac{\pi}{4} \times (0.3)^2 \times 2 = 0.1413 \text{ m}^3/\text{s}$$

38. A sign is required to be put up asking drivers to slow down to 30 km/h before entering Zone Y (see figure). On this road, vehicles require 174 m to slow down to 30 km/h (the distance of 174 m includes the distance travelled during the perception-reaction time of drivers). The sign can be read by 6/6 vision drivers from a distance of 48 m. The sign is placed at distance of x m from the start of Zone Y so that even a 6/9 vision driver can slow down to 30 km/h before entering the zone. The minimum value of x is

Direction of vehicle movement



Answer: 142

Exp: For a 6/6 person, driver can see from a distance of 48 m.

For a 6/9 person, driver can see from distance $48 \times \frac{6}{9} = 32 \text{ m}$

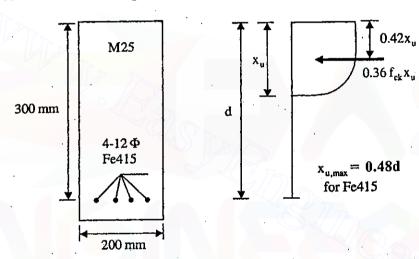
Objective Civil Engineering

582

Exp:
$$\frac{\rho VD}{\mu} = Re \rightarrow \text{ dimensionles parameter}$$

$$\frac{F_{D}\left(kg - m/s^{2}\right)}{\rho\left(\frac{kg}{m^{3}}\right)V^{2}\left(\frac{m^{2}}{s^{2}}\right) \times D^{2}\left(m^{2}\right)} \rightarrow \text{dimensionless parameter}$$

36. Consider the singly reinforced beam section given below (left figure). The stress block parameters for the cross-section from IS:456-2000 are also given below (right figure). The moment of resistance for the given section by the limit state method is _____ kN-m.



Answer: 42.82

Exp:
$$A_{st} = 4 \times \frac{\pi}{4} \times (12)^2 = 453 \text{ mm}^2$$

 $0.36 \text{ f}_{ck}.b.x_u = 0.87 \text{ f}_y A_{st}$

$$\Rightarrow x_{tt} = \frac{0.87 f_y A_{st}}{0.36 f_{ct} \cdot b} = \frac{0.87 \times 415 \times 453}{0.36 \times 25 \times 200}$$

= 90.86 mm

$$x_{u \cdot max} = 0.48 d$$

$$= 0.4 \times 300 = 120 \text{ mm}$$

$$x_u < x_{u.max}$$
 so U.R. section

$$M_u = 0.87 \times f_y \times A_{st} \times (d - 0.42 x_u)$$

$$= 0.87 \times 415 \times 453 \times (300 - 0.42 \times 90.86) = 42.82 \text{ kNm}$$

Gate - 2015, Paper-1

| | Time(hr) | 0 | 2 | 4 | 6 | 8 | 10 | 12 | 1.4 | 16 | 18 | 20 | 22 | 24 |
|-----|-----------------------------|---|-----|-----|----|----|----|----|-----|-----|-----|-----|-----|----|
| | Unit | | | | | | | | | | | | | |
| | hydrograph | 0 | 0.6 | 3.1 | 10 | 13 | 9 | 5 | 2 | 0.7 | 0.3 | 0.2 | 0.1 | 0 |
| . [| ordinate(m ³ /s) | | | | | İ | | | | | | | | |

Answer: 22

Exp:

| Time | UHO | S-curve Addition | S _A |
|------|-----|------------------|----------------|
| 0 | 0 | | 0 |
| 2 | 0.6 | | 0.6 |
| 4 | 3.1 | 0 | 3.1 |
| 6 | 10 | 0.6 | 10.6 |
| 8 | 13 | 3.1 | 16.1 |
| 10 | 9 | 10.6 | 19.6 |
| 12 | 5 | 16.1 | 21.1 |
| 14 | 2 | 19.6 | 21.6 |
| 16 | 0.7 | 21.1 | 21.8 |
| 18 | 0.3 | 21.6 | 21.9 |
| 20 | 0.2 | 21.8 | 22 |
| 22 | 0.1 | 21.9 | 22 |
| 24 | . 0 | 22 | 22. |

Maximum S-curve ordinate is 22.

- 35. The drag force, Fo, on a sphere kept in a uniform flow field depends on the diameter of the sphere, D; flow velocity, V; fluid density, ρ; and dynamic viscosity, μ. Which of the following options represents the non-dimensional parameters which could be used to analyze this problem?
 - (A) $\frac{F_D}{VD}$ and $\frac{\mu}{\rho VD}$

(B)
$$\frac{F_D}{\rho V D^2}$$
 and $\frac{\rho V D}{\mu}$

(C)
$$\frac{F_D}{\rho V^2 D^2}$$
 and $\frac{\rho VD}{\mu}$

(D)
$$\frac{F_D}{\rho V^3 D^3}$$
 and $\frac{\mu}{\rho V D}$

Answer: (C)

Objective Civil Engineering

580

$$\frac{V}{1+e} = \frac{V_x}{1+e_1} = \frac{V_y}{1+e_2} = \frac{V}{1+e_3}$$

$$\gamma_{\rm d} = \frac{\rm G}{1 + \rm e} \cdot \gamma_{\rm w} \Longrightarrow 16.2 = \frac{2.67}{1 + \rm e} \times 10$$

$$\Rightarrow$$
 e = 0.648

$$\frac{5000}{1.648} = \frac{V_x}{1.6} = \frac{V_y}{1.7} = \frac{V_z}{1.64}$$

$$\Rightarrow$$
 Vx = 4854.36 m³

$$Vy = 5157.76 \text{ m}^3$$

$$Vz = 4975.73 \text{ m}3$$

Let, $C = cost of excavation per m^3$

$$C_v = C \times 4854.36 + 2C \times 140 \times 4854.36 = 1.364 \times 10^6 C$$

$$C_y = C \times 5157.76 + 2C \times 80 \times 5157.76 = 0.83 \times 10^6 C$$

$$Cz = C \times 4975.73 + 2C \times 100 \times 4975.33 = 1.0 \times 10^6 C$$

Total cost of site Y is minimum..

33. The concentration of Sulfur Dioxide (SO_2) is ambient atmosphere was measured as $30\mu g$ / m^3 . Under the same conditions, the above SO^2 concentration expressed in ppm is

Given: $3 P / (RT) = 41.6 \text{mol} / \text{m}^3$; where P=Pressure; T=Temperature; R=universal gas constant; Molecular weight of $SO^2 = 64$.

Answer: 0.0133

Exp:

 $1~\text{m}^3$ of air has 30 $\mu\text{g SO}_2$

106 m³ of air has 30g SO₂

$$=\frac{30}{64 \times 41.6}$$
 mol SO₂

$$V = \frac{nRT}{P} = \frac{n}{P/RT} = \frac{30/64}{41.6} \frac{mol}{mol/m^3}$$

Concentration of SO in ppm 0.0113ppm

34. The 4-hr unit hydrograph for a catchment is given in the table below. What would be the maximum ordinate of the S-curve 3 (in m³/s) derived from this hydrograph?

579

Answer: 51

Exp:
$$\frac{M_c}{\rho_c} + \frac{M_s}{\rho_s} + \frac{M_a}{\rho_a} + V_w + V_a = 1$$

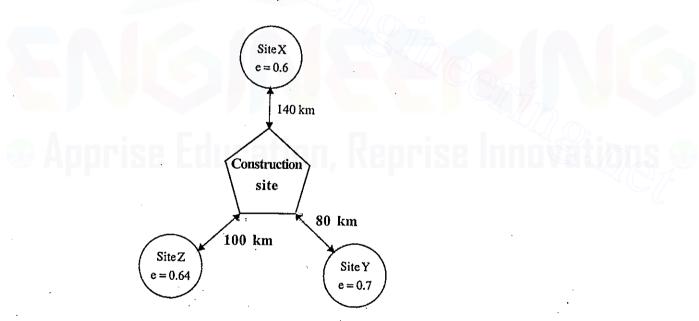
$$\Rightarrow \frac{368}{3.14 \times 1000} + \frac{606}{2.67 \times 1000} + \frac{1155}{2.74 \times 1000} + V_v = 1.0$$

$$\Rightarrow 0.117 + 2.227 + 0.421 + 0.184 + V_v = 1.0$$

$$\Rightarrow V_v = 0.051$$

$$= 0.051 \times 1000 = 51 = 50.32 \text{ 1/m}^3$$

32. An earth embankment is to be constructed with compacted cohesionless soil. The volume of the embankment is 5000 m³ and the target dry unit weight is 16.2 kN/m³. Three nearby sites (see figure below) have been identified from where the required soil can be transported to the construction site. The void ratios (e) of different sites are shown in the figure. Assume the specific gravity of soil to be 2.7 for all three sites. If the cost of transportation per km is twice the cost of excavation per m³ of borrow pits, which site would you choose as the most economic solution? (Use unit Weight of water =10 kN / m³).



(A) Site X

(B) Site Y

(C) Site Z

(D) Any of the sites

Answer: (B)

Objective Civil Engineering

$$\Delta\sigma_0 = \frac{1500}{\frac{\pi}{4} (3 + 6 + 6)^2} = 8.488 \text{ kN/m}^2$$
$$= 0.0532 \text{ m}$$

 $\Delta H = 53.236 \text{ mm}$

30. Consider the following differential equation:

Which of the following is the solution of the above equation (c is an arbitrary constant)?

(A)
$$\frac{x}{y}\cos\frac{y}{x} = c$$
 (B) $\frac{x}{y}\sin\frac{y}{x} = c$ (C) $xy\cos\frac{y}{x} = c$ (D) $xy\sin\frac{y}{x} = c$

Answer: (C)

578

Exp: Given D.E

$$x (ydx + xdy) \cos \frac{y}{x} = y (xdy - ydx) \sin \frac{y}{x}$$

$$\Rightarrow (ydx + xdy) \cos \frac{y}{x} + \left(-\sin \frac{y}{x}\right) y (xdy - ydx) = 0$$

$$\Rightarrow (ydx + xdy) \cos \left(\frac{y}{x}\right) + \left(-\sin \frac{y}{x}\right) \frac{y (xdy - ydx)}{x} = 0$$

$$\Rightarrow (ydx + xdy) \cos \left(\frac{y}{x}\right) + (xy) \left(-\sin \frac{y}{x}\right) \left(\frac{(xdy - ydx)}{x^2}\right) = 0$$

By observing, the above equation is $d\left((xy)\cos\frac{y}{x}\right) = 0$

By integrating, xycos $\left(\frac{y}{x}\right) = c$

31. The composition of an air-entrained concrete is given below:

Water : 184 kg / m

Ordinary PortlandCement(OPC) : 368 kg / m

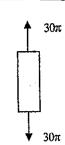
Sand : 606kg / m

Coarse aggregate : 1155 kg / m

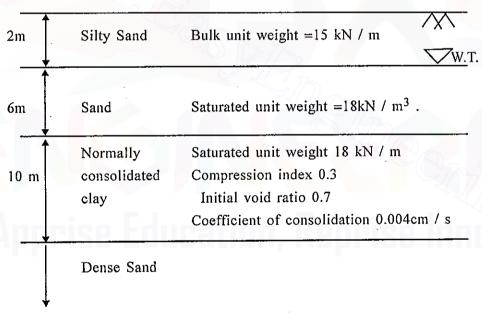
Assume the specific gravity of OPC, sand and coarse aggregate to be 3.14, 2.67 and 2.74, respectively, The air content is _____ liters/ m^3 .

577

$$= \frac{4 \times 30\pi \times 10^3 \times 1500}{\pi \times 10 \times 10 \times 2 \times 10^5}$$
$$= 9 \text{ mm}$$
$$\Delta = \Delta_1 + \Delta_2 = 15 \text{ mm}$$



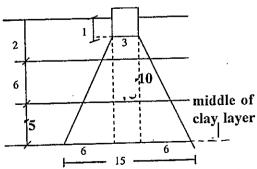
29. A water tank is to be constructed on the soil deposit shown in the figure below. A circular footing of diameter 3m and depth of embedment 1m has been designed to support the tank. The total vertical load to be taken by the footing is 1500 kN. Assume the unit weight of water as 10kN/m³ and the load dispersion pattern as 2V:1H. The expected settlement of the tank due to primary consolidation of the clay layer is mm.



Answer: 53.236

Exp: Settlement =
$$\frac{C_c}{1 + e_0} H_0 \log \left(\frac{\sigma_0 + \Delta \sigma}{\sigma_0} \right)$$

 $\sigma_0 = 15 \times 2 + (18 - 10) \times 6 + (18 - 10) \times 5$
= 118kN/m^2

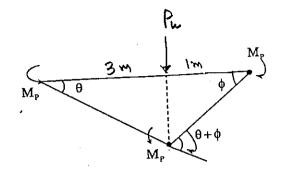


Objective Civil Eng. \ 2016 \ 73

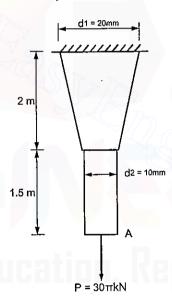
Objective Civil Engineering

$$\Rightarrow 10M_{p} \cdot \theta + P_{U} \times 3\theta \times \frac{L}{4}$$

$$\Rightarrow P_{U} = \frac{40}{3} \cdot \frac{M_{p}}{L} = 13.33 \cdot \frac{M_{p}}{L}$$
So, C = 13.33



28. A tapered circular rod of diameter varying from 20 mm to 10 mm is connected to another uniform circular rod of diameter 10 mm as shown in the following figure. Both bars are made of same material with the modulus of elasticity, $E = 2 \times 10^5$ MPa. When subjected to a load $P = 30\pi$ kN, the deflection at point A is ____ mm.



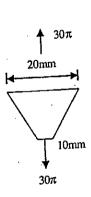
Answer: 15

$$\Delta_1 = \frac{4P.L}{\pi d_1 d_2 \times E}$$

$$= \frac{4 \times 30\pi \times 10^3 \times 2000}{\pi \times 20 \times 10 \times 2 \times 10^5}$$

$$= 6 \text{ mm}$$





575

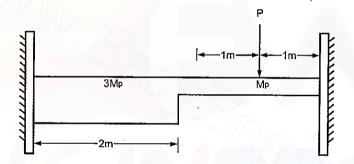
directionalderivative = $\nabla u \cdot \hat{a}$

$$= (4i - 12j + 3k) \cdot \frac{(i + j - 2k)}{\sqrt{6}}$$

$$= \frac{4 - 12 - 6}{\sqrt{6}}$$

$$= \frac{-14}{\sqrt{6}} = -5.72$$

27. For formation of collapse mechanism in the following figure, the minimum value of P_u is cM_p/L . M_p and $3M_p$ denote the plastic moment capacities of beam sections as shown in this figure. The value of c is _____.



Answer: 13.33

Exp: Mechanism-I

$$4M_{p} \cdot \theta + M_{p} (2\theta) + MP \cdot \theta = P_{U} \times \frac{L}{4} \times \theta$$

$$\Rightarrow 6M_{p} \cdot \theta = P_{U} \cdot \frac{L}{4} \cdot \theta$$

$$\Rightarrow P_U = 24 \frac{M_P}{r}$$

Mechanism-II

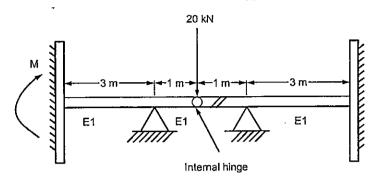
$$1.q = 3.q$$
 $f = 3q$

$$3M_p$$
. $\theta + M_p (\theta + \phi) + M_p . \theta = P_U \times \frac{L}{4} \phi$

$$\Rightarrow 3M_p \cdot \theta + M_p (\theta + 3\theta) + M_p \cdot 3\theta = P_U \times 3\theta \times \frac{L}{4}$$

Objective Civil Engineering

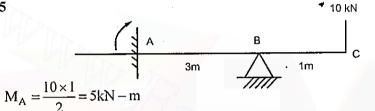
25. For the beam shown below, the value of the support moment M is ____ kN-m.



Answer: 5

Exp:

574



$$M_{BC} = 10 \times 1 = -10 \text{ kN.m}$$

$$\therefore M_{BA} = + 10 \text{ kN.m}$$

$$\therefore M_{AB} = \frac{10}{2} \text{ kN.m (half moment carry over)}$$

Q. No. 26-55 carry Two Marks Each

26. The directional derivative of the field $u(x, y, z) = x^2 - 3yz$ in the direction of the vector $(\hat{i} + \hat{j} - 2\hat{k})$ at point (2, -1, 4) is _____.

Answer: -5.72

Exp: Let
$$u(x, y, z) = x^2 - 3yz$$

$$\vec{a} = i + j - 2k \text{ and } P(2, -1, 4)$$

$$\nabla u = i \frac{\partial u}{\partial x} + j \frac{\partial u}{\partial y} + k \frac{\partial u}{\partial z}$$

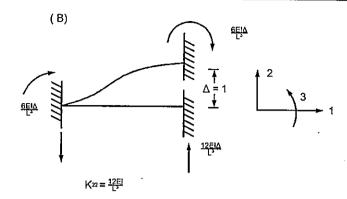
$$= 12x + j (-3z) + k (-3y)$$

$$\nabla u|_{(2,-1,4)} = 4i - 12j + 3k$$

$$|\vec{a}| = \sqrt{1 + 1 + 4} = \sqrt{6}$$

573

Exp:



22. The penetration value of a bitumen sample tested at 25°C is 80. When this sample is heated to 60 °C and tested again, the needle of the penetration test apparatus penetrates the bitumen sample by d mm. The value of d CANNOT be less than ____ mm.

Answer: 8

23. The development length of a deformed reinforcement bar can be expressed as $(1/k)(\phi\sigma_s/\tau_{bd})$. From the IS:456-2000, the value of k can be calculated as ____.

Answer: 6.4

Exp: $L_d = \frac{\phi \sigma_s}{4\tau_{bd}}$ But for deformed bars τ_{bd} is increased by 60%.

So,

$$Ld = \frac{\phi \sigma_{st}}{4 \times 1.6 \times \tau_{bd}} = \frac{\phi \sigma_{s}}{6.4 \tau_{bd}}$$

So,
$$k = 6.4$$
.

- 24. Total Kjeldahl Nitrogen (TKN) concentration (mg/L as N) in domestic sewage is the sum of the concentrations of:
 - (A) organic and inorganic nitrogen in sewage
 - (B) organic nitrogen and nitrate is sewage
 - (C) organic nitrogen and ammonia is sewage
 - (D) ammonia and nitrate in sewage

Answer: (C)

Exp: Total Kjeldahl Nitrogen (TKN) = Ammonia (60%) + Organic Nitrogen (40%)

FOR MORE EXCLUSIVE

(Civil, Mechanical, EEE, ECE) ENGINEERING & GENERAL STUDIES

(Competitive Exams)

TEXT BOOKS, IES GATE PSU's TANCET & GOVT EXAMS
NOTES & ANNA UNIVERSITY STUDY MATERIALS

VISIT

www.EasyEngineering.net

AN EXCLUSIVE WEBSITE FOR ENGINEERING STUDENTS & GRADUATES



**Note: Other Websites/Blogs Owners Please do not Copy (or) Republish this Materials without Legal Permission of the Publishers.

**Disclimers: EasyEngineering not the original publisher of this Book/Material on net. This e-book/Material has been collected from other sources of net.