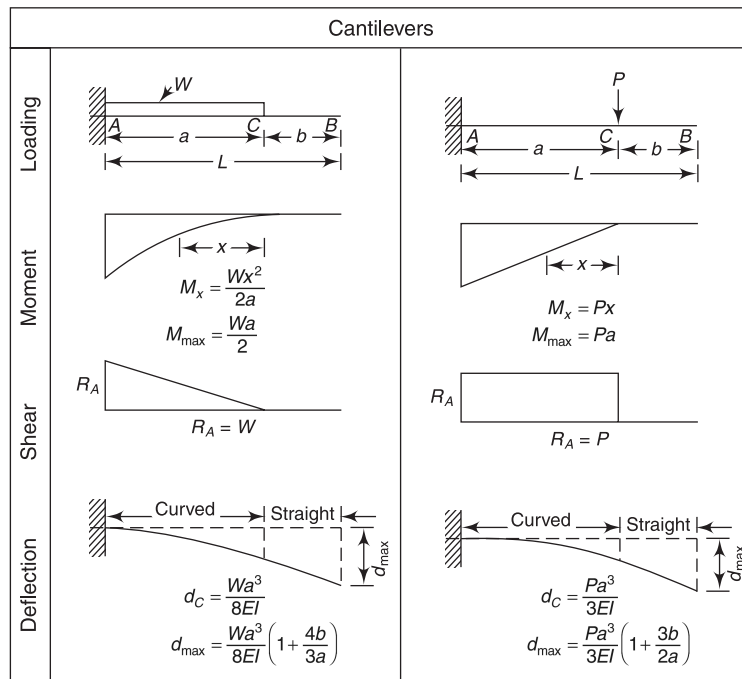


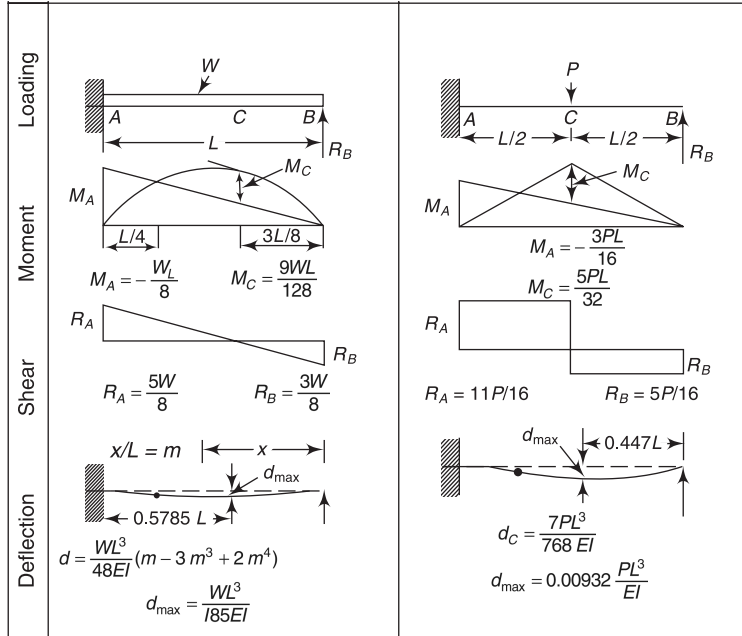
Bending Moment, Shear Force, and Deflection of Beams and Frames

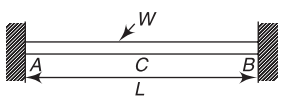
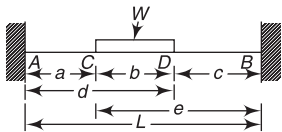
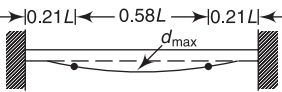
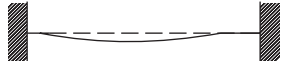
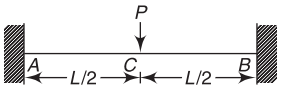
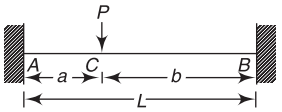
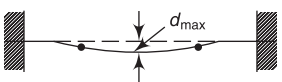

The analysis methods described in Chapter 4 are to be used to find out the bending moments, shear force, and deflection of beams and frames. However, in this Appendix, formulae are provided for calculating the same for some beams and simple frames, which can be used for the design of these structures.



A.D.2 Design of Steel Structures

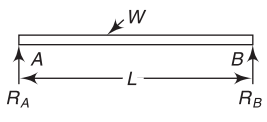
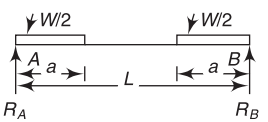


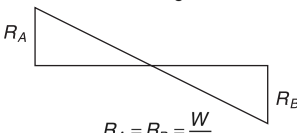
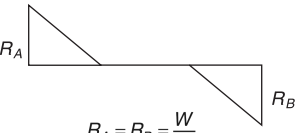
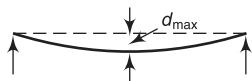

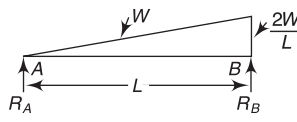
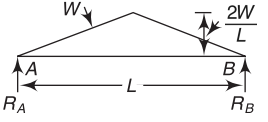

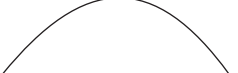
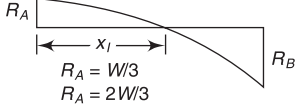
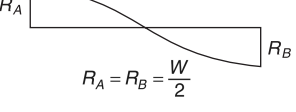
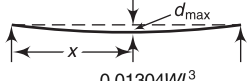
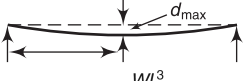
Propped Cantilevers



		Built-in-beams			
Loading	Moment	Shear	Deflection		
				$M_A = M_B = -\frac{WL}{12}$ $M_C = \frac{WL}{24}$	$M_A = \frac{-W}{12L^2b} [e^3(4L - 3e) - c^3(4L - 3c)]$ $M_B = \frac{-W}{12L^2b} [d^3(4L - 3d) - a^3(4L - 3a)]$
				$R_A = R_B = W/2$	<p>When r is simple support reaction</p> $R_A = r_A + \frac{M_A - M_B}{L} \quad R_B = r_B + \frac{M_B - M_A}{L}$
				 $d_{\max} = \frac{WL^3}{384EI}$	 <p>When $a = c$, d_{\max}</p> $= \frac{W}{384EI} (L^3 + 2L^2a + 4La^2 - 8a^3)$
Loading	Moment	Shear	Deflection		
				$-M_A = -M_B = M_C = PL/8$	$M_A = -\frac{Pab^2}{L^2} \quad M_B = -\frac{Pba^2}{L^2}$ $M_C = \frac{2Pa^2b^2}{L^3}$
				$R_A = R_B = P/2$	$R_A = P \left(\frac{b}{L}\right)^2 \left(1 + 2\frac{a}{L}\right)$ $R_B = P \left(\frac{a}{L}\right)^2 \left(1 + 2\frac{b}{L}\right)$
				 $d_{\max} = \frac{PL^3}{192EI}$	 $d_C = \frac{Pa^3b^3}{3EIL^3}$ $d_{\max} = \frac{2Pa^2b^3}{3E(3L-2a)^2} \text{ when } x = \frac{L^2}{3L-2a}$

A.D.4 Design of Steel Structures

Simply supported beams	
Loading	
Moment	$M_{\max} = \frac{PL}{4}$
Shear	$R_A = R_B = \frac{P}{2}$
Deflection	$d_{\max} = \frac{PL^3}{48EI}$
Loading	
Moment	$M_{\max} = Pa$
Shear	$R_A = R_B = P$
Deflection	$d_{\max} = \frac{PL^3}{6EI} \left[\frac{3a}{4L} - \left(\frac{a}{L} \right)^3 \right]$
Loading	
Moment	$M_{\max} = \frac{Pab}{L}$
Shear	$R_A = Pb/L \quad R_B = Pa/L$
Deflection	<p>d_{\max} always occurs within 0.0774 L of the centre of the beam. When $b \geq a$,</p> $d_{\text{centre}} = \frac{PL^3}{48EI} \left[\frac{3a}{L} - 4 \left(\frac{a}{L} \right)^3 \right]$ <p>This value is always within 2.5% of the maximum value.</p>
Loading	
Moment	$M_A = M_B = -\frac{wN^2}{2} \quad M_D = \frac{wL^2}{8} + M_A$
Shear	$R_A = R_B = w \left(N + \frac{L}{2} \right)$
Deflection	$d_C = d_E = \frac{wL^3 N}{24EI} (1 - 6n^2 - 3n^3)$ $d_D = \frac{wL^4}{384EI} (5 - 24n^2)$ <p>where $n = \frac{N}{L}$</p>

		Simply supported beams			
Loading	Moment	Shear	Deflection		
				 $M_x = \frac{Wx}{2} \left(1 - \frac{x}{L}\right)$ $M_{\max} = \frac{WL}{8}$	 $M_{\max} = \frac{Wa}{4}$
				 $R_A = R_B = \frac{W}{2}$	 $R_A = R_B = \frac{W}{2}$
				 $d_{\max} = \frac{5}{384} \cdot \frac{WL^3}{EI}$	 $d_{\max} = \frac{Wa(3L^2 - 2a^2)}{96EI}$
Loading	Moment	Shear	Deflection		
				 $M_x = \frac{Wx}{3} \left(1 - \frac{x^2}{L^2}\right)$ $M_{\max} = 0.128WL$ <p>when $x_1 = 0.5774L$</p>	 $M_x = Wx \left(\frac{1}{2} - \frac{2x^2}{3L^2}\right)$ $M_{\max} = WL/6$
				 $R_A = W/3$ $R_B = 2W/3$	 $R_A = R_B = \frac{W}{2}$
				 $d_{\max} = \frac{0.01304WL^3}{EI}$ <p>when $x = 0.5193L$</p>	 $d_{\max} = \frac{WL^3}{60EI}$

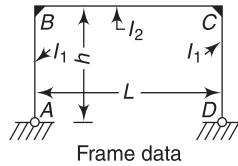
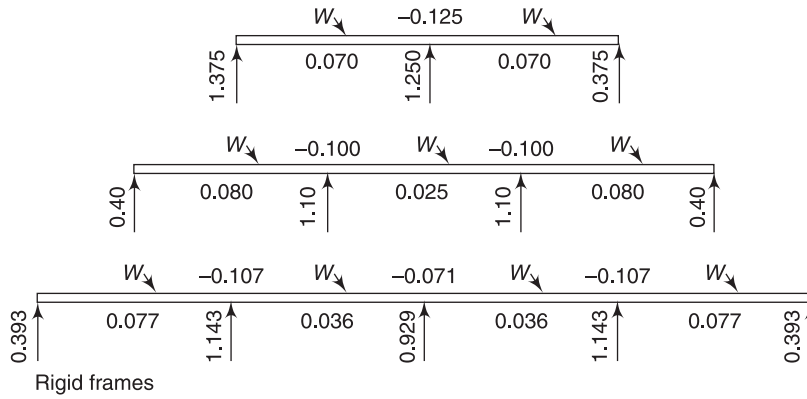
A.D.6 Design of Steel Structures

Equal Span Continuous Beams
Uniformly Distributed Loads (UDL)

Moment = coefficient $\times W \times L$

Reaction = coefficient $\times W$

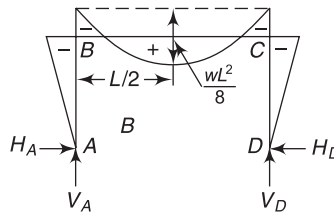
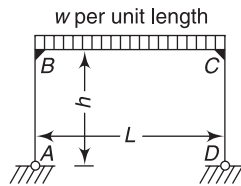
where W is the UDL on one span only and L is one span



Coefficients

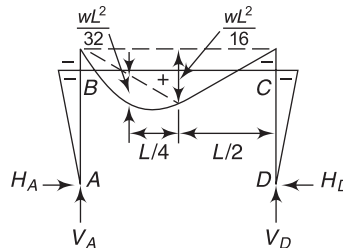
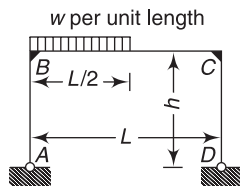
$$k = \frac{I_2}{I_1} \cdot \frac{h}{L}$$

$$N = 2k + 3$$



$$M_B = M_C = -\frac{wL^2}{4N} \quad M_{\max} = \frac{wL^2}{8} + M_B$$

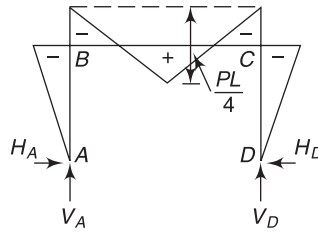
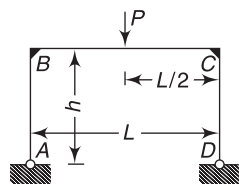
$$V_A = V_D = \frac{wL}{2} \quad H_A = H_D = -\frac{M_B}{h}$$



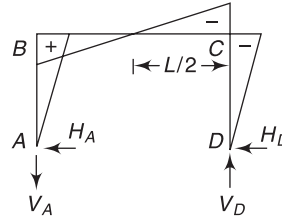
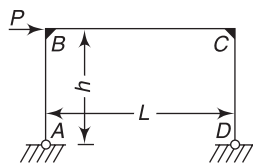
$$M_B = M_C = -\frac{wL^2}{8N}$$

$$V_A = \frac{3wL}{8} \quad V_D = \frac{wL}{8} \quad H_A = H_D = -\frac{M_B}{h}$$

Bending Moment, Shear Force, and Deflection of Beams and Frames **A.D.7**

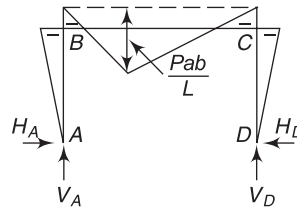
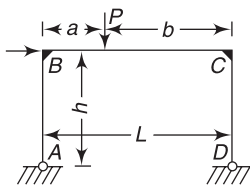


$$M_B = M_C = -\frac{3PL}{8N} \quad V_A = V_D = \frac{P}{2} \quad H_A = H_D = -\frac{M_B}{h}$$



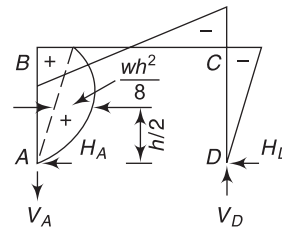
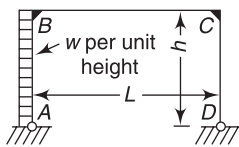
$$M_B = -M_C = +\frac{Ph}{2}$$

$$V_A = -V_D = -\frac{Ph}{L} \quad H_A = -H_D = -\frac{P}{2}$$



$$M_B = M_C = -\frac{Pab}{L} \cdot \frac{3}{2N}$$

$$V_A = \frac{Pb}{L} \quad V_D = \frac{Pa}{L} \quad H_A = H_D = -\frac{M_B}{h}$$

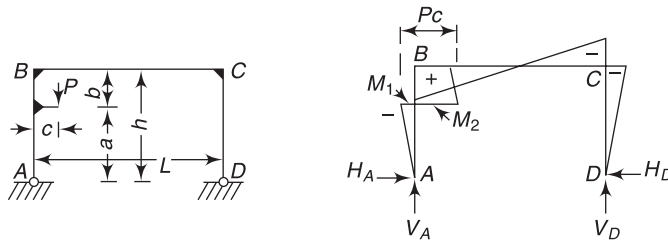


$$M_B = \frac{wh^2}{4} \left[-\frac{k}{2N} + 1 \right] \quad H_D = -\frac{M_C}{h}$$

$$M_C = \frac{wh^2}{4} \left[-\frac{k}{2N} - 1 \right] \quad H_A = -(wh - H_D)$$

$$V_A = -V_D = -\frac{wh^2}{2L}$$

A.D.8 Design of Steel Structures



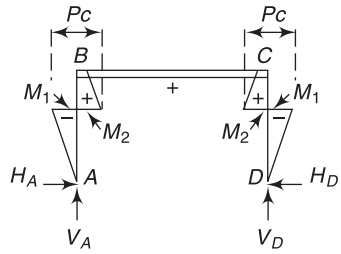
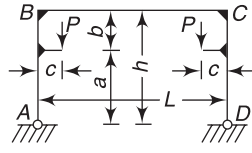
Constant: $a_1 = \frac{a}{h}$

$$M_B = \frac{Pc}{2} \left[\frac{(3a_1^2 - 1)k}{N} + 1 \right] \quad H_A = H_D = -\frac{Mc}{h}$$

$$M_C = \frac{Pc}{2} \left[\frac{(3a_1^2 - 1)k}{N} - 1 \right]$$

$$V_D = \frac{Pc}{L} \quad V_A = P - V_D$$

$$M_1 = -H_A a \quad M_2 = Pc - H_A a$$

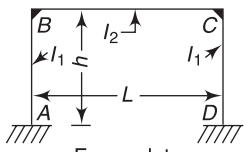


Constant: $a_1 = \frac{a}{h}$

$$M_B = M_C = \frac{Pc(3a_1^2 - 1)k}{N}$$

$$H_A = H_D = \frac{Pc - M_B}{h} \quad V_A = V_D = P$$

$$M_1 = -H_A a \quad M_2 = Pc - H_A a$$



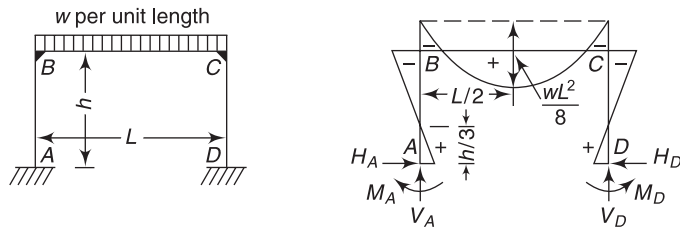
Frame data

Coefficients:

$$k = \frac{I_2}{I_1} \cdot \frac{h}{L}$$

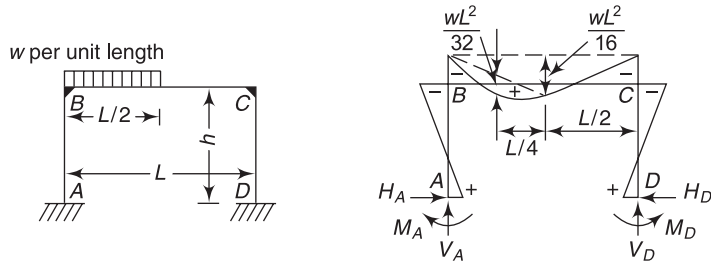
$$N_1 = k + 2 \quad N_2 = 6k + 1$$

Bending Moment, Shear Force, and Deflection of Beams and Frames A.D.9



$$M_A = M_D = \frac{wL^2}{12N_1} \quad M_B = M_C = -\frac{wL^2}{6N_1} = -2M_A$$

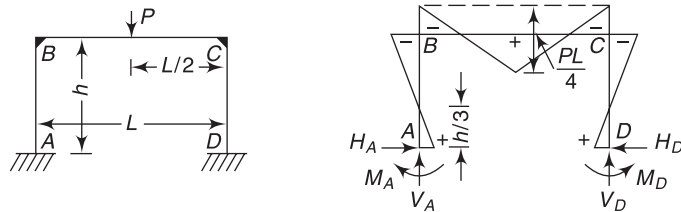
$$M_{\max} = \frac{wL^2}{8} + M_B \quad V_A = V_D = \frac{wL}{2} \quad H_A = H_D = \frac{3M_A}{h}$$



$$M_A = \frac{wL^2}{8} \left[\frac{1}{3N_1} - \frac{1}{8N_2^2} \right] \quad M_B = -\frac{wL^2}{8} \left[\frac{2}{3N_1} + \frac{1}{8N_2} \right]$$

$$M_D = \frac{wL^2}{8} \left[\frac{1}{3N_1} + \frac{1}{8N_2} \right] \quad M_C = -\frac{wL^2}{8} \left[\frac{2}{3N_1} - \frac{1}{8N_2} \right]$$

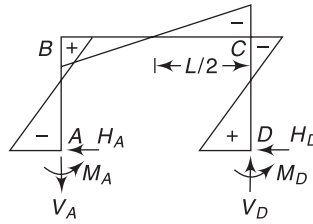
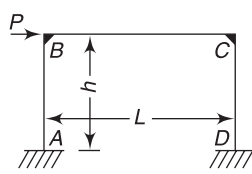
$$V_D = \frac{wL}{8} \left[1 - \frac{1}{4N_2} \right] \quad V_A = \frac{wL}{2} - V_D \quad H_A = H_D = \frac{wL^2}{8hN_1}$$



$$M_A = M_D = +\frac{PL}{8N_1} \quad M_B = M_C = -2M_A$$

$$V_A = V_D = \frac{P}{2} \quad H_A = H_D = \frac{3M_A}{h}$$

A.D.10 Design of Steel Structures



$$M_A = -\frac{Ph}{2} \cdot \frac{3k+1}{N_2}$$

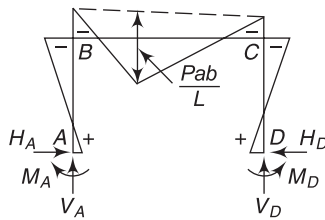
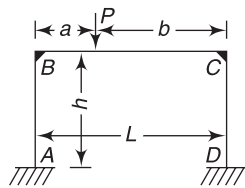
$$M_B = +\frac{Ph}{2} \cdot \frac{3k}{N_2}$$

$$M_D = +\frac{Ph}{2} \cdot \frac{3k+1}{N_2}$$

$$M_C = -\frac{Ph}{2} \cdot \frac{3k}{N_2}$$

$$H_A = -H_D = -\frac{P}{2}$$

$$V_A = -V_D = -\frac{2M_B}{L}$$



Constants: $a_1 = a/L$

$b_1 = b/L$

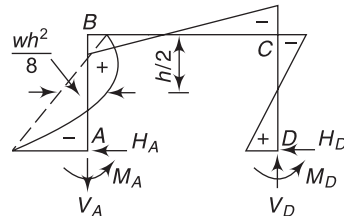
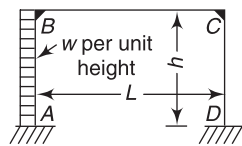
$$M_A = +\frac{Pab}{L} \left[\frac{1}{2N_1} - \frac{b_1 - a_1}{2N_2} \right]$$

$$M_B = -\frac{Pab}{L} \left[\frac{1}{N_1} + \frac{b_1 - a_1}{2N_2} \right]$$

$$M_D = +\frac{Pab}{L} \left[\frac{1}{2N_1} + \frac{b_1 - a_1}{2N_2} \right]$$

$$M_C = -\frac{Pab}{L} \left[\frac{1}{N_1} - \frac{b_1 - a_1}{2N_2} \right]$$

$$V_A = Pb_1 \left[1 + \frac{a_1(b_1 - a_1)}{N_2} \right] \quad V_D = P - V_A \quad H_A = H_D = \frac{3Pab}{2LhN_1}$$



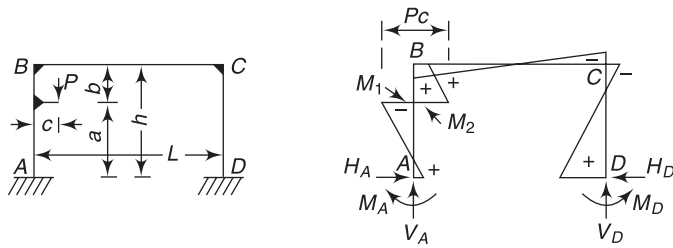
$$M_A = \frac{wh^2}{4} \left[-\frac{k+3}{6N_1} - \frac{4k+1}{N_2} \right]$$

$$M_B = \frac{wh^2}{4} \left[-\frac{k}{6N_1} + \frac{2k}{N_2} \right]$$

$$M_D = \frac{wh^2}{4} \left[-\frac{k+3}{6N_1} + \frac{4k+1}{N_2} \right]$$

$$M_C = \frac{wh^2}{4} \left[-\frac{k}{6N_1} - \frac{2k}{N_2} \right]$$

$$H_D = \frac{wh(2k+3)}{8N_1} \quad H_A = -(wh - H_D) \quad V_A = -V_D = -\frac{wh^2 k}{LN_2}$$



Constants: $a_1 = \frac{a}{h}$ $b_1 = \frac{b}{h}$

$$X_1 = \frac{Pc}{2N_1} [1 + 2b_1k - 3b_1^2(k+1)] \quad X_2 = \frac{Pcka_1(3a_1 - 2)}{2N_1}$$

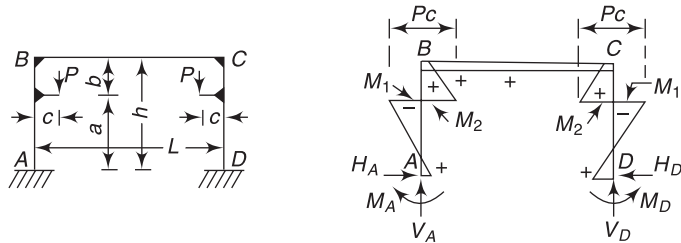
$$X_3 = \frac{3Pcka_1}{N_2}$$

$$M_A = +X_1 - \left(\frac{Pc}{2} - X_3\right) \quad M_B = +X_2 + X_3$$

$$M_D = +X_1 + \left(\frac{Pc}{2} - X_3\right) \quad M_C = +X_2 - X_3$$

$$H_A = H_D = \frac{Pc}{2h} + \frac{X_1 - X_2}{h} \quad V_D = \frac{2X_3}{L} \quad V_A = P - V_D$$

$$M_1 = M_A - H_A a \quad M_2 = M_B + H_D b$$



Constants: $a_1 = \frac{a}{h}$ $b_1 = \frac{b}{h}$

$$X_1 = \frac{Pc}{2N_1} [1 + 2b_1k - 3b_1^2(k+1)] \quad X_2 = \frac{Pcka_1(3a_1 - 2)}{2N_1}$$

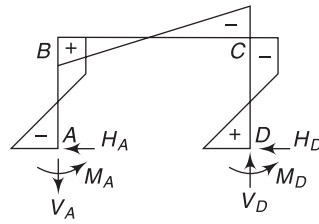
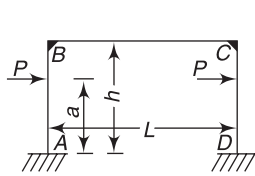
$$M_A = M_D = \frac{Pc}{N_1} [1 + 2b_1k - 3b_1^2(k+1)] = 2X_1$$

$$M_B = M_C = \frac{Pcka_1(3a_1 - 2)}{N_1} = 2X_2$$

$$V_A = V_D = P \quad H_A = H_D = \frac{Pc + M_A - M_B}{h}$$

$$M_1 = M_A - H_A a \quad M_2 = M_B + H_D b$$

A.D.12 Design of Steel Structures



Constants: $a_1 = \frac{a}{h}$

$$M_A = -Pa + X_1$$

$$M_D = +Pa - X_1$$

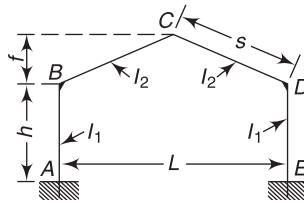
$$V_A = -V_D = -\frac{2X_1}{L}$$

$$X_1 = \frac{3Paa_1k}{N_2}$$

$$M_B = X_1$$

$$M_C = -X_1$$

$$H_A = -H_D = -P$$



Coefficients:

$$k = \frac{l_2}{l_1} \cdot \frac{h}{s} \quad \phi = \frac{f}{h}$$

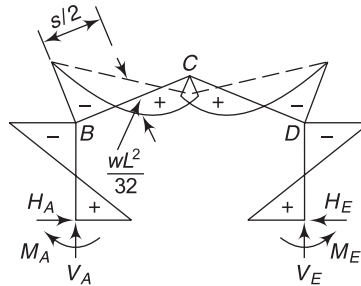
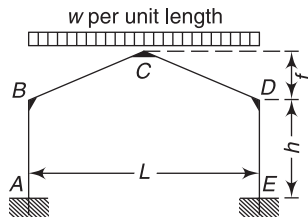
$$m = 1 + \phi$$

$$B = 3k + 2 \quad C = 1 + 2m$$

Frame data

$$K_1 = 2(k + 1 + m + m^2) \quad K_2 = 2(k + \phi^2)$$

$$R = \phi C - k \quad N_1 = K_1 K_2 - R^2 \quad N_2 = 3k + B$$

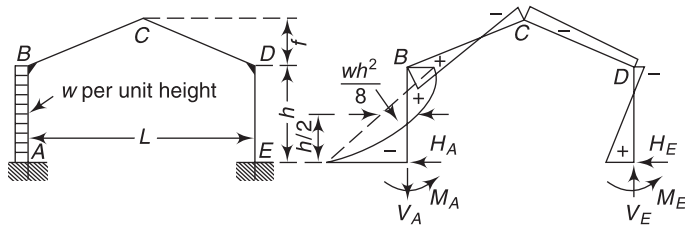


$$M_A = M_E = \frac{wL^2}{16} \cdot \frac{k(8 + 15\phi) + \phi(6 - \phi)}{N_1}$$

$$M_B = M_D = -\frac{wL^2}{16} \cdot \frac{k(16 + 15\phi) + \phi^2}{N_1}$$

$$M_C = \frac{wL^2}{8} - \phi M_A + m M_B$$

$$V_A = V_E = \frac{wL}{2} \quad H_A = H_E = \frac{M_A - M_B}{h}$$



$$\text{Constants: } X_1 = \frac{wh^2}{8} \cdot \frac{k(k+6) + k\phi(15+16\phi) + 6\phi^2}{N_1}$$

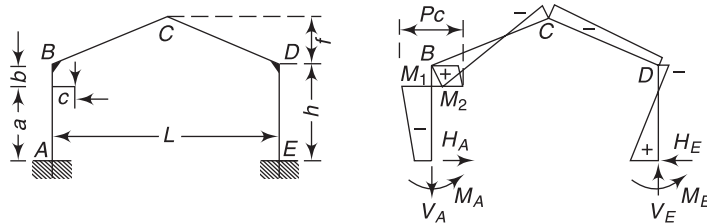
$$X_2 = \frac{wh^2 k(9\phi + 8\phi^2 - k)}{8N_1} \quad X_3 = \frac{wh^2(2k+1)}{2N_2}$$

$$M_A = -X_1 - X_3 \quad M_B = +X_2 + \left(\frac{wh^2}{4} - X_3 \right)$$

$$M_E = -X_1 + X_3 \quad M_D = +X_2 - \left(\frac{wh^2}{4} - X_3 \right)$$

$$M_C = -\frac{whf}{4} + \phi X_1 + mX_2$$

$$V_A = -V_E = -\frac{wh^2}{2L} + \frac{2X_3}{L} \quad H_E = \frac{wh}{4} - \frac{X_1 + X_2}{h} \quad H_A = -(wh - H_E)$$



$$\text{Constants: } a_1 = \frac{a}{h} \quad b_1 = \frac{b}{h}$$

$$Y_1 = Pc[2\phi^2 - (1 - 3b_1^2)k] \quad Y_2 = Pc[\phi c - (3a_1^2 - 1)k]$$

$$X_1 = \frac{Y_1 K_1 - Y_2 R}{2N_1} \quad X_2 = \frac{Y_2 K_2 - Y_1 R}{2N_1} \quad X_3 = \frac{Pc}{2} \cdot \frac{b - 3(a_1 - b_1)k}{N_2}$$

$$M_A = -X_1 - X_3 \quad M_B = +X_2 + \left(\frac{Pc}{2} - X_3 \right)$$

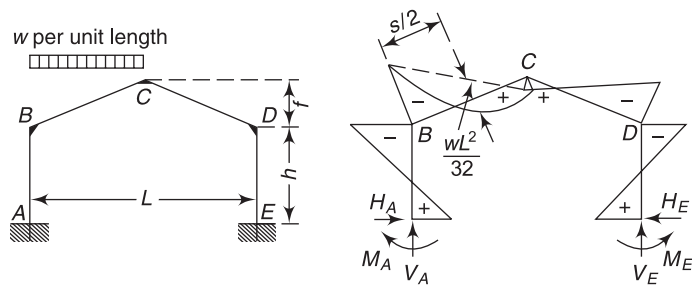
$$M_E = -X_1 + X_3 \quad M_D = +X_2 - \left(\frac{Pc}{2} - X_3 \right)$$

$$M_C = -\frac{\phi Pc}{4} + \phi X_1 + mX_2$$

$$M_1 = M_A - H_A a \quad M_2 = M_B + H_E b$$

$$V_E = \frac{Pc - 2X_3}{L} \quad V_A = P - V_E \quad H_A = H_E = \frac{Pc}{2h} - \frac{X_1 + X_2}{h}$$

A.D.14 *Design of Steel Structures*



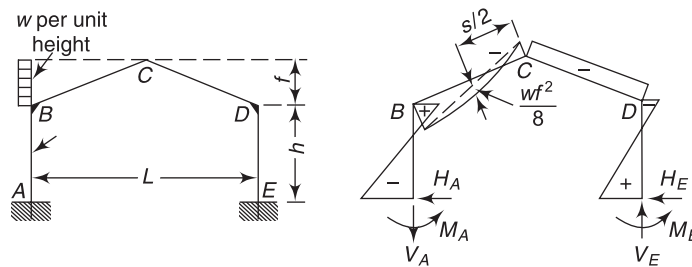
$$\text{Constants: } X_1 = \frac{wL^2}{32} \cdot \frac{k(8 + 15\phi) + \phi(6 - \phi)}{N_1}$$

$$X_2 = \frac{wL^2}{32} \cdot \frac{k(16 + 15\phi) + \phi^2}{N_1} \quad X_3 = \frac{wL^2}{32N_2}$$

$$M_A = +X_1 - X_3 \quad M_B = -X_2 - X_3 \quad M_E = +X_1 + X_3 \quad M_D = -X_2 + X_3$$

$$M_C = \frac{wL^2}{16} - \phi X_1 - mX_2$$

$$V_E = \frac{wL}{8} - \frac{2X_3}{L} \quad V_A = \frac{wL}{2} - V_E \quad H_A = H_E = \frac{X_1 + X_2}{h}$$



$$\text{Constants: } X_1 = \frac{wf^2}{8} \cdot \frac{k(9\phi + 4) + \phi(6 + \phi)}{N_1}$$

$$X_2 = \frac{wf^2}{8} \cdot \frac{k(8 + 9\phi) - \phi^2}{N_1} \quad X_3 = \frac{wfh}{8} \cdot \frac{4b + \phi}{N_2}$$

$$M_A = -X_1 - X_3 \quad M_B = +X_2 + \left(\frac{wfh}{2} - X_3 \right)$$

$$M_E = -X_1 + X_3 \quad M_D = +X_2 - \left(\frac{wfh}{2} - X_3 \right)$$

$$M_C = -\frac{wf^2}{4} + \phi X_1 + mX_2$$

$$V_A = -V_E = -\frac{wfh(2 + \phi)}{2L} + \frac{2X_3}{L} \quad H_E = \frac{wf}{2} - \frac{X_1 + X_2}{h} \quad H_A = -(wf - H_E)$$

Source: Kleinogel, A., *Rahmenformeln*, 11th edn, Verlag von Wilhelm Ernst & Sohn., Berlin, 1949.